DEVELOPMENT PLANS ASHIEY PLANTATION-SECTION IV AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA

DATE: 05-01-02

PIERSON ENGINEERING SURVEYING

P.O. BOX 311 1324 ROANOKE ROAD DALEVILLE, VA 24083

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COVER SHEET

COMMISSION

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SITEBRAEMAR DRIVE CHARLESTON PLACE GREENFIELD ST. RTE. 673

MR. A.R. OVERBAY 3705 WINGSAP ROAD ROANOKE, VIRGINIA 24019 PHONE: (540)992-6600

INDEX OF DRAWINGS

COVER SHEET

- 1. PLAN SHEET
- 2. PLAN SHEET
- 3. ROADWAY AND WATERLINE PROFILE SHEET
- ROADWAY AND WATERLINE PROFILE SHEET
- 5. CROSS-SECTION
- 6. CROSS-SECTION
- 7. CROSS-SECTION
- ROADWAY SPECIFICATIONS
- 9. SEWER DETAIL
- 10. SEWER DETAIL
- 11. WATER DETAIL
- 13. E & S PLAN SHEET

12. E & S PLAN SHEET

- 14. E & S DETAIL SHEET
- SEWER PROFILE
- SEWERLINE PROFILES & GENERAL NOTES
- 17. SEWER PUMP STATION DETAIL SHEET
- 18. OVERALL SCHEMATIC

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RODERICK F. PIERSON

PIERSON ENGINEERING & SURVEYING

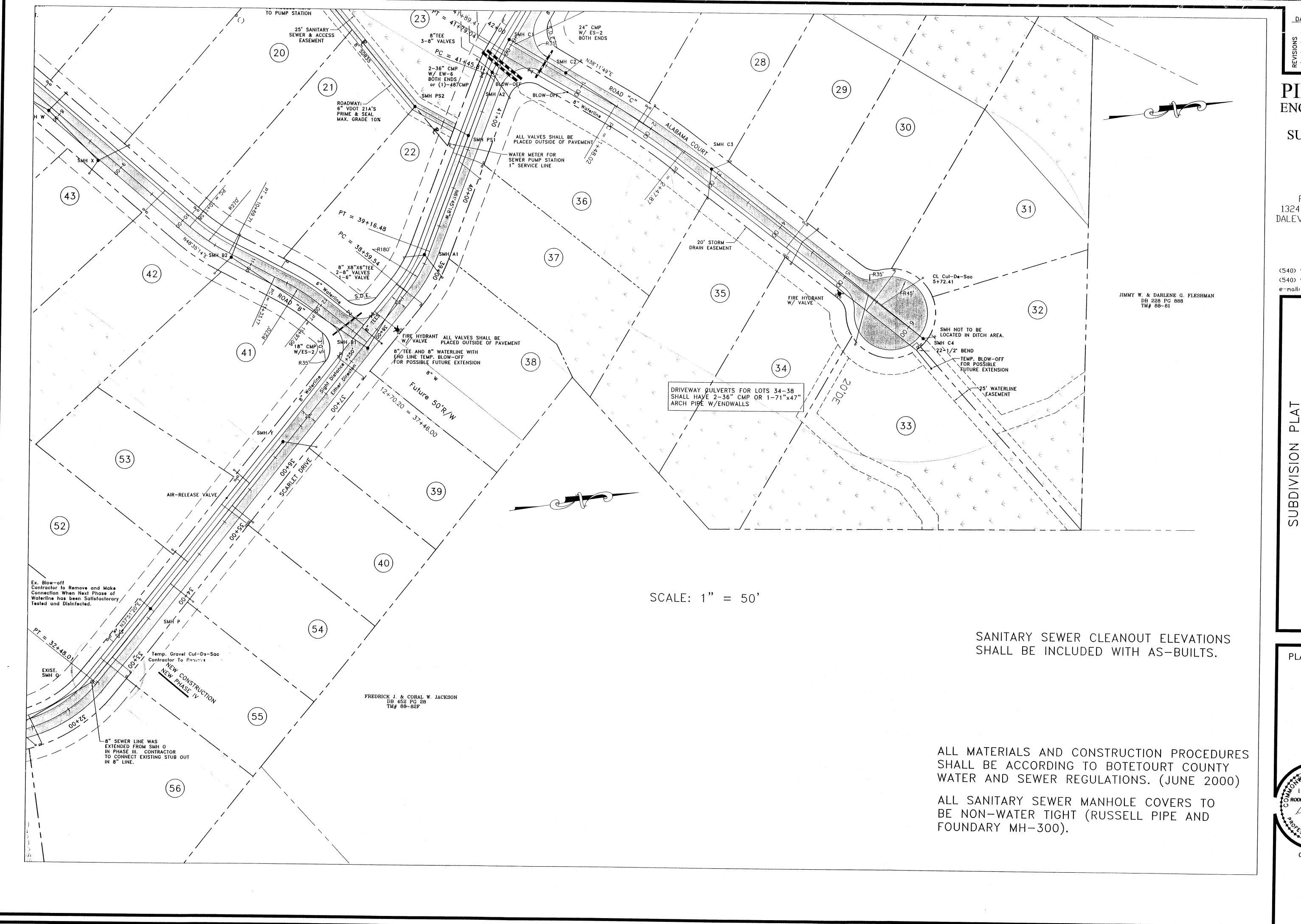
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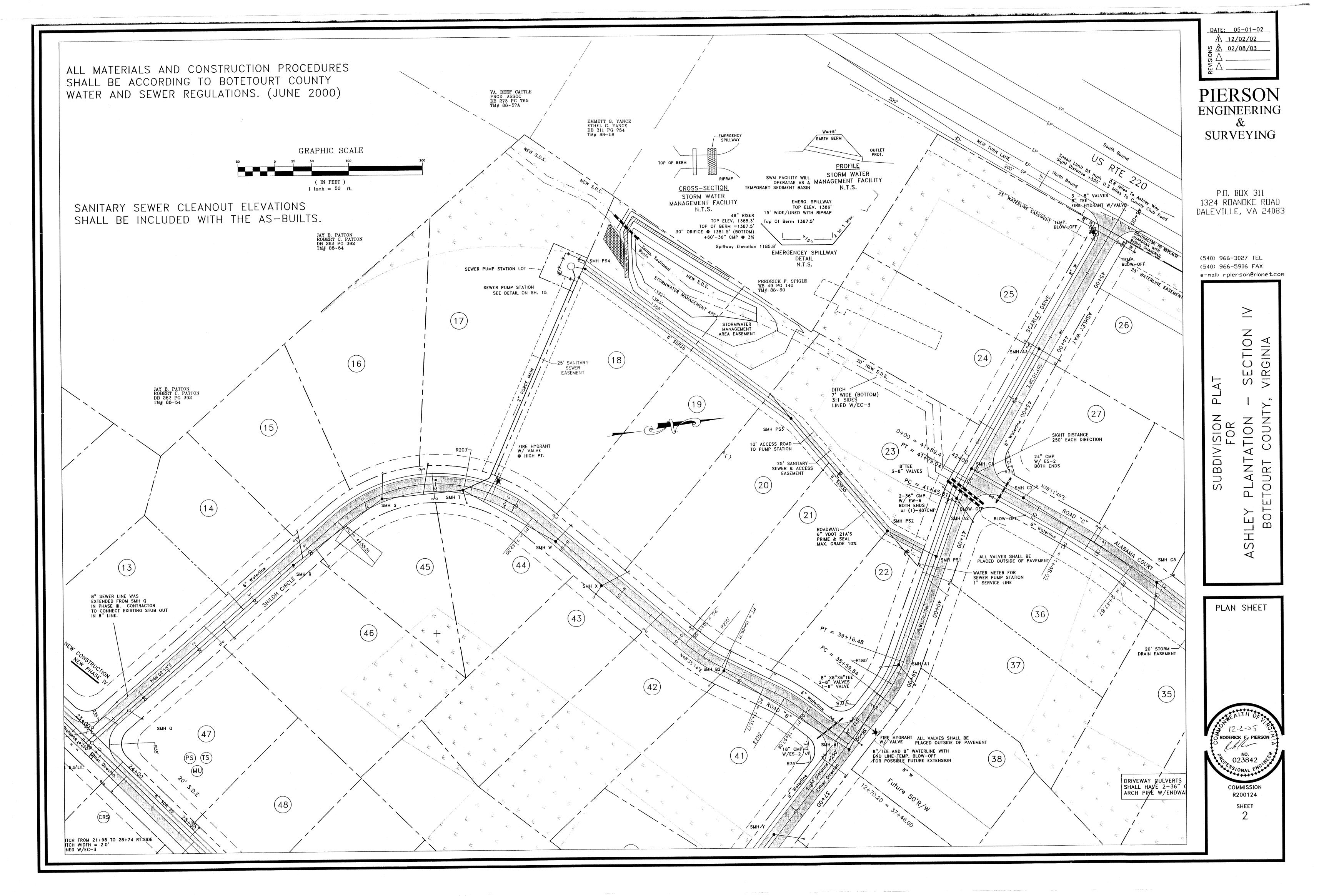
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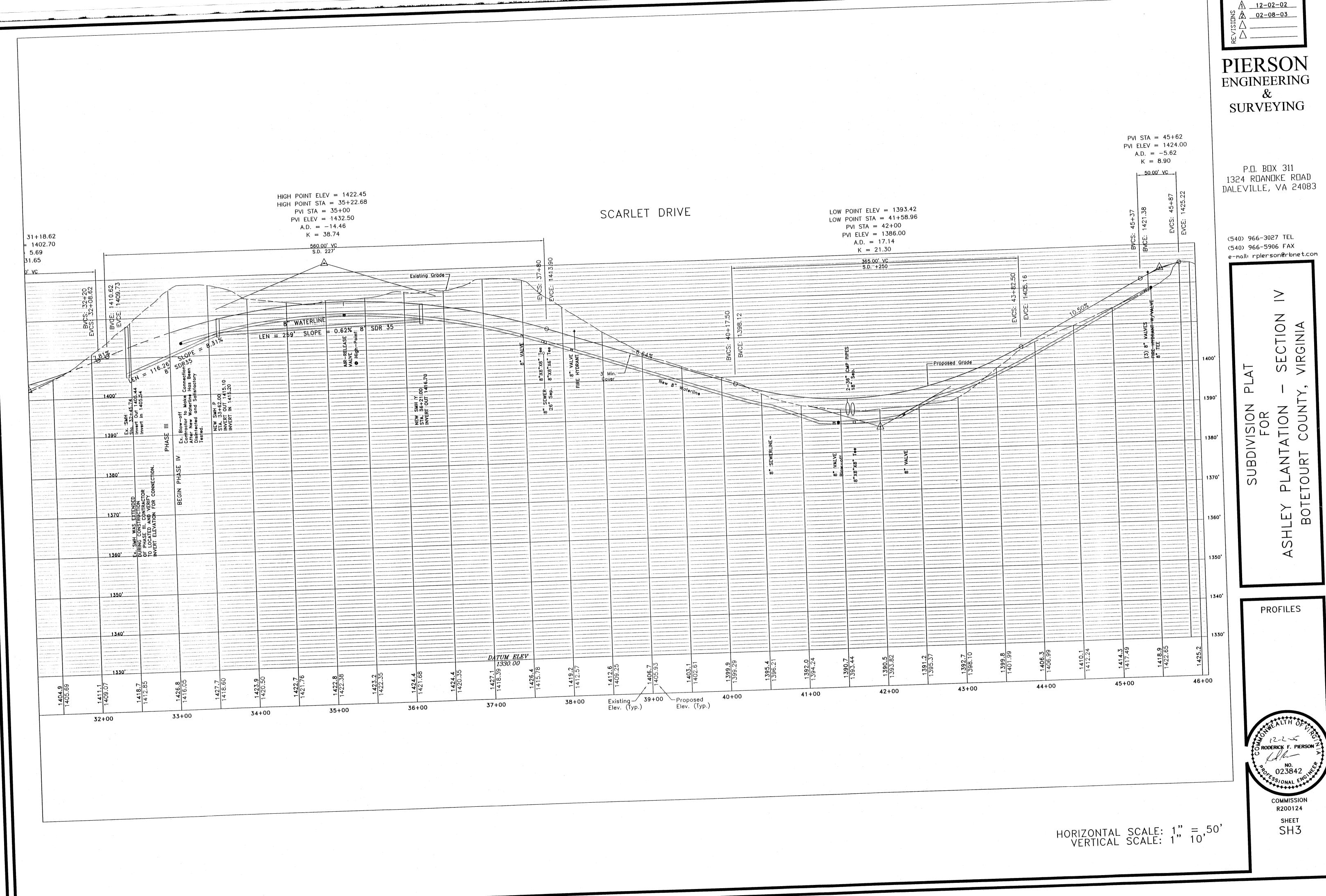
SUBDIVISION PLAT FOR ASHLEY PLANTATION - SECTION IN BOTETOURT COUNTY, VIRGINIA

PLAN SHEET

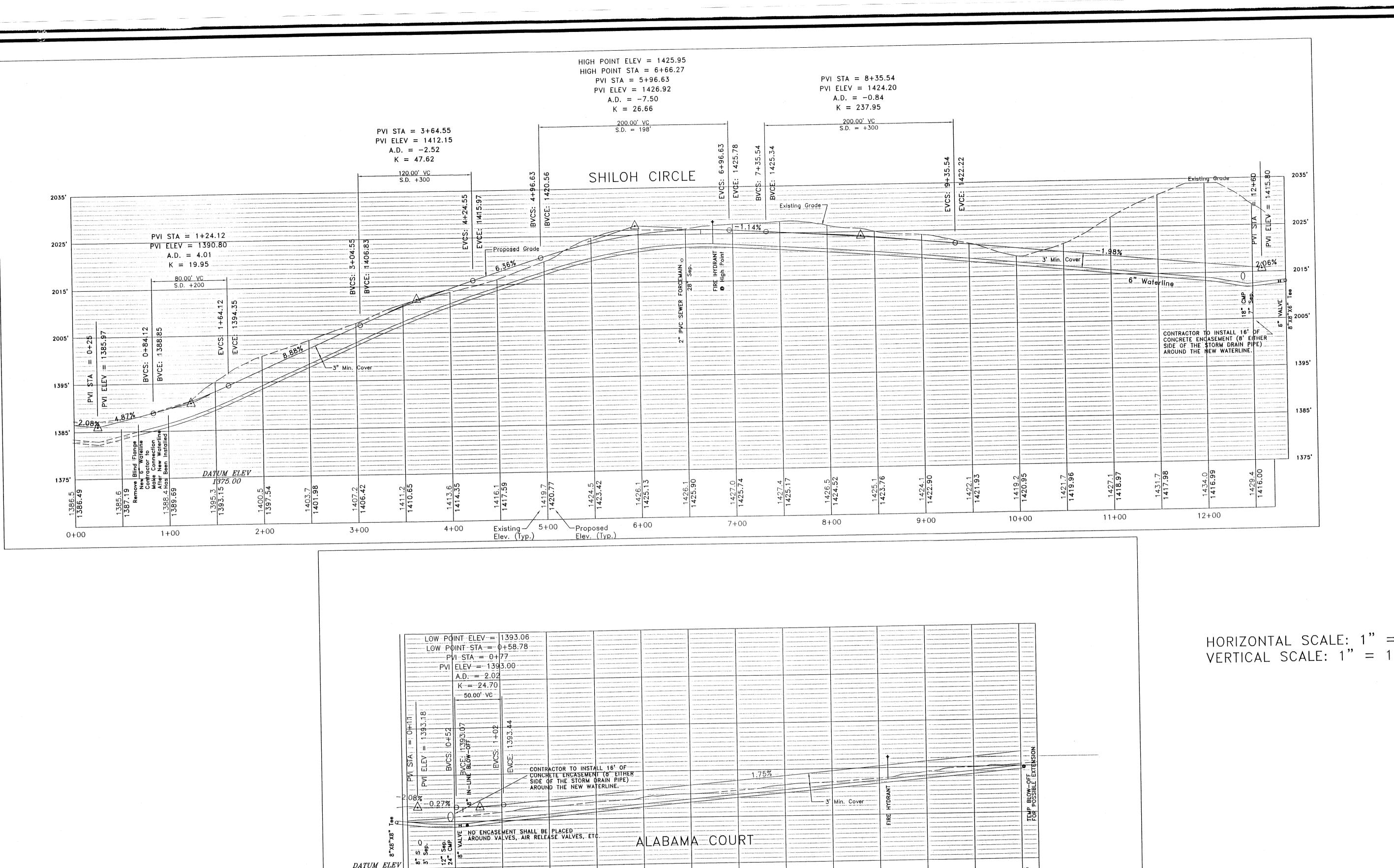
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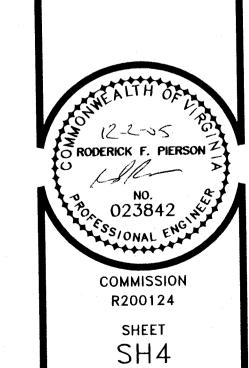
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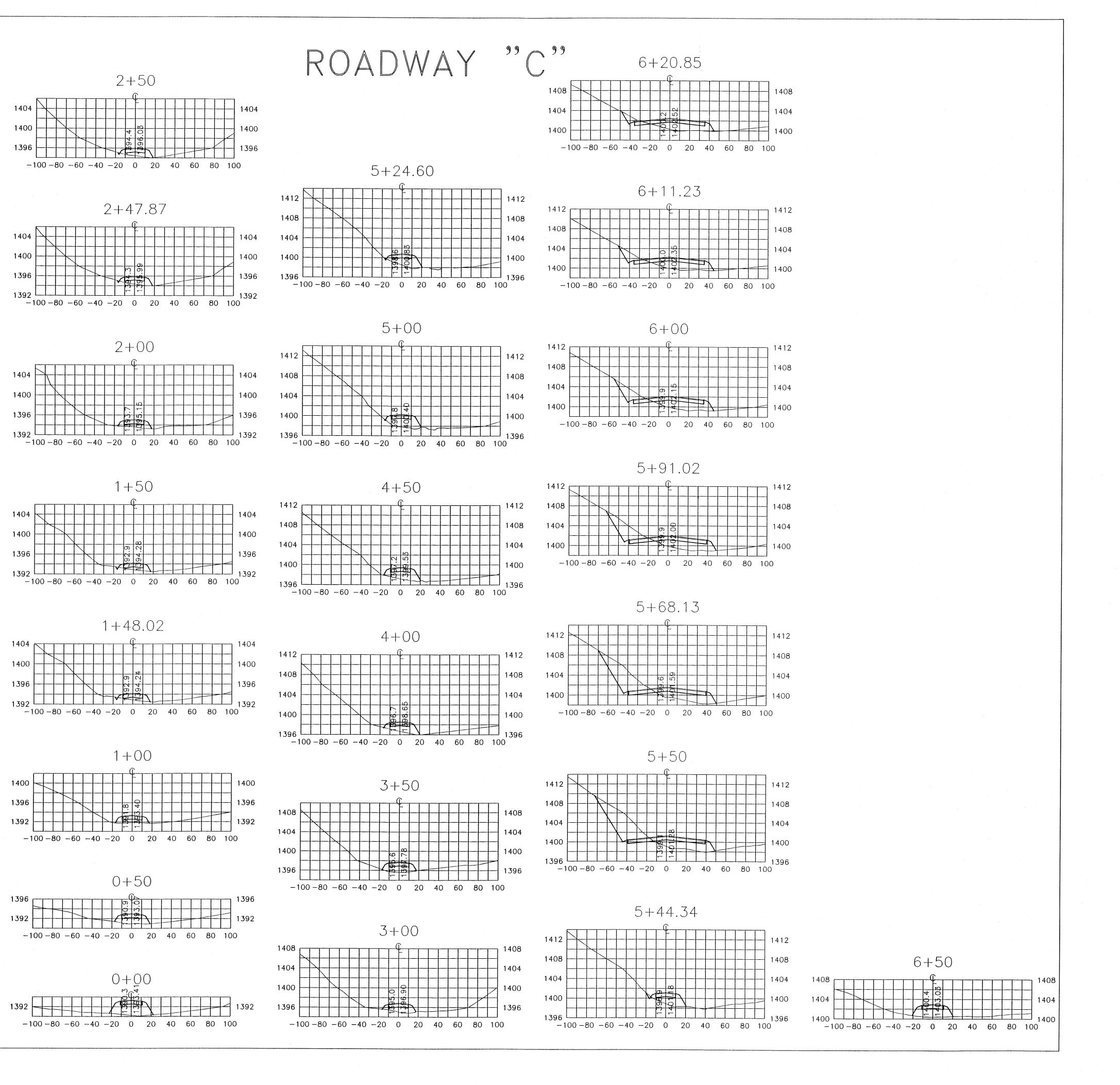
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> SECTION SUBDIVISION FOR

