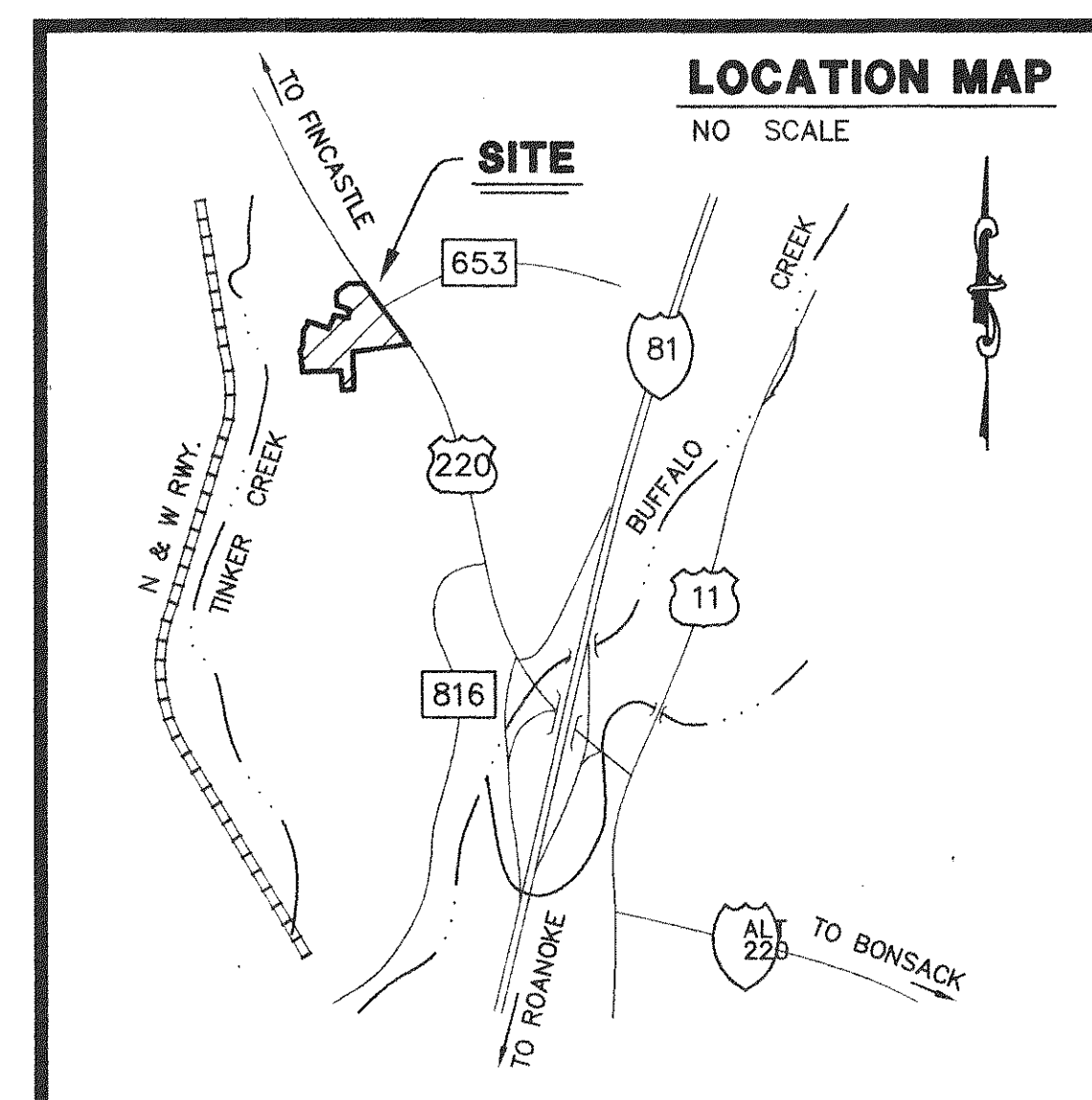


ROADWAY PLANS
FOR
PHASE I
"BOTETOURT COMMONS"

SITUATED IN
AMSTERDAM MAGISTERIAL DISTRICT
BOTETOURT COUNTY, VIRGINIA

DATE: 18 JANUARY 1999
"AS-BUILT" 23 AUGUST 1999
PROPERTY OF

TIMBERBROOK ASSOCIATES, L.L.C.



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6.	GRADING & STORM DRAINAGE
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LUMSDEN ASSOCIATES, P.C.
ENGINEERS—SURVEYORS—PLANNERS
ROANOKE, VIRGINIA

"AS-BUILT"

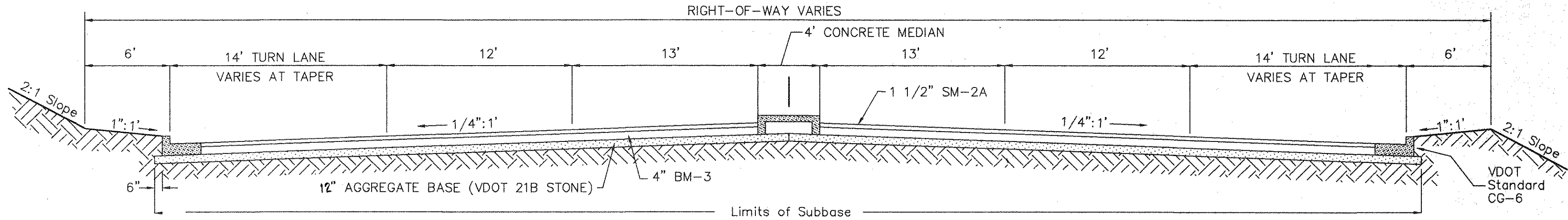
NOTE:

1. "AS-BUILT" INFORMATION TAKEN FROM PLANS FURNISHED BY J.P. TURNER & BROTHERS, INC., CONTRACTOR.
2. "AS-BUILT" DIMENSIONS SHOWN IN BRACKETS.