HORIZONTAL SCALE: 1" = 50' VERTICAL SCALE: 1" = 10'

PROFILES





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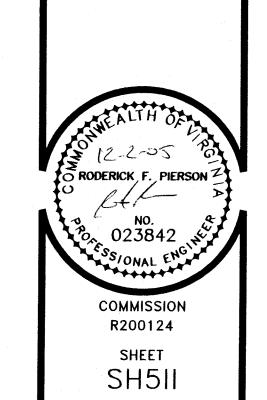
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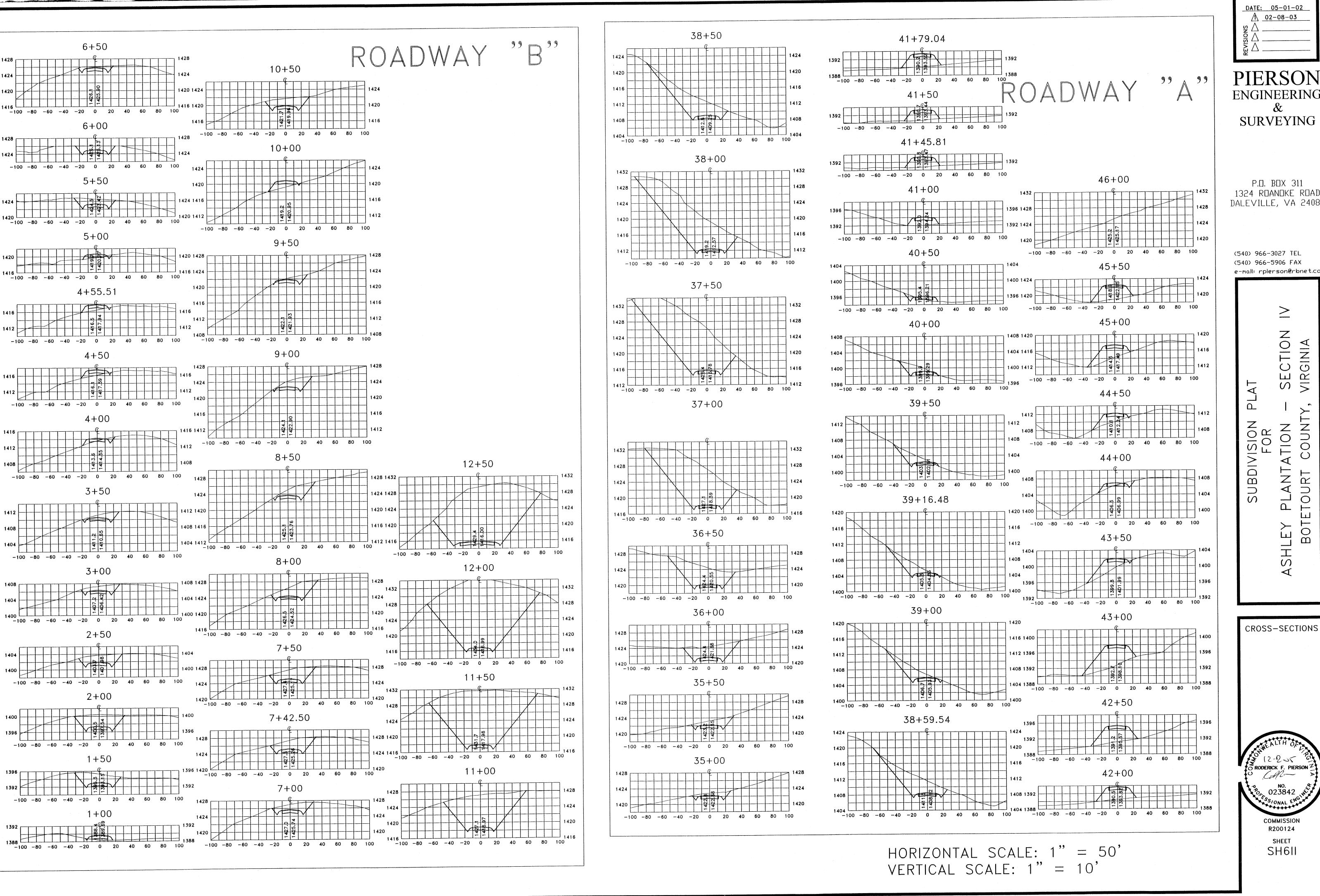
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CROSS-SECTIONS



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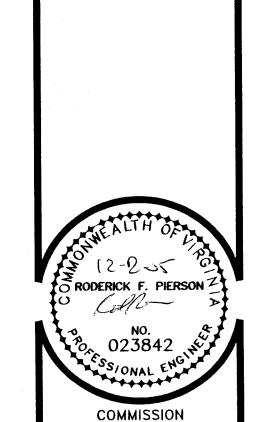
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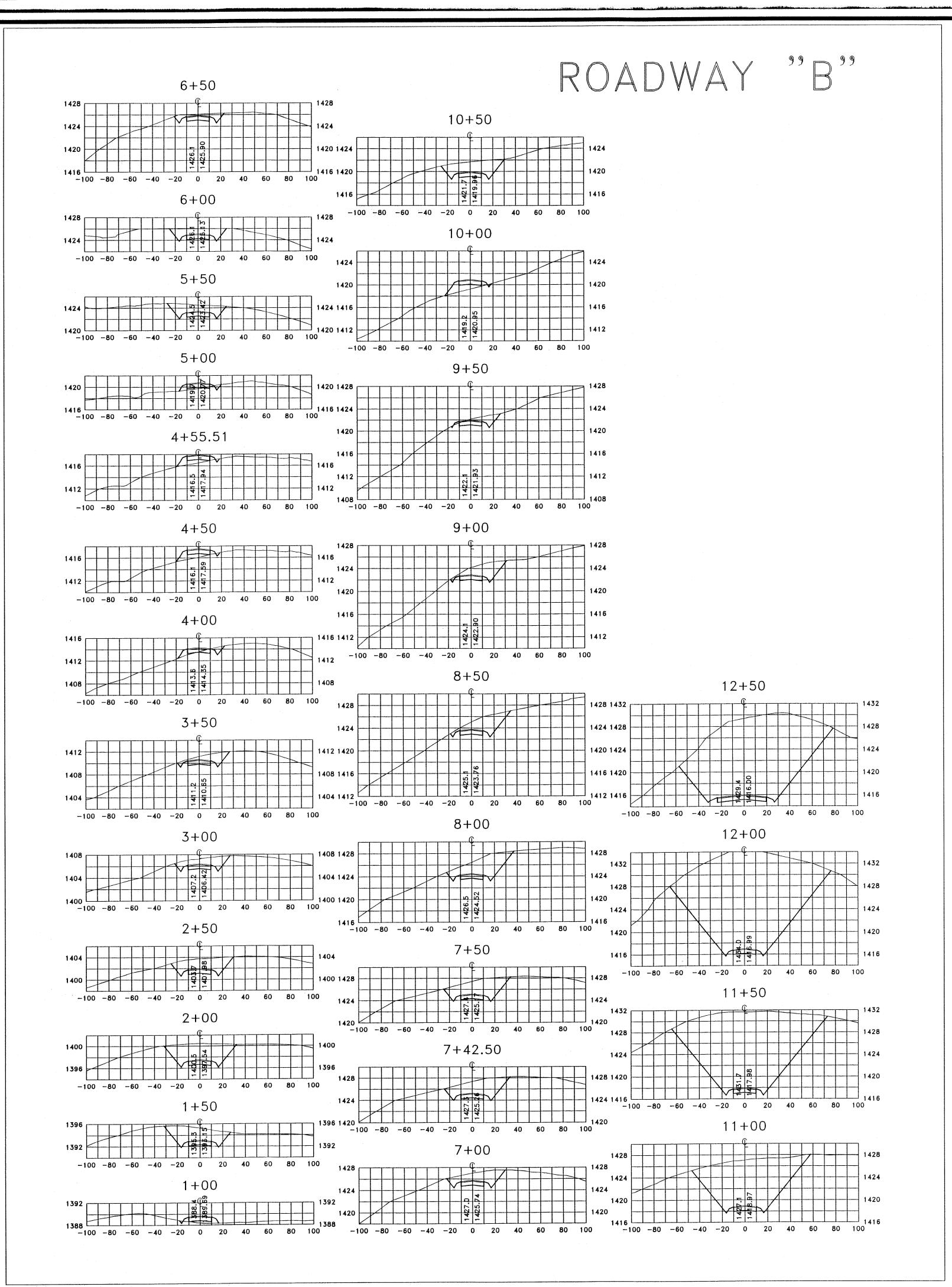
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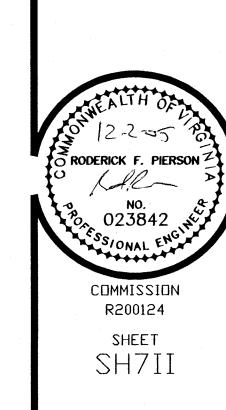
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> SUBDIVISION PLAT FOR ASHLEY PLANTATION - SECTION BOTETOURT COUNTY, VIRGINIA

CROSS-SECTIONS

5O'

HORIZONTAL SCALE: 1" = 50' VERTICAL SCALE: 1" = 10'



VIRGINIA DEPARTMENT OF TRANSPORTATION NOTE:

Quality Control

Streets to be graded and paved and all structural components erected within the proposed rights of way shall be constructed in accordance with the Virginia Department of Transportation Road and Bridge Specifications dated January 2001, Road and Bridge Standards dated January 1, 2002, and The Work Area Protection Manual dated January 1, 1996, Botetourt County. All materials used shall be tested in accordance with standard policies. The developer must contact the office of the Resident Engineer, prior to beginning of any construction at which time an Inspection and Testing Procedure Policy will be drawn. The developer will produce test reports from approved independent laboratories at the developer's expense.

The pavement designs shown are based on a subgrade CBR value 10 or greater. The subgrade soil is to be tested by an independent laboratory and the results submitted to the Virginia Department of Transportation prior to pavement construction. Should the subgrade CBR values be less than 10, then additional base material will be required in accordance with departmental specifications.

The subgrade must be approved by Virginia Department of Transportation prior to placement of the base. Base must be approved by Virginia Department of Transportation for depth, template and compaction before surface is applied.

Utilities

All necessary utility laterals along with provisions for conduits (I.E. water, sewer, storm, gas and telephone) will be constructed prior to placement of base material.

Gas or petroleum transmission lines will not be permitted within the pavement or shoulder element (back of curb to back of curb) of the development. Service laterals crossing and pipe lines located outside the pavement but inside the right of way will be constructed in conformity with ASA B 31.8 Specifications and Safety Regulations. Distribution lines with pressure less than 120 lbs. are unaffected by the above.

Permits will be required for all utilities within street right of way prior to acceptance into the secondary highway system.

Any easements granted to a utility company for placement of power, telephone, etc. must be released prior to acceptance.

Private Entrances

Modified CG-9D gutter will be provided at all entrances to private lots where standard CG-6 curb and gutter is approved for use.

Driveways connecting to roads without curb & gutter shall conform to the pavement, shoulder & slope.

Permits will be required for all private entrances constructed on street rights of way prior to acceptance into the secondary highway system.

Erosion Control and Landscaping

Care must be taken during construction to prevent erosion, dust and mud from damaging adjacent property, clogging ditches, tracking public streets and otherwise creating a public or private nuisance to surrounding areas.

The entire construction area including ditches, channels, back of curbs and or pavement are to be backfilled and seeded at the earliest possible time after final grading.

Drainage easements must be defined by excavated ditches or channels for their full length to well defined existing natural watercourses.

This road will be reviewed during construction for the need of paved gutters. If erosion is encountered in any drainage easement, it will be the responsibility of the developer to sod, rip rap, grout, pave, or to do whatever is necessary to correct the problem.

All vegetation and overburden to be removed form shoulder to shoulder prior to the conditioning (cutting and/or preparation) of the subgrade.

Intersection Pavement Radius

Minimum pavement radius of 25 feet is required at all street intersections.

If the proposed streets are to be traveled by school bus, the return radii must be increased to 50' minimum.

Connections to State - Maintained Roads

While these plans have been approved, such approval does not exempt connections with existing state—maintained roads from critical review at the time permit applications are made. This is necessary in order that the prevailing conditions be taken into consideration regarding safety accompaniments such as turning lanes.

Guardrails

Standard guardrail with safety end sections may be required on fills as deemed necessary by the VDOT Resident Engineer. After completion of rough grading operations, the office of the Resident Engineer, Virginia Department of Transportation, shall be notified so that a field review may be made of the proposed locations.

Where guardrails are to be installed the shoulder width shall be increased in accordance with VDOT Road and Bridge Standards.

Storm Drainage

Field review will be made by the VDOT Resident Engineer during construction to determine the need and limits of paved ditches and/or ditch stabilization treatments, and to determine the need and limits of additional easements. All drainage easements must be cut and made to function to a natural watercourse. Any erosion problems encountered in an easement must be corrected by whatever means necessary prior to subdivision acceptance.

Ditch slopes are to be four to one (4:1) for shoulder widths of six feet (6') or greater and three to one (3:1) for shoulder widths of four feet (4') or five feet (5'), unless otherwise specified in the plans.

Entrance Permit

Contractor shall obtain entrance permit to the existing Virginia Department of Transportation Right of Way from Resident Engineer prior to road construction.

10. Inspection

An inspector will not be furnished except for periodic progress inspection, the above mentioned field reviews and for required stone depths. The developer will be required to post a surety to guarantee the road free of defects for one year after acceptance by the Department of Transportation.

11. Street Maintenance

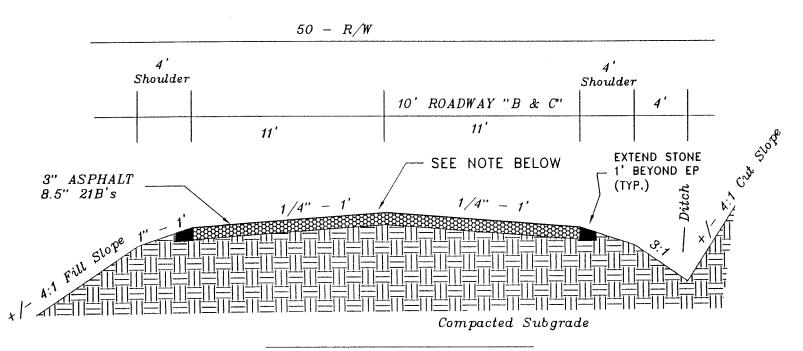
The streets must be properly maintained until acceptance. At such time as all requirements have been met for acceptance, another inspection will be made to determine that the street has been properly maintained.

Underground Utilities

Contractor shall verify location and elevation of all underground utilities shown on the plans in areas of construction prior to starting work by contacting Miss Utility. Contact site engineer immediately if location or elevation is different from that shown on the plans. If there appears to be a conflict, and upon discovery of any utility not shown on the plans call "Miss Utility" of central Virginia at 1-800-552-7001.

Revisions of Specifications and Standards

Approval of these plans will be based on specifications and standards in effect at the time of approval and will be subject, until completion of the roadway and acceptance by the Department, to future revisions, of the Specifications and Standards.



Roadway Typical Section Not To Scale

INSTALLATION OF PIPE CULVERTS AND STORM SEWERS PIPE BEDDING SHOULD CONFORM TO THE 2001 VDOT ROAD AND BRIDGE STANDARDS.

A FIELD REVIEW WILL BE MADE DURING CONSTRUCTION TO DETERMINE THE NEED AND LIMITS OF PAVED DITCH, EC-2 AND/OR EC-3.

ALL ENTRANCE PIPES SHOULD BE A MINIMUM SIZE OF 15" UNLESS OTHERWISE NOTED.

IF SEDIMENT IS LOST FROM THE SITE AND COLLECTS WITHIN THE VDOT RIGHT-OF-WAY IT WILL BE THE DEVELOPERS RESPONSIBILITY TO REMOVE THE SEDIMENT AND/OR CLEAN OUT PIPES AS NECESSARY.

ALL DRAINAGE WAYS SHALL BE MADE TO FUNCTION.

THE DEVELOPER MUST SUBMIT A CBR TEST FOR BOTH SECTIONS 3 & 4 TO DETERMINE ACTUAL PAVEMENT STRUCTURE.

PAVEMENT DESIGN WILL BE REVISED AFTER THE RESULTS OF CBR TESTING ARE OBTAINED. ALL PAVEMENT ITEMS SHALL MEET VDOT 211F-SECTION 211 ASPHALT CONCRETE MIXTURES (SUPERPAVE)

CBR NOTE: SUB-BASE DEPTH FOR SUBGRADE BASED ON CBR VALUE OF 10. SOIL TEST OF SUBGRADE MUST BE TAKEN FOR ACTUAL DETERMINATION OR REQUIRED SUB-BASE THICKNESS PRIOR TO CONSTRUCTION.

A FIELD REVIEW WILL BE MADE OF THE DEVELOPMENT'S ROADWAY DITCH LINE DURING AND AFTER CONSTRUCTION TO ENSURE THAT NO CONCENTRATED RUNOFF IS BEING CONVEYED ACROSS FILL MATERIAL. IF IT IS FOUND THAT CONCENTRATED RUNOFF IS BEING CONVEYED ACROSS FILL MATERIAL, THE DITCH LINE MUST BE LINED WITH RIP RAP TO PREVENT FUTURE EROSION SCOUR PROBLEMS.

GENERAL NOTES FOR SUBDIVISION PLANS

VDOT General Notes

- V1. All work on this project shall conform to the latest editions of the Virginia Department of Transportation (VDOT) Road and Bridge Specifications, and Standards, The Virginia Erosion and Sediment Control Regulations and any other state, federal or local regulations applicable. In the event of conflict between any of these standards, specifications or plans, the most stringent shall govern.
- All construction shall comply with the latest U.S. Department of Labor Occupational Safety & Health Administration and VOSH Rules & Regulations.
- When working on VDOT right or way, all traffic control, whether permanent or temporary, shall be in accordance with the current edition of VDOT's work area protection manual. Furthermore, all traffic control flaggers must be certified in accordance with VDOT's January, 1994 (or latest edition of) Road & Bridge Specifications, Section 104.04-C.
- Design features relating to construction or to regulation, control and safety of traffic may be subject to change as deemed necessary by VDOT
- Prior to initiation of work, Contractor shall be responsible for acquiring all necessary VDOT land use permits for any work on VDOT right of way.
- If required by the local VDOT Residency Office, a preconstruction conference must be arranged and held by the engineer and/or developer with the attendance of the contractor, various County agencies, utility companies and VDOT prior to initiation of work.
- Contractor shall notify the local VDOT Residency office when work is to begin or cease for any undetermined length of time. VDOT will also require 48 hours notice for any inspection.
- The Contractor will be responsible for maintaining adequate access to the project from the adjacent public roadway through construction and maintenance of a construction entrance in accordance with the Virginia Erosion & Sediment Control Handbook, Sec. 3.02. furthermore, access to other properties affected by this project shall be maintained through construction.
- Contractor shall ensure adequate drainage is achieved and maintained on the site during and at the end of construction.
- All water and sewer lines within existing or proposed VDOT right of way are to have minimum 36" cover and, to be installed under roadway drainage facilities.
- V11. Any unusual subsurface conditions encountered durina the course of construction shall be immediately brought to the attention of the engineer and VDOT. Work shall cease in that vicinity until an adequate design can be determined by the engineer and approved
- V12. All undercut areas and borrow material shall be Inspected and approved by VDOT Inspection prior to placement of fill.
- V13. All roadway fill, base, subsurface material and backfill of utility/storm sewer trenches shall be compacted in 6" lifts to 95% of theoretical maximum density as determined by ASSHTO T-99 Method A, within plus or minus 2% of optimum moisture for the full width of any dedicated street right-of-way. At the direction of VDOT Inspector density tests performed by a qualified independent agency shall be conducted as required in the VDOT Road and Bridge Specifications. A copy of all tests shall be submitted to VDOT prior to final VDOT approval.
- V14. VDOT Standard CD and UD underdrains shall be installed where indicated on these plans and further where determined necessary by VDOT Inspector.
- V15. The installation of any entrances and mailboxes within any dedicated street right-of-way shall meet VDOT minimum design standards and is the developer's responsibility.
- V16. If required by the local VDOT Residency Office, copies of all invoices for materials within any dedicated street right-of-way must be provided to the VDOT Inspector prior to acceptance of work. Unit and total prices may be obscured.
- V17. Prior to acceptance by VDOT of any streets, any required street signage and/or pavement markings must be installed by the developer or, at VDOT's discretion, by VDOT on an account receivable basis.

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DATE: 05-01-02

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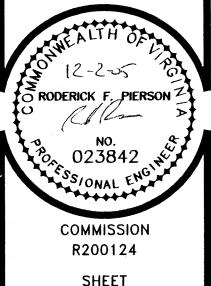
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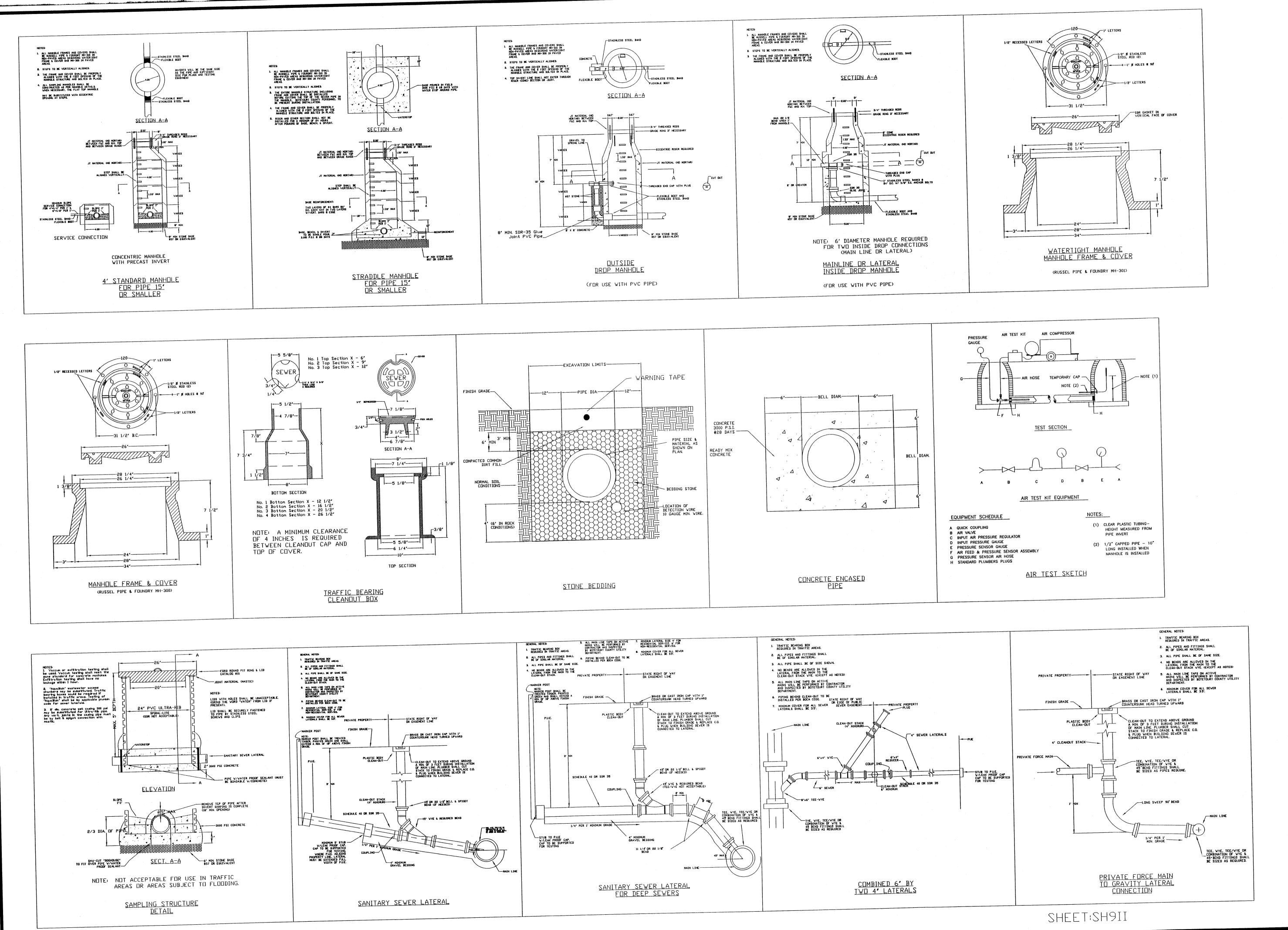
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ROADWAY **SPECIFICATIONS**



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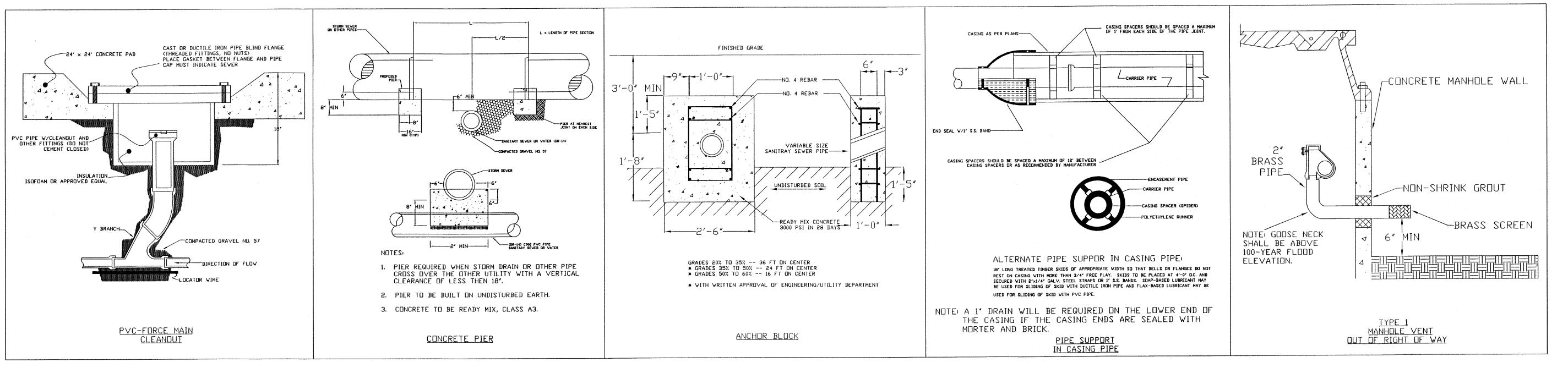
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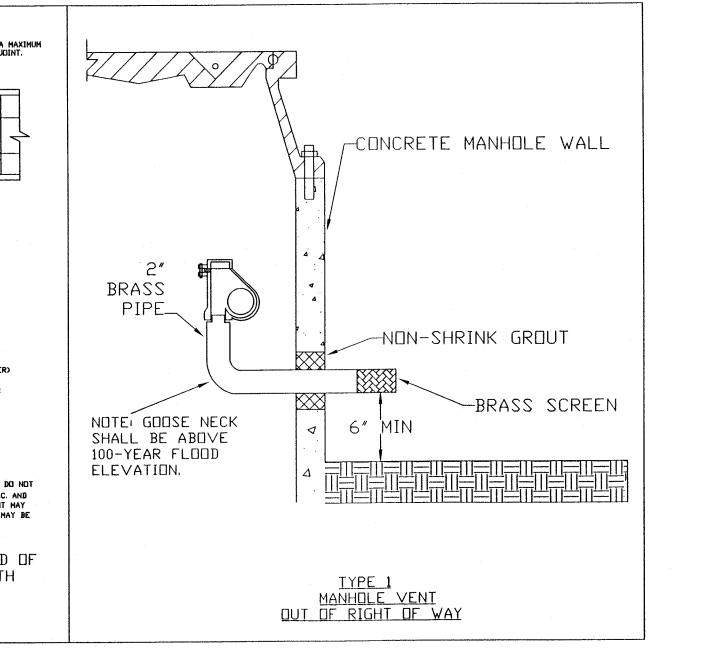
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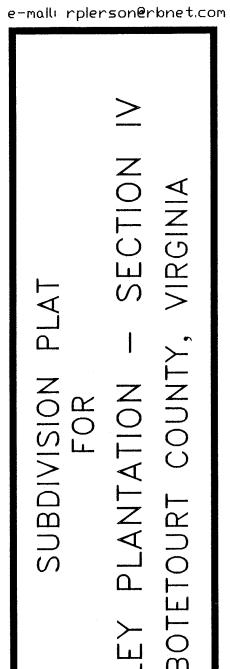
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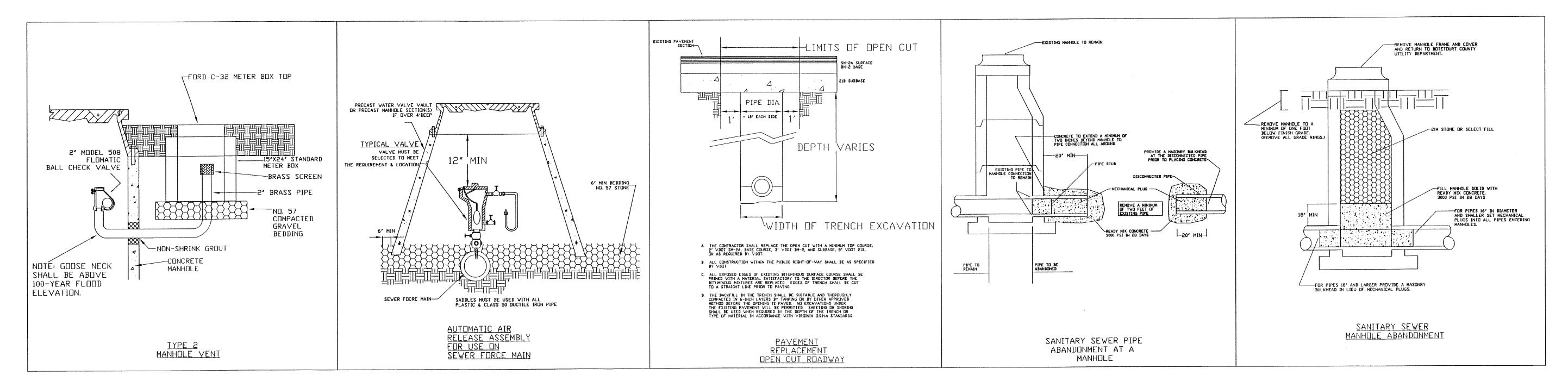
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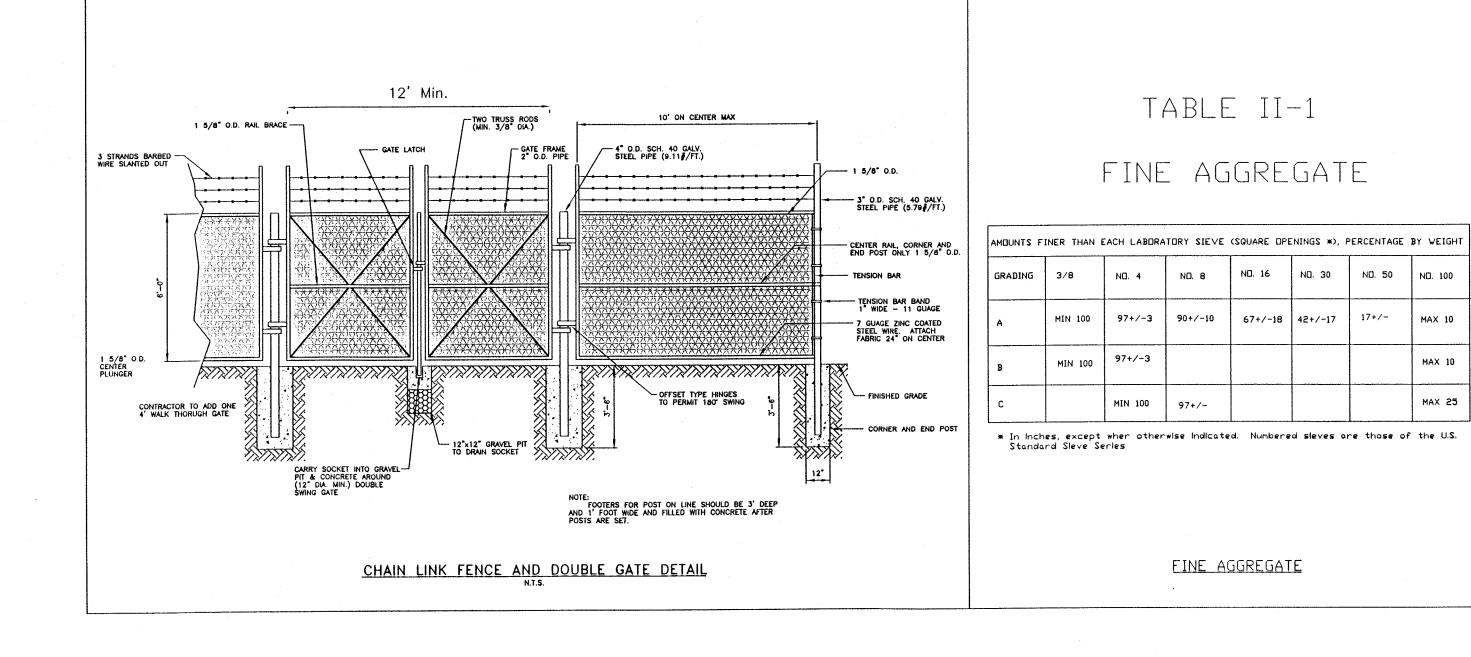
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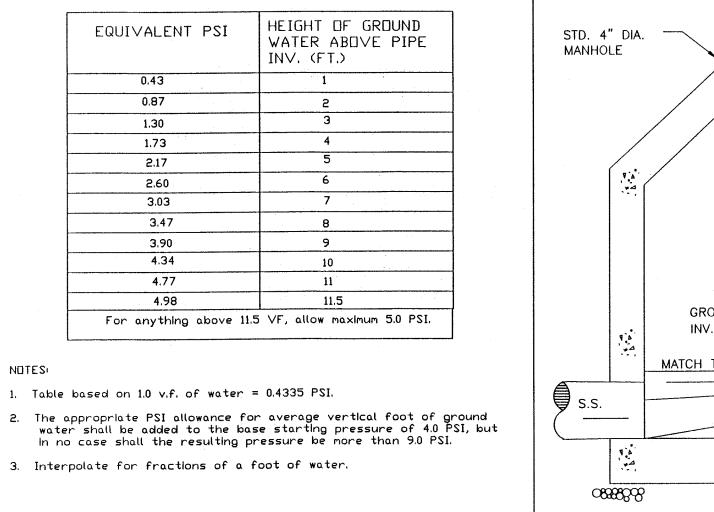
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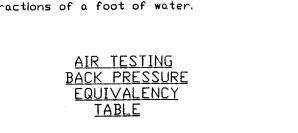
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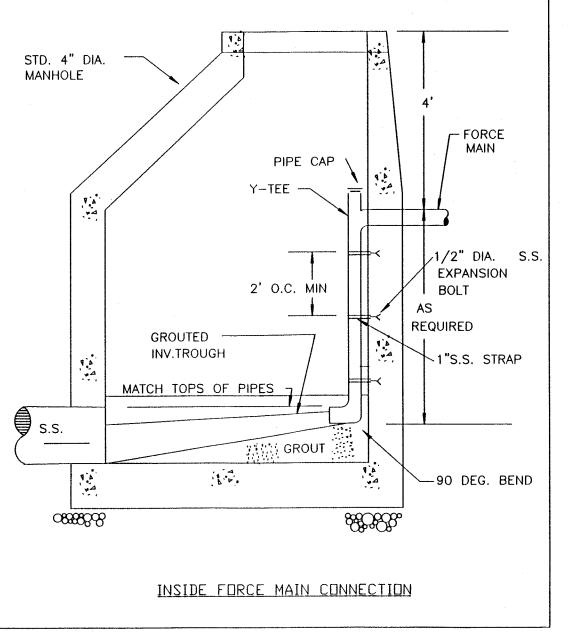
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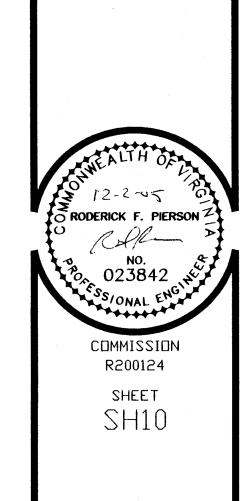




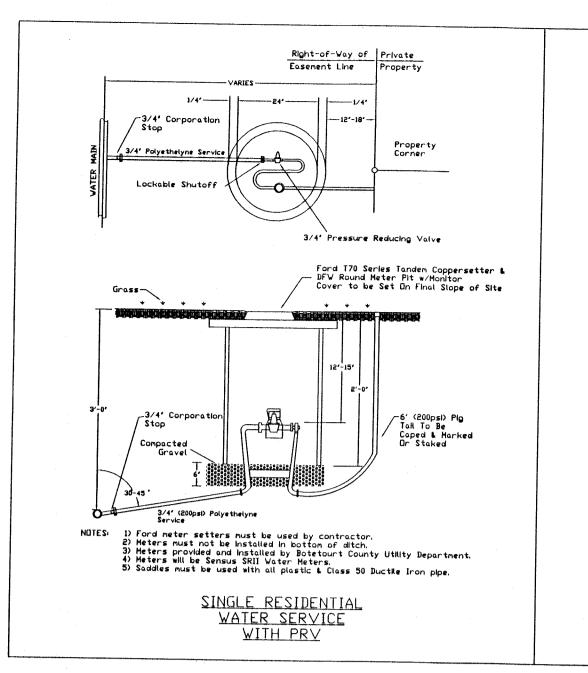


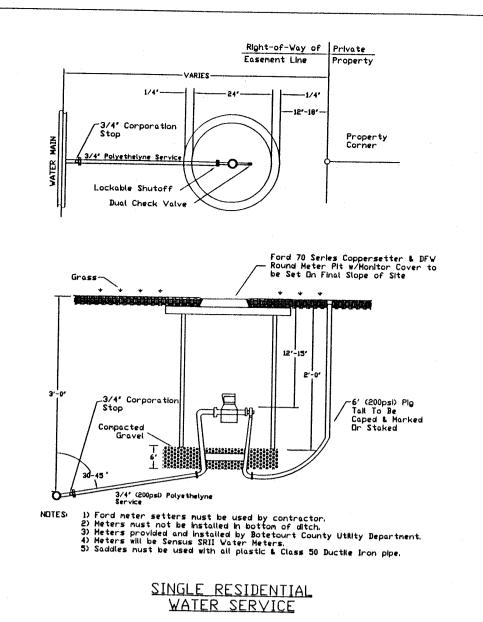
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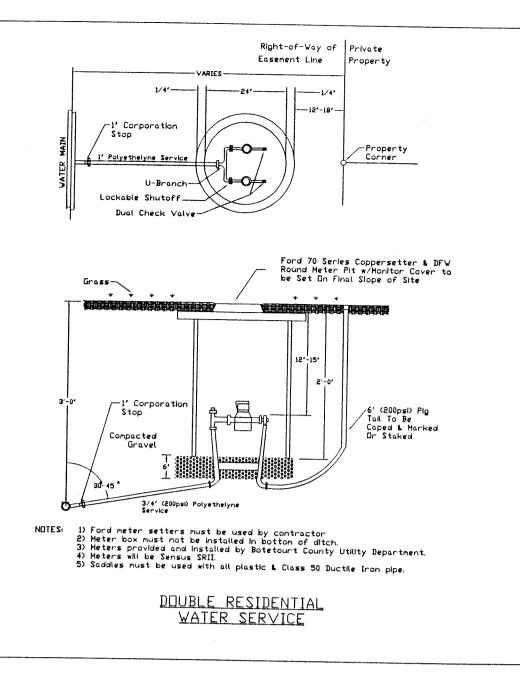
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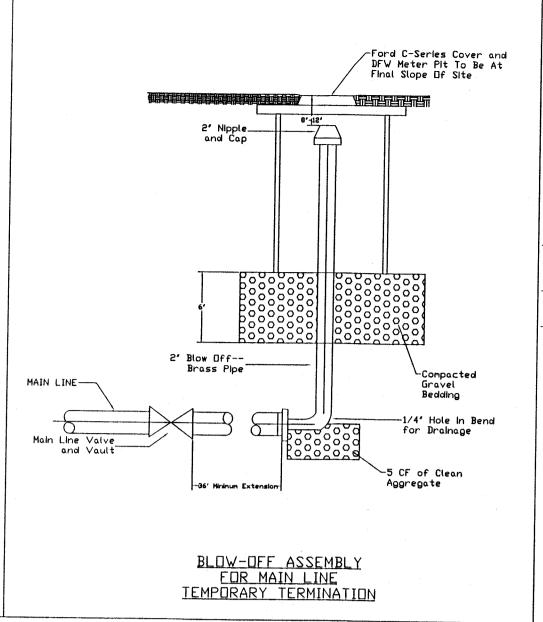


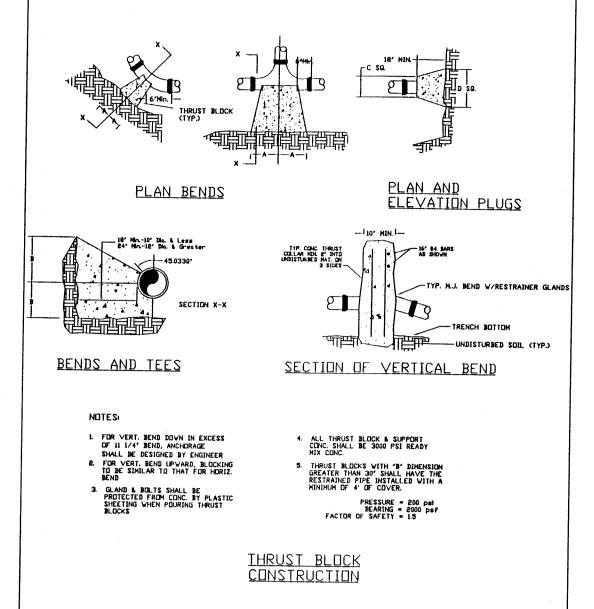
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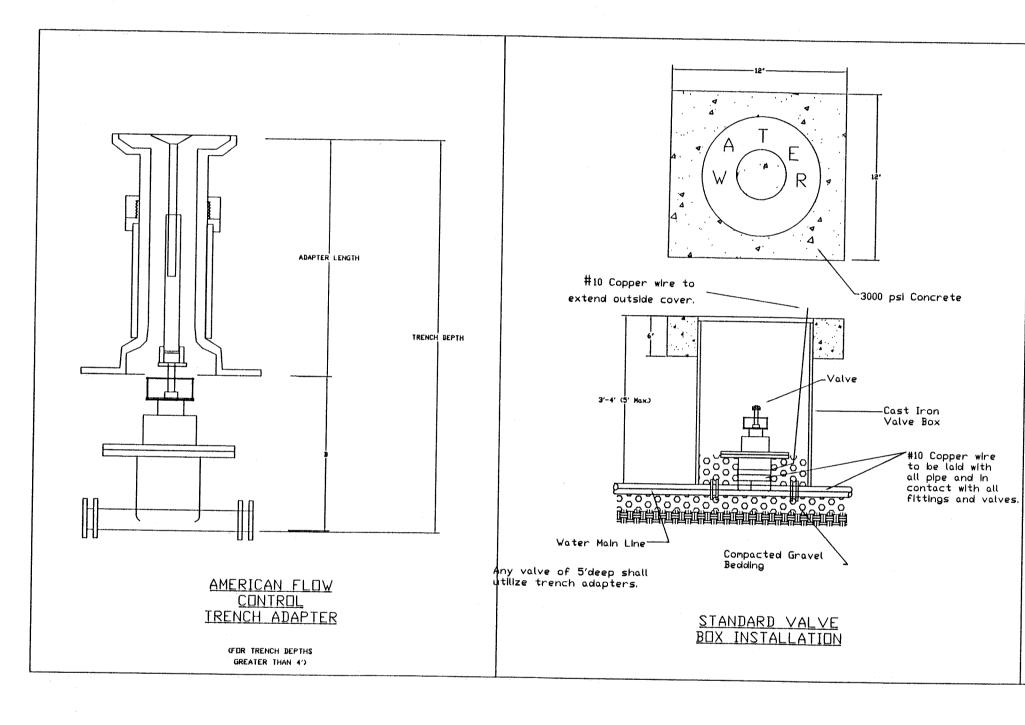
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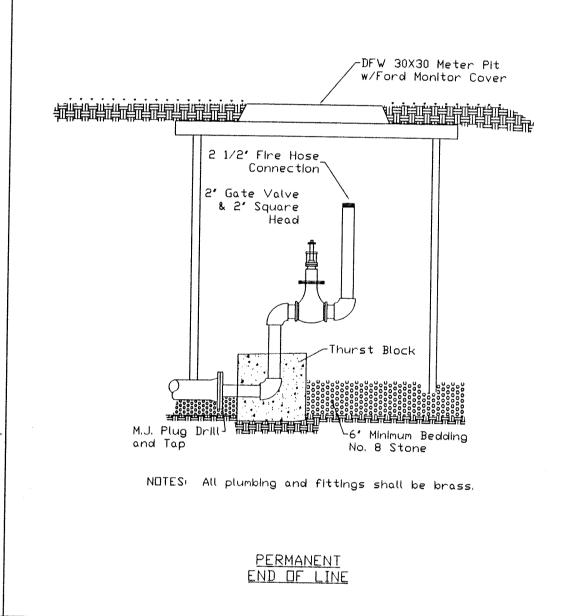
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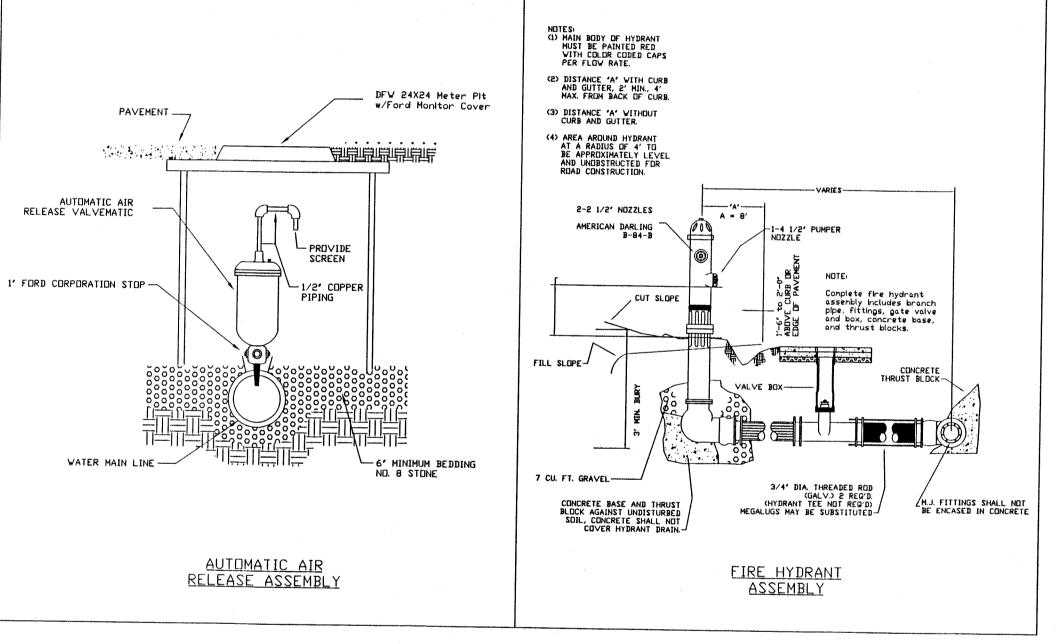
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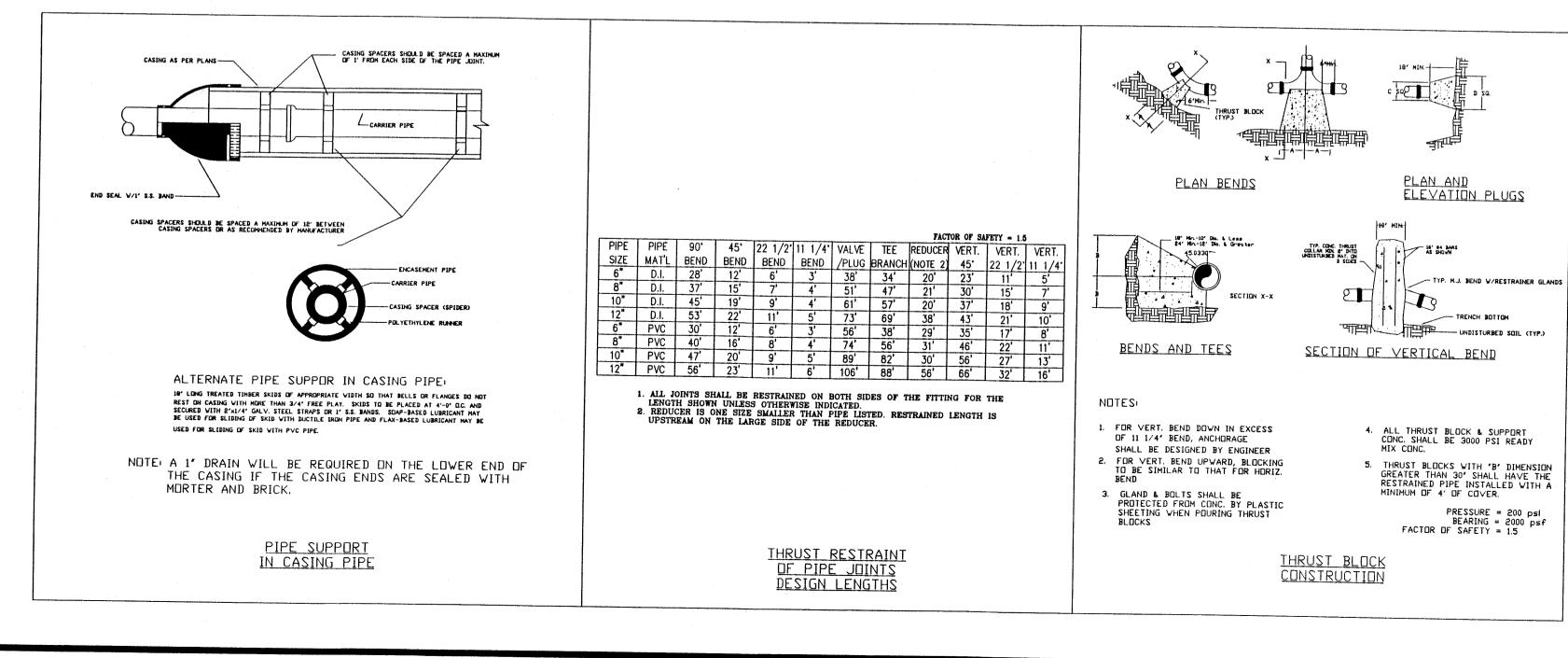
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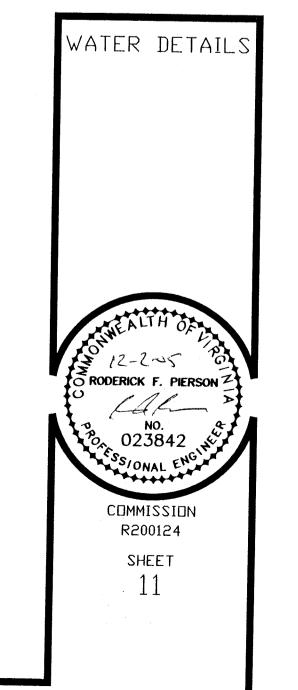
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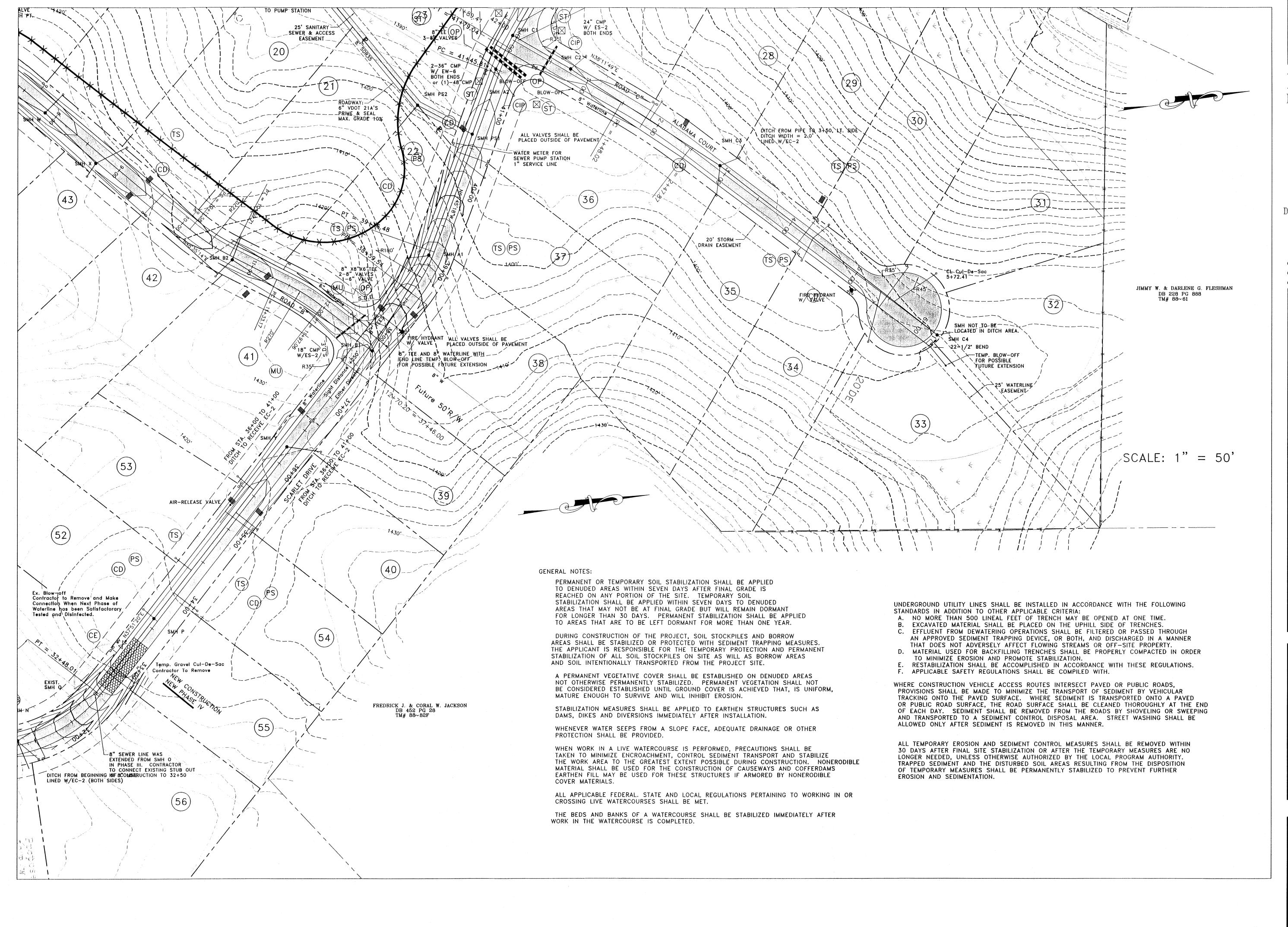












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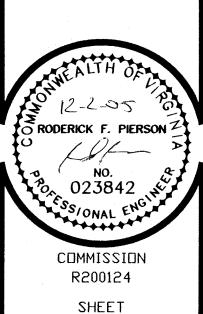
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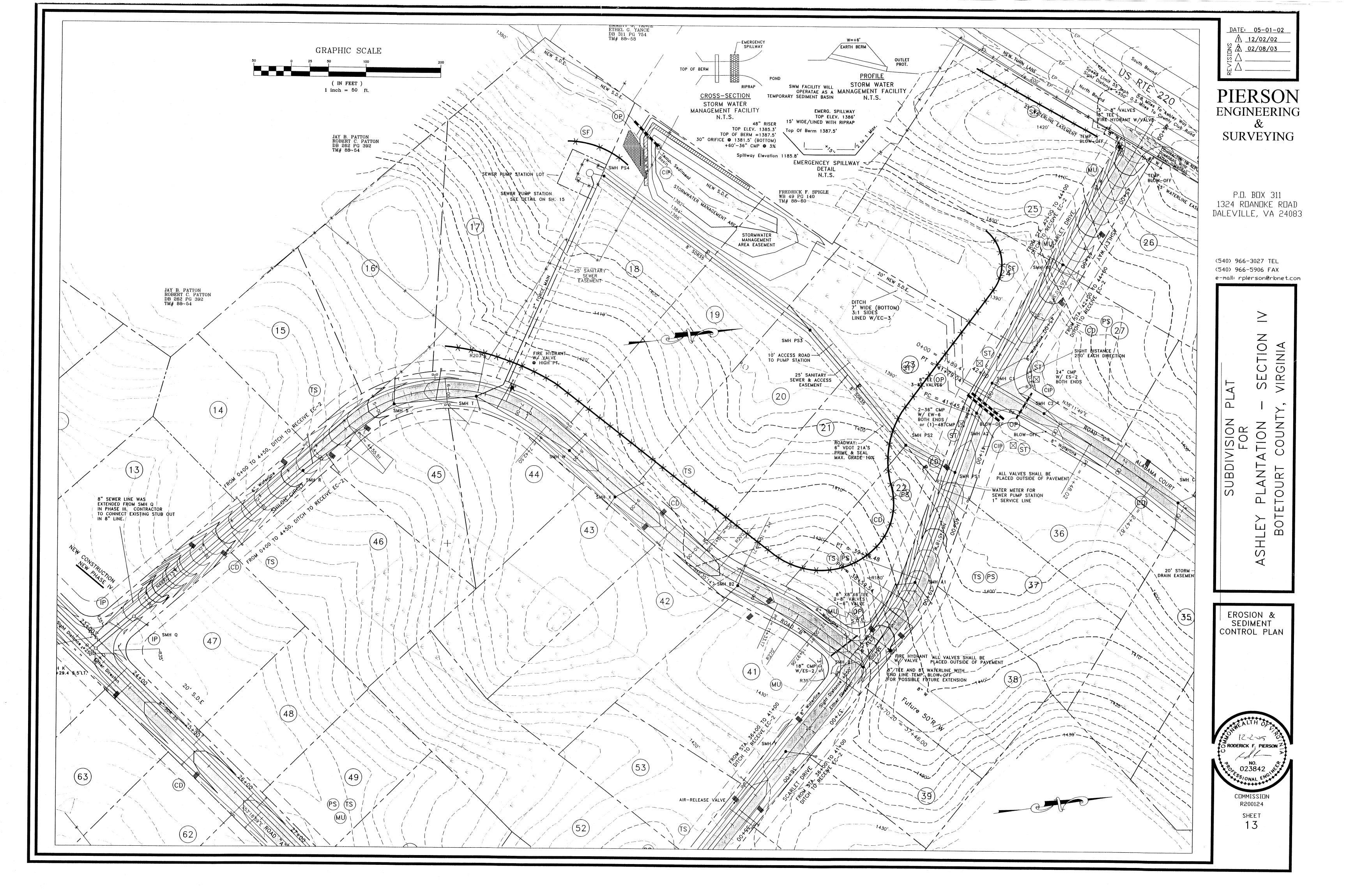
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SUBDIVISION PLAT
FOR
ASHLEY PLANTATION - SECTION IN
BOTETOURT COUNTY, VIRGINIA

EROSION &
SEDIMENT
CONTROL PLAN





Project Description

The project is located in the Greenfield area of Botetourt County, Virginia This plans shows the construction of roadways associated with a + /- 40 lot subdivision. The disturbed area consists of approximately 4.5 acres. It will take approximately 4 months, to grade the road and install the water, sewer, drainage utilites.

Existing Site Condition

Most of the site consists of sparse woods & fields. The terrain slopes in the 0.5% to 15% range.

Adjacent areas

The site is bordered on the north by US 220, Ashley Plantation, Section III to the south, and wooded areas to the east and west.

Soils found at this site are common to the area. Soils common to this area are Carbo, Chilhowie and Groseclose.

Critical Areas

Their are no critical erosion areas.

Erosion and Sediment Control Measures

Unless otherwise indicated, all vegetative and structural erosion and sediment control practices shall be constructed and maintained according to minimum standards and specifications of the handbook. The minimum standards of the VESCR shall be adhere to unless otherwise waived or approved by a variance.

Structural Practices

- a. Temporary Construction Entrance (Section 3.02); One temporary construction entrance will be installed.
- b. Silt Fence (Section 3.05)
- Temporary silt fence will be installed as indicated on the site plan.
- c. Culvert Inlet Protection (Section 3.08) Culvert Inlet Protection will be installed as indicated on the site plan.
- d. Outlet Protection (Section 3.18)
- Outlet Protection will be installed as indicated on the site plan.
- e. Temporary & Permanent Seeding (Section 3.31 & 3.32) Temporary seeding will be placed on all disturbed areas that will not be brought to final grade within 30 days or less. Temporary seeding will aid in the reduction of dust and sediment.

Temporary Seeding (Section 3.31)

Rates per acre: Winter - 40 lbs. annual rye and 40 lbs. cereale rye. Summer - 40 lbs. annual rye and 40 lbs foxtail millet. Fertilizer - 1500 lbs. of 10-18-10 per acre.

Lime — 2 tons per acre.

Permanent Seeding (Section 3.32) After final grading, permanent seeding will be employed to reduce erosion and sediment

Seeding Specifications:

Seasonal rates per acre: Feb. 1 to May 15

Aug. 1 to Sept. 15 100 lbs. tall fescue 100 lbs. tall fescue 15 lbs. annual rve 15 lbs. annual rye 2 lbs. red clover 2 lbs. red clover

Roderick F. Pierson, P.E., L.L.S.

1324 Roanoke Road, 220 North

May 16 to July 31 Sept. 16 to Jan. 31 120 lbs. tall fescue 10 lbs. cereale rye 10 lbs foxtail millet 2 lbs. red clover 2 lbs. red clover

Fertilizer - all seasons - 1500 lbs. of 10-18-10 per acre Lime - all seasons - 2 tons per acre

A mulch cover is required on every seeding: Straw at 80 bales per acre or an approved manufactured

All seeding, with required associated practices, will be in accordance with all applicable sections of the Virginia Erosion and Sediment Control Handbook.

PLATE. 3.14-2

SOURCE: VA. DSWC

g. Mulching (Section 3.35)

To prevent erosion by protecting the soil surface from raindrop impact and reducing velocity of overland flow. Areas which have been permanently seeded shall be mulched immediately following seeding. If fiber mulch is used as part of hydroseeding, no tacking is required. If straw is utilized as mulch, fiber mulch shall be utilized as anchor material and applied by means of a hydrospreader.

h. Dust Control (Section 3.39)

If arid condition prevail, dust control practices will be employed as required.

I. Construction Road Stabilization (Section 3.03) Temporary stabilization of roadways with stone immediatelly after grading.



Management

- a. Construction should be sequenced so that grading operations can begin and end as quickly as possible.
- b. Erosion and Sediment control devices shall be installed as the first step of construction. c. Areas which are not to be disturbed shall be clearly marked by flags, signs, etc.
- d. The grading contractor shall be responsible for the installation and maintenance of all erosion and sediment control practices. Inspections are to be made periodically and after every significant rainfall.

Maintenance

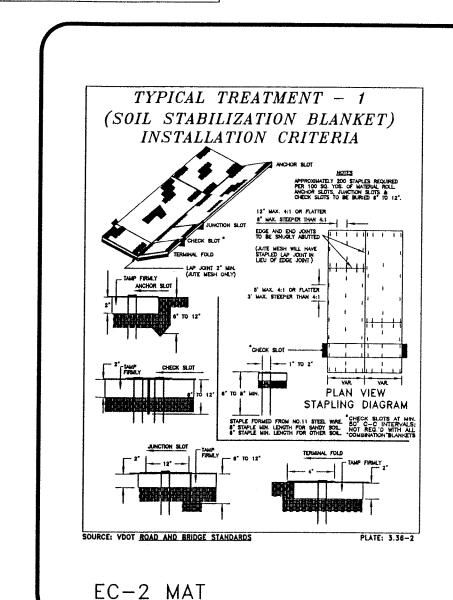
P.O. Box 311

540.966.3027

Daleville, VA 24083

In general, all erosion and sediment control measures will be checked daily and after each significant rainfall. Any Items not found in accordance with the Virginia Erosion and Sediment Control Handbook will be immediately replaced and/or repaired. The following items will be checked inn particular:

- The silt fence barrier will be checked regularly for undermining of deterioration of the fabric. Sediment shall be removed when the level of sediment deposition
- b. The seeded areas will be checked regularly to ensure that a good stand is maintained. Areas should be fertilized, limed and reseeded as needed.
- c. All graveled areas shall be checked for a uniform grade with no ruts or channels and top dressed with new gravel as required.



EC-2 TREATMENT ONE

Soil retention mats shall consist of a machine produced mat of wood fibers, coconut fibers, wood excelsior or manmade fiber that shall intertwine or interlock. Mats shall be of consistent thickness with fiber evenly distributed over its entire area and covered on the top and bottom side with netting having a web strength (not glued) but machine sewn. Staple and installation patterns shall be according to Bedford County

d. All erosion and sediment control measures shall remain in place until their removal has been approved by the Botetourt County Erosion and Sediment Control Administrator.

DESIGN ELEVATIONS WITH

DESIGN HIGH WATERN. 2.0' (25-YR. STORM ELEV.)

EMERGENCY SPILLWAY

MIN. 1.0'

DESIGN ELEVATIONS WITHOUT

EMERGENCY SPILLWAY

(RISER PASSES 25-YR. EVENT)

MIN. 3.0'

DEWATERING

If, during construction, the Erosion and Sediment Control reviewing Officers considers the Erosion and Sediment Items to be inadequate, he or she, at their discretion require the addition of other control measures.

General

The erosion and sediment control measures shown on the construction plans are the minimum measures required. Due to construction phasing and other considerations, all measures can not be shown. The owner, through his contractor will employ whatever measures which may be required to assure that sediment laden runoff does not leave the site.

All materials and measures employed for erosion and sediment control will be in accordance with the Virginia Erosion and Sediment Control Handbook, latest edition.

If during construction, additional Erosion and Sediment Control measures are deemed necessary, they shall be installed as directed by the Owner, Engineer or County agent.

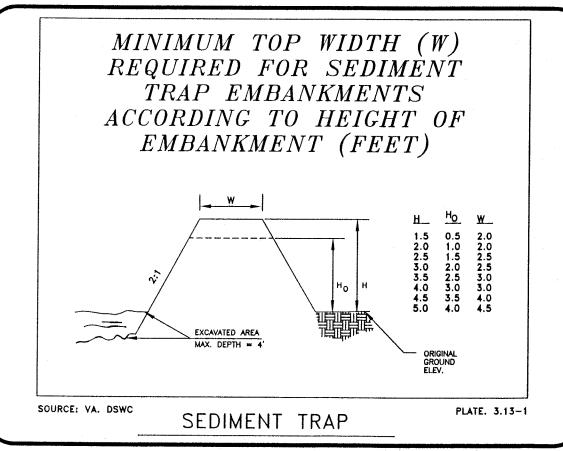
This project is to be constructed consistent with the Virginia Erosion and Sediment Control Regulations, (Latest Edition).

GENERAL NOTES:

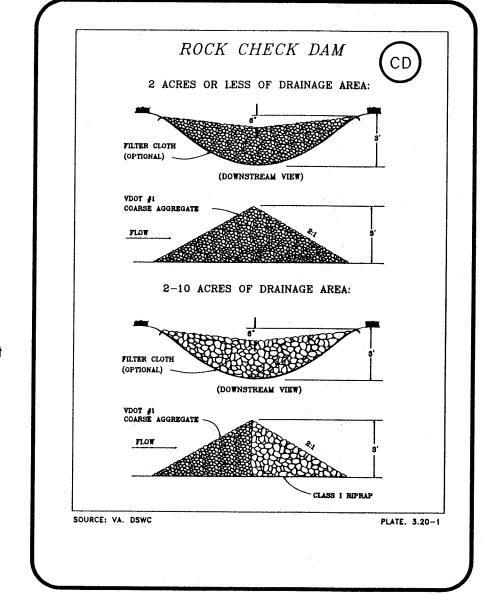
NO SEARCH OF UNDERGROUND UTILITIES WAS CONDUCTED.
CONTRACTOR SHALL CONTACT THE COUNTY OF BOTETOURT AND MISS UTILITY PRIOR TO WORK FOR LOCATION OF UNDERGROUND UTILITIES.

DATE DESCRIPTION 12/02/02 PER COUNTY AND VDOT COMMENTS 02/08/03 PER COUNTY AND VDOT COMMENTS

OUTLET PROTECTION TABLE: Use Tallwater < 0.5Do Diameter, Do Q/cfs Station of Pipe d/d50 d = ftLa 0+48 ROAD C 24" 9.5 0,51 0.8' 11′ 12+40 ROAD B 18" 3.0 0.5′ 0.81 6′ 41+60 ROAD A 2-36* 75 1.1' 29' 2.25 POND OUTLET 2-36* 85 1.2' 2,25' 30'

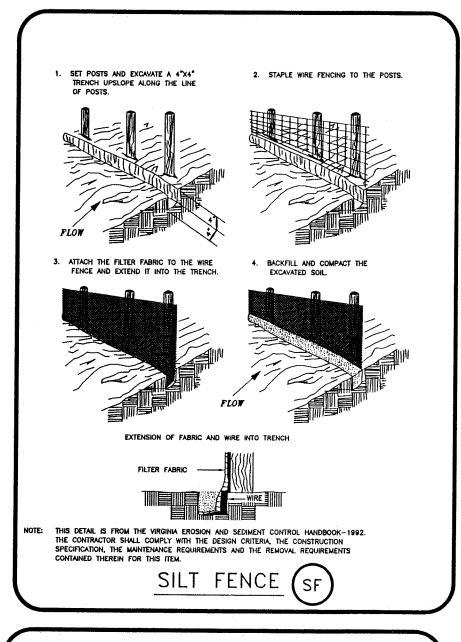


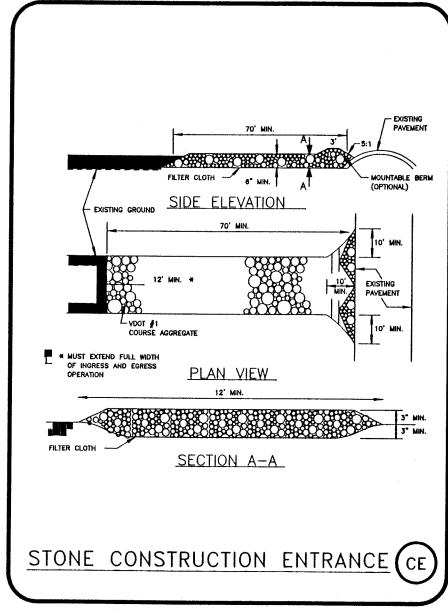
E & S QUANTITIES AND COST ESTIMATE				
ITEM	QUANTITY	UNIT PRICE	cost	
CONSTRUCTION ENTRANCE	1	\$700.00	\$700	
SILT FENCE	260 LF	\$1.75/LF \$455		
PERM. SEEDING	1.5 Ac.	\$2,100Ac.	\$3,150	
OUTLET PROTECTION	4	\$100	\$400	
INLET PROTECTION	3	\$100	\$300	
RIP RAP	100/TN	\$15/TN	\$ 1500	
EC-2 MAT	1000 LF	\$2.50LF	\$2,500	
DENTENTION POND	LS		\$5,000	
DENTENTION POND CLEANOUT	LS		\$2,000	
ROCK CHK. DAM	22	\$100EA	\$2200	
TEMP. SEEDING	1.0Ac.	\$1,200Ac.	\$1,200	
SEDIMENT TRAP	4	500/EA.	\$2,000	
CONST. ROAD STABILIZATION	120 TONS	\$15 TON	\$12,500	
SUBTOTAL			\$39,705	
20% CONTINGENCY			\$8,000	
TOTAL COST			\$47,705	

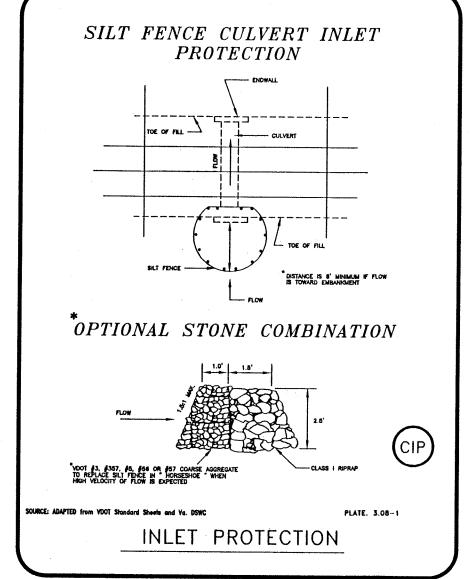


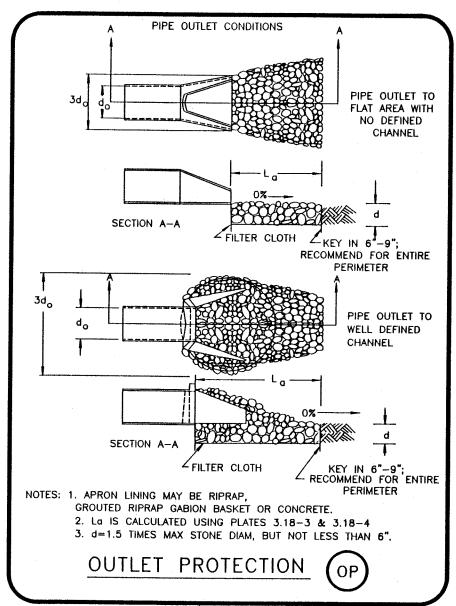
GENERAL EROSION AND SEDIMENT CONTROL NOTES

- Unless otherwise indicated, all vegetative and structural erosion and sediment control practices will be constructed and maintained according to minimum standards and specifications of the Virginia Erosion and Sediment Control Handbook and Virginia Regulations VR 625-02-00 Erosion and Sediment Control Regulations.
- The plan approving authority must be notified one week prior to the preconstruction conference, one week prior to the commencement of land disturbing activity, and one week prior to the final inspection.
- All erosion and sediment control measures are to be placed to or as the first step in clearing.
- A copy of the approved erosion and sediment control plan & narrative, as well as a copy of the Land Disturbing Permit, shall be maintained on the site at all times. The Erosion and Sediment Control Administrator will deliver these materials at the onsite preconstruction conference.
- Prior to commencing land disturbing activities in areas to her than indicated on these plans (including, but not limited to, off-site borrow or waste areas), the contractor shall submit supplementary erosion control plan to the owner for review and approval by the plan approving authority.
- The contractor is responsible for installation of any additional erosion control measures necessary to prevent erosion and sedimentation as determined by the plan approving authority.
- All disturbed areas are to drain to approved sediment control measures at all times during land disturbing activities and during site development until final stabilization is
- During any dewatering operations, water will be pumped into an approved filtering
 - The contractor shall inspect all erosion control measures periodically and after each runoff-producing rainfall event. Any necessary repairs or cleanup to maintain the effectiveness of the erosion control devices shall be made immediately. An inspections report must be filed with the Botetourt County Erosion and Sediment Administrator once every two weeks, beginning with commencement of the land disturbing activity, and with 48 hours of any runoff-producing rainfall event. Failure to submit a report will be grounds for immediate revocation of the Land Disturbing Permit. Reports must be postmarked within 24 hours of the deadline. A standard inspection report form will be supplied, which should be copied as necessary. This provision in no way waives the right of Botetourt County personnel to conduct site inspections, nor does it deny the right of the permittee(s) to accompany the inspector(s).

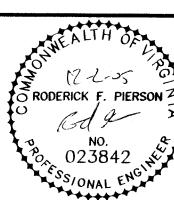








E & S DETAIL SHEET ASHLEY PLANTATION - SECTION IV BOTETOURT COUNTY, VIRGINIA

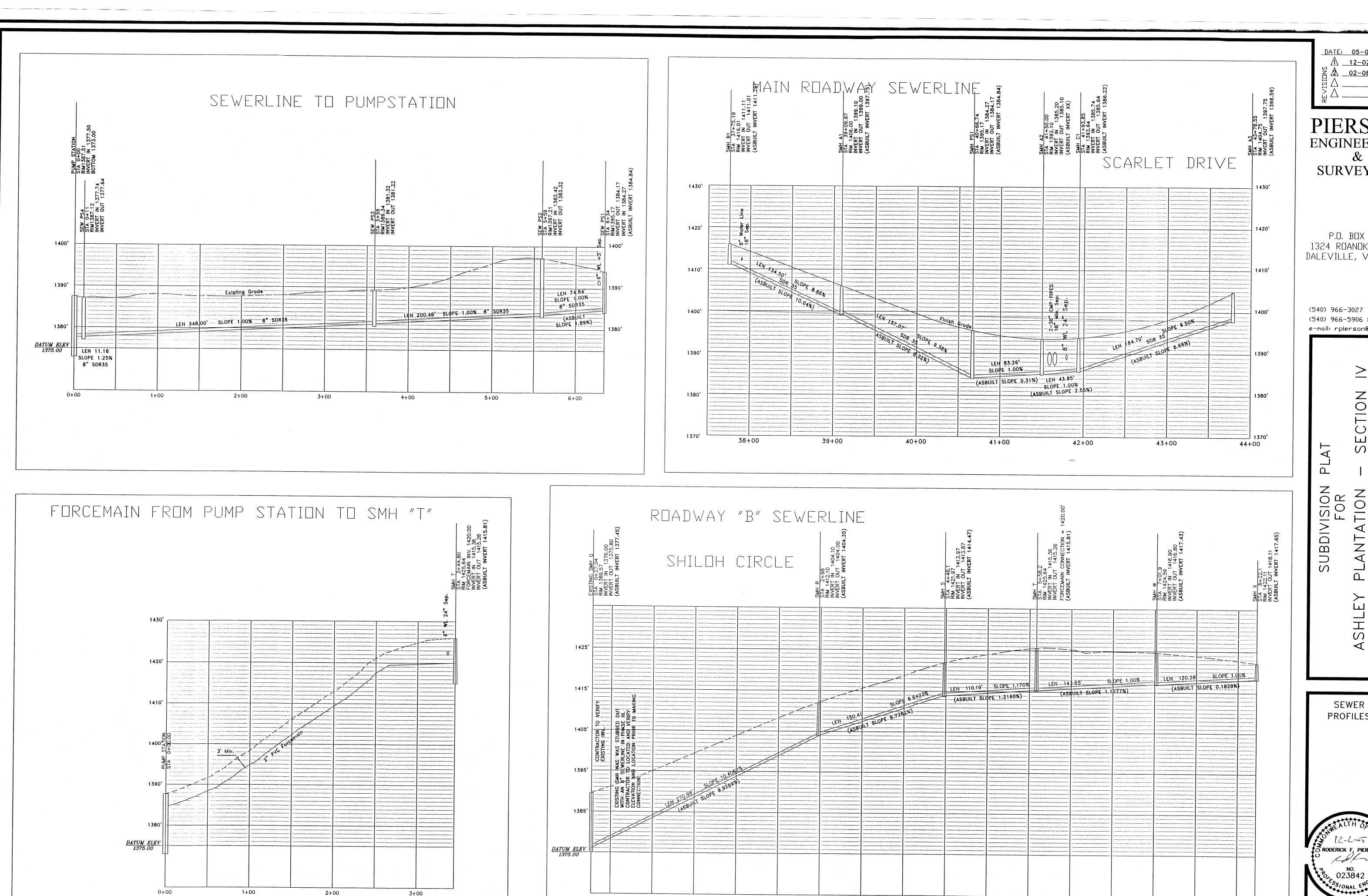


SCALE: -COMM R200124 DATE: 05/01/02

SHEET

ASHLEYPLANTATION/SH12IIES

14



1+00

2+00

3+00

DATE: 05-01-02 12-02-02

PIERSON ENGINEERING SURVEYING

P.O. BOX 311 1324 ROANOKE ROAD DALEVILLE, VA 24083

(540) 966-3027 TEL

(540) 966-5906 FAX e-mall: rplerson@rbnet.com

SECTION SUBDIVISION FOR PLANTATION ETOURT COUNT

> SEWER **PROFILES**

COMMISSION R200124 SHEET SH15

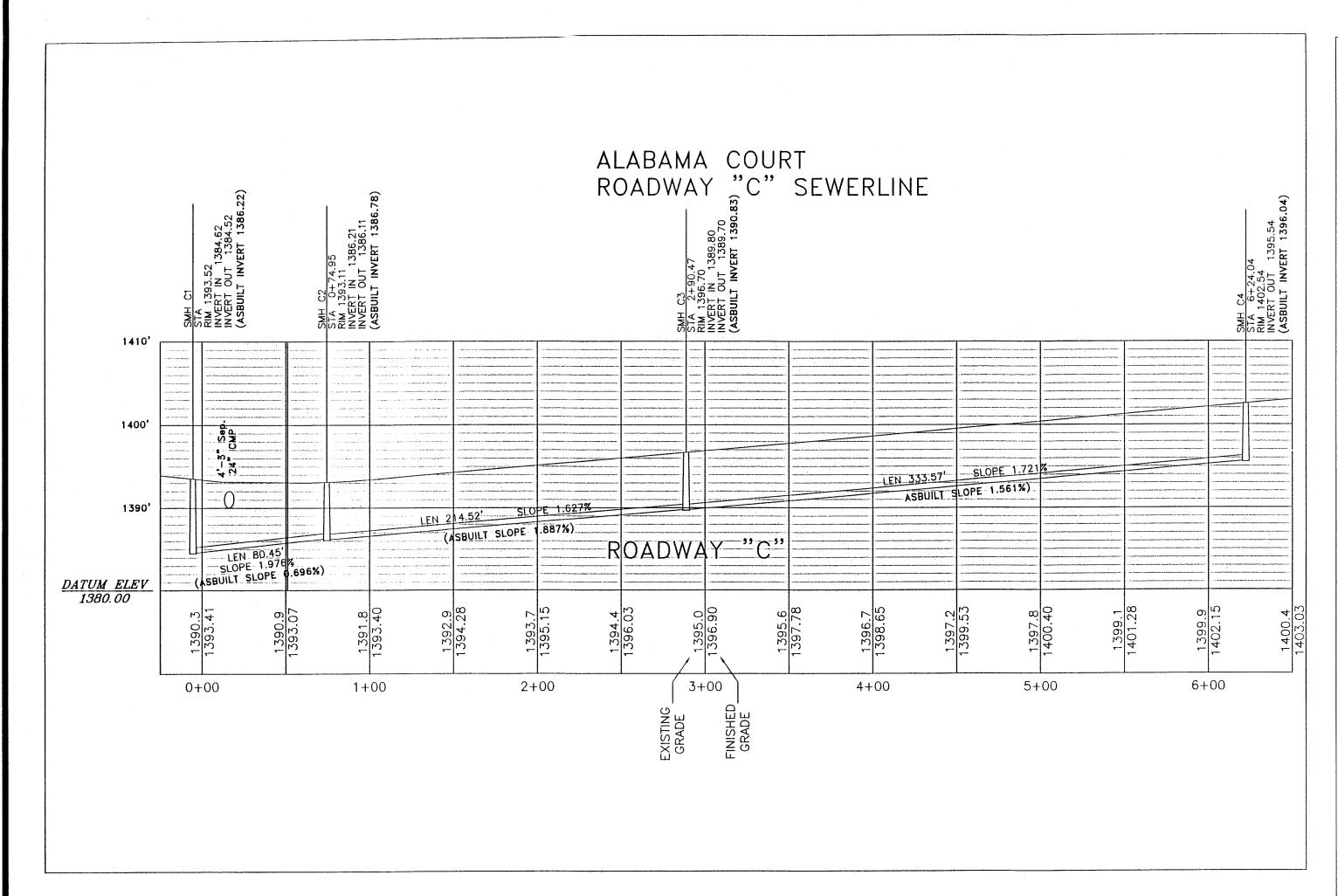
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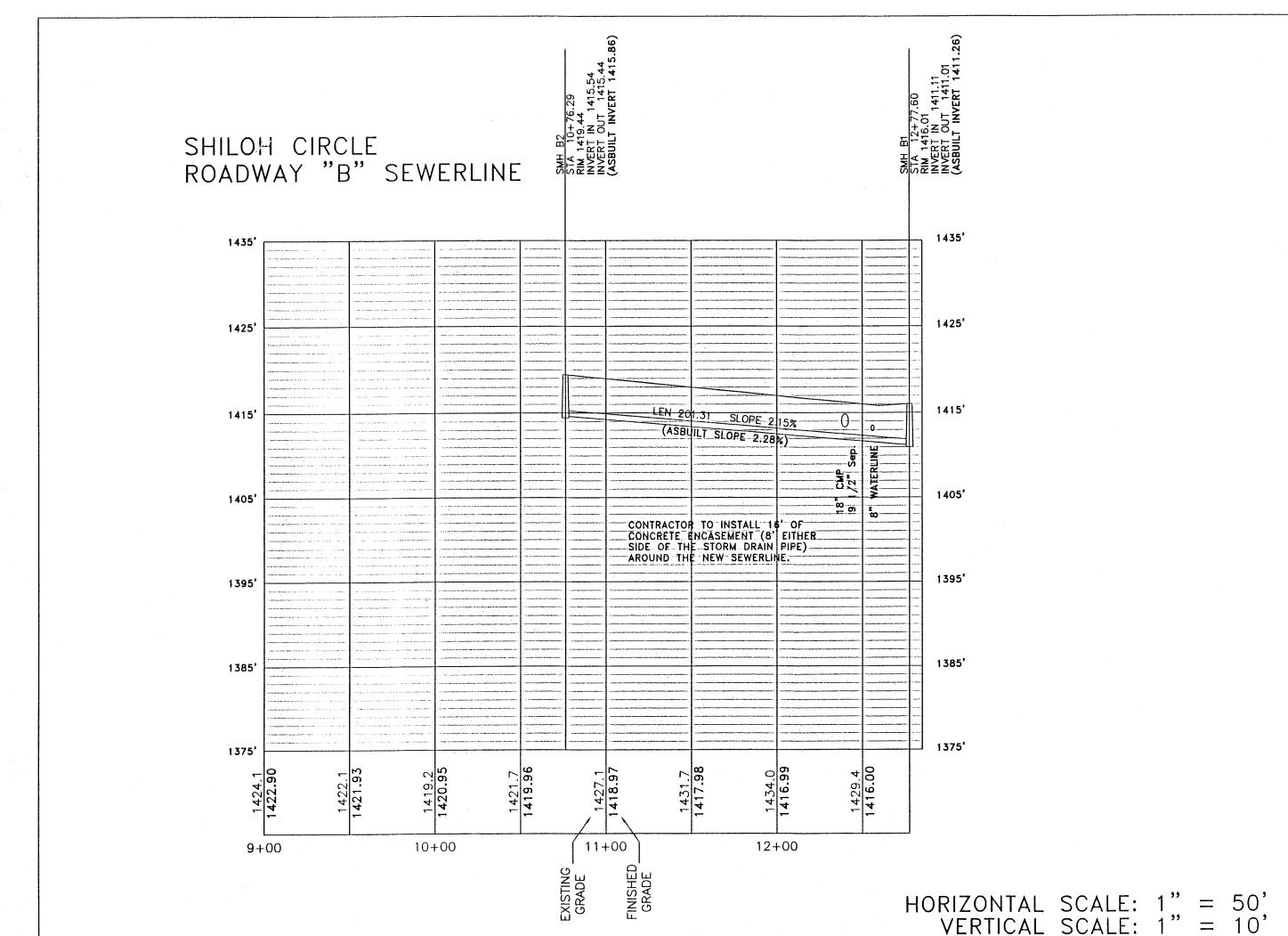
5+00

6+00

HORIZONTAL SCALE: 1" = 50' VERTICAL SCALE: 1" = 10'

8+00





SANITARY SEWER TESTING

Manhole Acceptance Tests (Vacuum Testing)

Manholes, including frame, shall be tested by vacuum testing from the top of the frame. Inflatable stoppers shall be used to plug all lines into and out of the manhole being tested including any vent line. The stoppers shall be positioned in the lines far enough from the manhole to insure testing to those portions of the lines not air tested. Vacuum tests shall be made with a vacuum of 10" Hg. The time for the vacuum to drop from 10" to 9" of Hg must be greater than 60 seconds.

Contractor shall furnish weirs, stand pipes, pipe plugs, water, pressure gauges, stop watches, air compressor, vacuum pump, hose and such materials and assistance as required to perform these tests. All acceptance tests shall be conducted by Contractor in the presence of a County

Acceptance tests shall not be made until sanitary sewer, manholes and proposed sewer service connections, as shown on the approved sewer plans, have been installed, the sewer trenches (including manholes and cleanout stacks) backfilled and compacted to finish sub-grade.

Contractor shall schedule all acceptance tests with the project inspector at least forty-eight (48) hours in advance. Each section of completed sewer shall be tested from manhole to manhole. No sewers or sewer service connections are to be excluded from this testing procedure.

Sewer Pipe Testing Procedure

Whenever it is necessary to construct underdrains or place gravel under pipe lines in order to dewater trench during construction of sewers, acceptance test will not be made until any pumps, which have been used in dewatering process, have been disconnected of drains have been taken out

Contractor shall schedule all acceptance tests with the Engineering/Utility Department at least forty-eight (48) hours in advance. Each section of completed sewer shall be tested. Generally, sewers will be tested from manhole to manhole. No sewer or sewer service connection is to be excluded from this testing procedure.

Low Pressure Air Testing Procedure - The test procedure shall be conducted in the following manner: (Vacuum test of manholes is generally inverse of low pressure air test of sewer lines)

- a. Contractor shall thoroughly clean and remove all debris, silt, earth or other materials from the sewer prior to acceptance testing.
- b. Proper test plugs shall be supplied and installed by Contractor Test gauges used in air test procedure shall have a range of 0-10 psi and shall be calibrated in divisions of 0.10 psi with an accuracy of +/- one percent. Test gauges shall be calibrated at least once a year and the date and results displayed on the equipment including date of calibration. Calibrations shall be certified by an independent testing lab. Test gauges shall be located outside of manhole during testing.
- c. If pipe to be tested is expected to be below ground water table, Contractor shall either:
- Install a small diameter perforated vertical pipe from invert elevation of the sewer to the surface prior to backfilling; or
- Insert a pipe probe by boring or driving into the backfilling material adjacent to the invert elevation of the pipe, and determine the depth of the ground water level above the pipe invert immediately prior to acceptance testing the sewer. - All gauge pressures for test shall be increased by the amount of this back pressure due to ground water over the invert of the
- In lieu of the above water depth determination, Contractor may add three (3) psi to the gauge pressure in the test.
- d. Contractor shall add air slowly to the portion of the pipe under test until the internal air pressure is raised to 4.0 psi gauge plus the ground water pressure.
- e. As a safety precaution, <u>no one shall be allowed in manhole after air</u> pressure is increased in the sewer line. If the inspector suspects that the test plug may be leaking, pressure shall first be relieved before any adjustments are made to eliminate air leakage at the
- f. Contractor shall allow air temperature to stabilize for at least two (2) minutes with the pipe subjected to an internal pressure of 4.0 psi by adding only the amount of air required to maintain the pressure.
- After temperature stabilization, the test will begin. If the internal air pressure decreases, the time required for the pressure to drop from 3.5 to 2.5 psi gauge will be observed and recorded. The time interval shall be compared with the standards in accordance with the DD-30 or DD-31 for time and length of test section for various diameters of the sewer. All pipes 15 inches or less shall be tested for a pressure drop of 1.0 psi gauge.
- h. Pipe which fails to maintain the stipulated pressure for a period equal to or greater than the holding time shown in Table I shall be deemed to have failed the low pressure air test and is unsatisfactory for acceptance by the County. Any sewer the fails to pass this test shall be replaced by the Contractor at his expense.

Sewer Force Main Testing Procedure — Sewer force mains shall be hydrostatically tested at 150% of the design operating pressure of (60 psi) for 30 minutes. Allowable leakage shall be the same as established for water pipe lines in the Botetourt County Water and Sewer Stabdard and Specifications.

Mandrel Testing

The Contractor shall deflection test the entire length of pipe by means of a go-no-go mandrel to assure that a 5.0% deflection has not been exceeded. The testing shall be performed completely at the expense of the Contractor and shall be performed in the presence of Engineer. Mandrel and proving ring details shall be approved by the Engineer and shall be sized at 5% less than ASTM dimensions for the sewer pipe (In accordance with ASTM D-3034 and F-679). The mandrel test shall be performed no sooner than three (3) months after backfill of the pipe is completed. All pipe that fails the deflection test shall be removed and replaced at the Contractor's expense. The "rerounder" technique shall not be allowed. The Contractor shall use approved nine (9) arm mandrels and proving rings for each size of mainline pipe. The contract length "L" of the mandrel arms and the actual mandrel diameter "D" (ID of the ring) shall equal the dimensions in Table I below. Critical mandrel dimensions shall carry a tolerance of +/-.01.

		TABLE		
		9 Arm Mandi	rel	
D Dimension				
	(MIN)	ASTM D3034	ASTM D2751 (6" only)*	
Nom. Dia.	L	SDR 35	ASTM D2680	
6"	6"	5.65"	5.65" (SDR 35) 5.49" (SDR 23.5)	
8"	8"	7.56"	7.40"	
10"	10"	9.45"	9.31"	
12"	12"	11.26"	11.22"	
15"	15"	13.78"	14.09"	
18"	18"	16.69"		
21"	18"	19.67"		
24"	18"	22.13"		
27"	18"	24.95"		

Mandrel and proving ring may be obtained from Wortco, Inc., 220 High Street, Franklin, Ohio 45005 (1-513-746-6439), or Hurco Enterprises (1-800-843-1300), or Cherne industries (1-800-843-7584).

All tests are to be performed in the presence of the design engineer and properly documented by the design engineer for submittal with record drawings to Botetourt County prior to conveyance to Botefourt County. Tests submittals documented by anyone other than the design engineer (ie. the contractor) will not be acceptable as proof of compliance.

Developer to provide video camera inspection documentation prior to conclusion 1—year warrantly period. Video camera inspection work to be performed no sooner the 6 months after the date of system acceptance by Botetourt County and no later than 8 months after the date of system acceptance by Botetourt County. One (1) copy of the videotape to be provided to Botetourt County. Video camera inspection work to be coordinated with Botetourt County such that County personnel can be present during video inspection operations.

The contractor shall schedule a pre-construction meeting to be attended by no less than the following: Botetourt County representatives, contractor representative including the proposed sewer contractor site superintendent, design engineer, any meterial suppliers or subcontractors that the sewer contractor feels necesary to attend.

All sewer and water pipes (mains and service lines) shall have both magnetically locatable detection wire and or tape AND warning tape. Magnetically locatable detection wire and or tape shall be installed at same elevation of spring line of pipe. Warning tape (Caution!! Buried Sewer / Water Pipe Below) to be installed no more than 18 inches above top of the pipe.

All water and sewer shop drawings / cut sheets shall be submitted by the contractor to both the design engineer and Botetourt County for approval prior to Installation. Botetourt County will require three (3) complete sets of shop drawings for review.

All new water mains shall be tested, after backfilling to a hydrostatic pressure of not less than 100 psi above design water pressure for the system or 150 psi. whichever is greater. Allowable leakage shall be calculated by the following

 $L = \underline{SDP}^{1/2}$ 133,200

vessel of known volume.

Where: L = allowable leakage in gallons per hour S = length of pipe tested in feet. D = nominal diameter of pipe in inches. P = average test pressure during leakage test in psi.

No water line shall be placed in service until the leakage is less than the allowable leakage as indicated above. Testing of water mains shall only be done after installation of all valves, taps and service laterals are complete. All portions of the water system, including hydrants and service lines, shall be subject to the hydrostatic pressure during the leakage test. Testing of water mains shall be observed and documented by a County Utility or Engineering Inspector.

All high points and service lines in the portion of the system under test shall be vented and all air shall be expelled from the system prior to beginning the test. All fittings and hydrants shall be property braced or blocked before applying pressure. Where concrete thrust blocks are used, they shall have attained their

set prior to testina. After the portion of the system under the test has reached the required pressure as stated herein, said pressure shall be maintained for two (2) hours. At the conclusion of the pressure test, the volume of makeup water required to refill the pipeline shall be determined by measurement with a displacement meter or by pumping from a

All joints or fittings at which leakage occurs shall be re-worked to insure tightness. All visible leaks shall be repaired regardless of amount of leakage. If the measured amount of leakage exceeds the valves for the appropriate size as found in AWWA Specifications C600, Hydrostatic Testing (Table 6), the pipe-line shall be repaired prior to re-testing will be done the Utility or Engineering Departments approval and inspection. Repairs of new construction will be by adjustment or replacement of replacement of material only. The use of repairs clamps or bell clamps will not be acceptable.

DATE: 05-01-02

PIERSON ENGINEERING &**SURVEYING**

P.O. BOX 311 1324 ROANOKE ROAD DALEVILLE, VA 24083

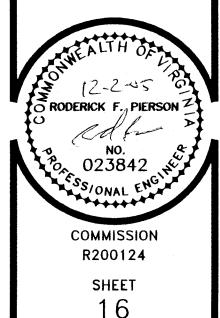
(540) 966-3027 TEL (540) 966-5906 FAX e-malli rplerson@rbnet.com

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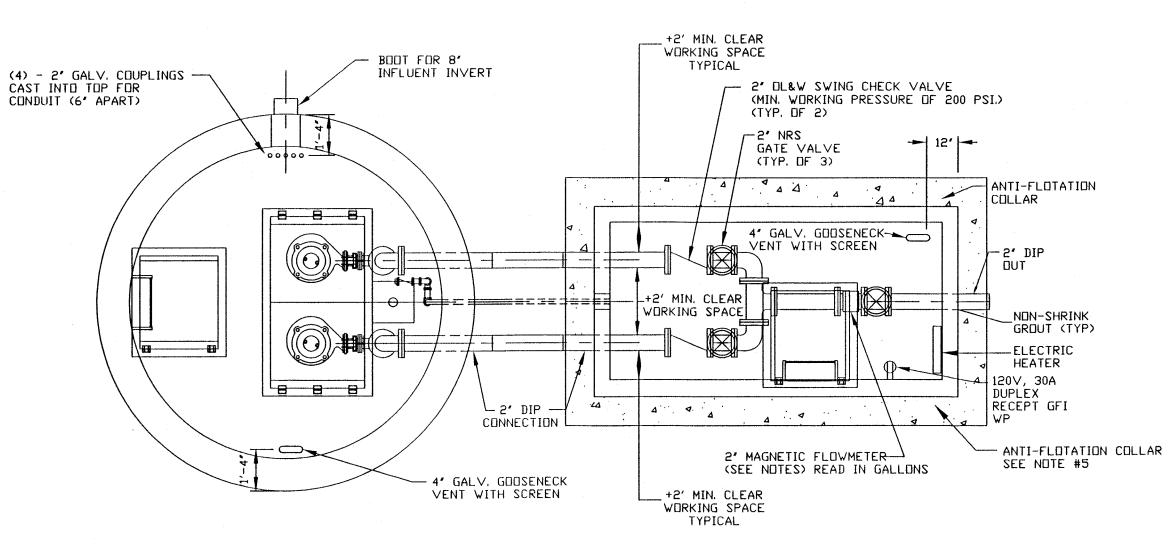
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SEWER **PROFILES**



AS-BUILT



PLAN VIEW

RATED ALUM. ACCESS

PENETRATION

NDN-SKRINK

HATCH W/PADLOCK

2' SCH. 40 PVC DRAIN WITH P-TRAP.

- DISCHARGED PIPE BASE PLATE ANCHORED

- SEE NOTE #3

TO BOTTOM OF WET WELL WITH

STAINLESS STEEL BOLTS.

CONNECTION BETWEEN

WET WELL & VALVE

POST (H1R-3030)

DESIGN CRITERIA

- ALUMINUM HANDRAIL

(ON 3 SIDES ONLY)
TYPICAL

ACCESS LADDER

-- STEEL PIPE SUPPORT

ALL SUCTION AND DISCHARGE PIPING SHALL BE

DUCTILE IRON FLANGED PIPE DESIGNED AND MANUFACTURED PER AWWA SPECIFICATIONS

(TYP. OF 4)

GENERAL NOTE:

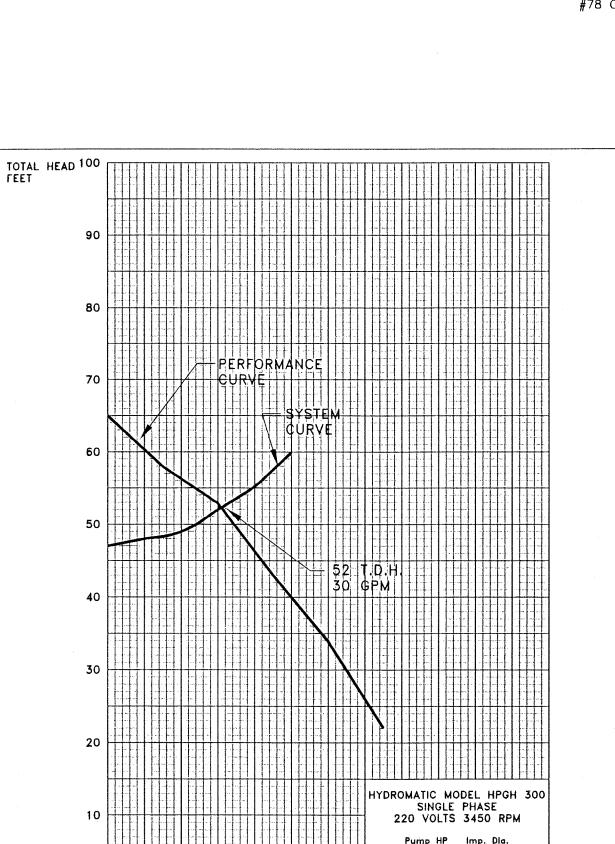
C150 AND C151.

CONCRETE ALL WAY

ARDUND TANK

- APPROX. # OF LOTS TO BE SERVED 20 - AVERAGE DAILY FLOW - 8.3 GPM - PEAK DAILY FLOW - 33 GPM
- PUMP RATED CAPACITY 30 GPM @ 55' TDH - 25YR STORM ELEVATION - 1347.42
- 100YR STORM ELEVATION 1347.84

- CONTRACTOR TO VERIFY ADEQUATE TURN AROUND FOR SERVICE VEHICLES SITE LIGHT 150W HIGH ----PRESSURE SODIUM MIN. 20' / ABOVE FINISHED GRADE WITH POLE MOUNTED DISCONNECT SWITCH /STANDBY 250 gal. LP Tank GENERATOR Contractor To Coordinate With Gas Company To 12' Wide Double Gate (Min.) Verify Size & Loc. SMH PS4 6' HIGH CHAIN-LINK FENCE W/ 3 STRANDS OF BARBED WIRE CONTROL PANEL-SHELTER ✓ FREEZE PROOF YARD HYDRANT W BACK-FLOW PREVENTOR SITE TO HAVE 1" WATER SERVICE LINE PUMP STATION SITE DETAIL



Electrical Controls:

(1) Control Sequence - On rising liquid level in the wet well, a mercury type float switch shall initiate operation of the lead pump at the elevation indicated on the DRAWINGS. Should the liquid level continue to rise. a second mercury float switch would initiate operation of the Lag Pump. The pump(s) would continue to operate until the liquid level recedes to the point where a third (bottom) mercury float switch would stop the pumps.

The two (2) pumps shall automatically alternate between the "lead" and "lag" positions by means of an electric alternator in the panel.

Should the liquid level continue to rise above the "Lag Pump On" level, a fourth mercury float switch would activate the alarm circuit.

- (2) Control Panel The duplex pump control panel shall be furnished by the pump Manufacturer, completely pre-wired, factory assembled, tested and ready for service. Where possible, pump controls shall be housed in a single panel. For outside installations at submersible stations, the panel shall be NEMA 3 door-in-door enclosure, fully gasketed with drip cap. The panel shall be suitable for mounting on the panel board as indicated on the DRAWINGS. The panel shall contain the following elements.
- Separate Manual Disconnect for each pump with 2pole adjustable overload protection for each phase
- Magnetic starter for each pump motor with all leg quick trip ambient compensated overload protection for each motor. Overloads are to have an auxiliary contract for automatic dialer.
- Hand-Off-Auto selector switch for each pump.
- Automatic Electric Alternator.

- Circuit Breaker for Control Circuit.
- Motor thermal protection Motor control circuit is to shut down if high temperature occurs. Manual resets to be provided.
- 4-Float control system for duplex pumps and alarm system.
- Control Disconnect.
- Seal failure light for each pump and contact closure for automatic dialer (submersible installations only).
- High temperature light for each pump and contact closure for automatic dialer (submersible installations
- Running light for each pump.
- Non-resettable, elapsed time meter for each pump, reading in tenths of hours. Capacity — 100,000
- High level glarm light with Red Globe and contact closure for automatic dialer. (Remote mounting for "package" pump station where panel is inside pump compartment).
- Audible alarm.
- Back-up battery (12v) for alarm.
- All necessary internal wiring, relays, etc. to provide the operation as described.

ENGINEERING SURVEYING P.O. BOX 311 1324 ROANOKE ROAD

PIERSON

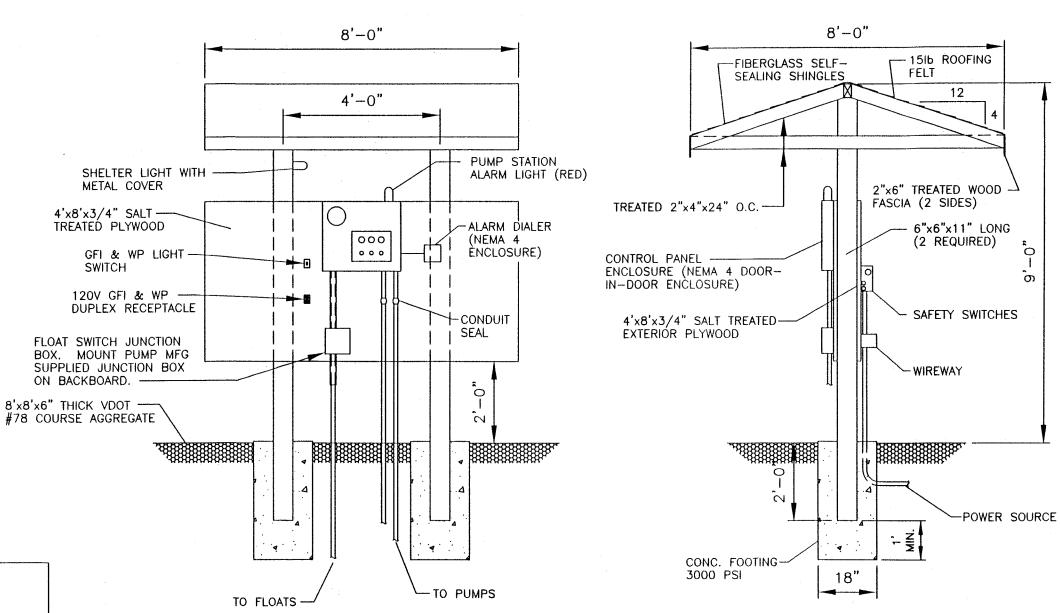
DATE: 05-01-02

03-03-03

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CONTROL PANEL SHELTER

- 1. DUPLEX EXPLOSION-PROOF HYDROMATIC MODEL HPGH 300 (SINGLE PHASE, 220 VOLTS, 3450 RPM)
- 2. 2" SEIMENS MAGNETIC FLOWMETER WITH TRIM OR EQUAL. METER TO HAVE A RANGE FROM 0 TO 60 GALLONS MINIMUM. DISPLAY SHALL BE LOCATED ON TOP OF METER IN DIRECTION OF ACCESS HATCH. FLOWMETER SHALL HAVE PROVISIONS FOR READING BY SENSUS TOUCH READ REMOTE READING METER. READ IN GALLONS. FLOW METER TO RECORD INSTANTANEOUS FLOW AND TOTAL FLOW IN GALLONS.
- 3. CONCRETE ANTI-FLOTATION COLLAR SHALL BE DOWELED TO BOTTOM OF PUMP STATION.
- 4. CONTRACTOR TO VERIFY ELEVATIONS PRIOR TO CONSTRUCTION.
- 5. ALUMINUM LADDER BRACKET @ 6' CENTER SECURED TO WET WELL WALL WITH STAINLESS STEEL ANCHORS. BITUMINUS COATING SHALL BE APPLIED AT ALL AREAS WHERE BRACKETS CONTACT CONCRETE SURFACE.
- 6. CONSTRUCTION SHALL BE IN ACCORDANCE TO BOTETOURT COUNTY SPECIFICATIONS FOR WASTEWATER PUMPING STATIONS AND FORCE MAINS.
- 7. ALL PIPING COUPLINGS, FITTINGS, VALVES, ETC. SHALL BE CLASS 125 FLANGED MEETING ANSI B16.1 SPECIFICATIONS.
- 8. WET WELL STRUCTURES SHALL BE PRECAST CONCRETE CONSTRUCTION AND CONFIRM TO
- ASTM C478, WITH WATERTIGHT JOINTS PER ASTM C443. 9. WET WELL CORROSION PROTECTION SHALL BE TWO (2) COATS KOPPERS "BITUAMASTIC"
- NO. 300M PER CORPS OF ENGINEERS SPECIFICATION C-200.
- 10. THE TWO (2) PUMPS SHALL AUTOMATICALLY ALTERNATE BETWEEN THE "LEAD" AND "LAG" POSITION BY MEANS OF AN ELECTRIC ALTERNATOR IN THE CONTROL PANEL.
- 11. AUTOMATIC DIALER SHALL BE AMERICAN MANUFACTURING CO. INC., MODEL #A4-AFLX WITH A
- SENAPHONE MODEL 1104 MONITORING SYSTEM. 12. AUTOMATIC STANDBY POWER GENERATOR SHALL BE INSTALLED PER BOTETOURT COUNTY SPECIFICATIONS FOR WASTEWATER PUMPING STATION AND FORCE MAINS. ONE (1) CATERPILLAR — OLYMPIAN L.P. GAS—ELECTRIC SET MODEL G25FS3 WITH BRUSHLESS GENERATOR, 25KW STANDBY AT 1.0 P.F. 120/240 VOTLS, 1 PHASE, 60 HERTZ AT 1800 rpm. 6" CONCRETE PAD TO BE PROVIDED
- 13. CONTRACTOR TO SUBMIT INFORMATION ON PROPSED ALARM DIALER SYSTEM TO BOTETOURT COUNTY
- AND ENGINEER PRIOR TO ORDERING EQUIPMENT. 14. CONTRACTOR TO SUBMIT INFORMATION ON PROPSED STANDBY POWER GENERATOR TO BOTETOURT
- COUNTY AND ENGINEER PRIOR TO ORDERING EQUIPMENT. 15. THE CONTRACTOR SHALL PROVIDE AND INSTAL ONE (1) HOIST AND EMBEDDED
- HOIST SOCKET (HALLIDAY D2B36B & D2E). 16. CONTRACTOR TO PROVIDE TELEPHONE LINE FOR AUTOMATIC DIALER SYSTEM.

RODERICK F. PIERSON NO. 023842 SS/ONAL F **** COMMISSION R200124

PUMP STATION

SHEET

(CONCRETE COLLAR NOT SHOWN FOR CLARITY)

-60' X 36' H-20 LOAD

RATED ALUM, ACCESS

-HDIST SDCKET

CAST INTO TOP

(FOR REMOVABLE CRANE)

∠s. DIb CONNECTION

CHECK VALVE

2' CLASS 53

ECCENTRIC

REDUCER

DIP (TYP.)

HATCH W/PADLOCK

POST (H2R-6036)

4' GALV. GOOSENECK -7

GUIDE RAIL ANCHORED-

PULTRUDED REINF

SUPPORT

GUIDE RAIL-

ANCHORED AT BOTTOM TO BASE A

ELBOW OF PUMP

LIFTING

INSIDE DIAMETER

FIBERGLASS GUIDE

W/ STAINLESS STEEL

VENT WITH SCREEN

(HKXXXXXXXX

30" X 30" H-20 LDAD

RATED ALUM, ACCESS

HATCH W/PADLOCK

PDST (H1R-3030)

PROTECTION

W/2 COATS

KOPPERS 300M

ACCESS LADDER

8' INFLUENT

N 1377.00

LAG PUMP IN 1376.80

LEAD PUMP

- SEE NOTE #3

1376.50

WET WELL INTERIOR COATED

ALUMINUM

TOTAL AMOUNT OF CONCRETE (+4.5 YARDS)

CONCRETE ALL THE WAY AROUND PUMP STATION

10 20 30 40 50 60 70 80 90 100 110 120

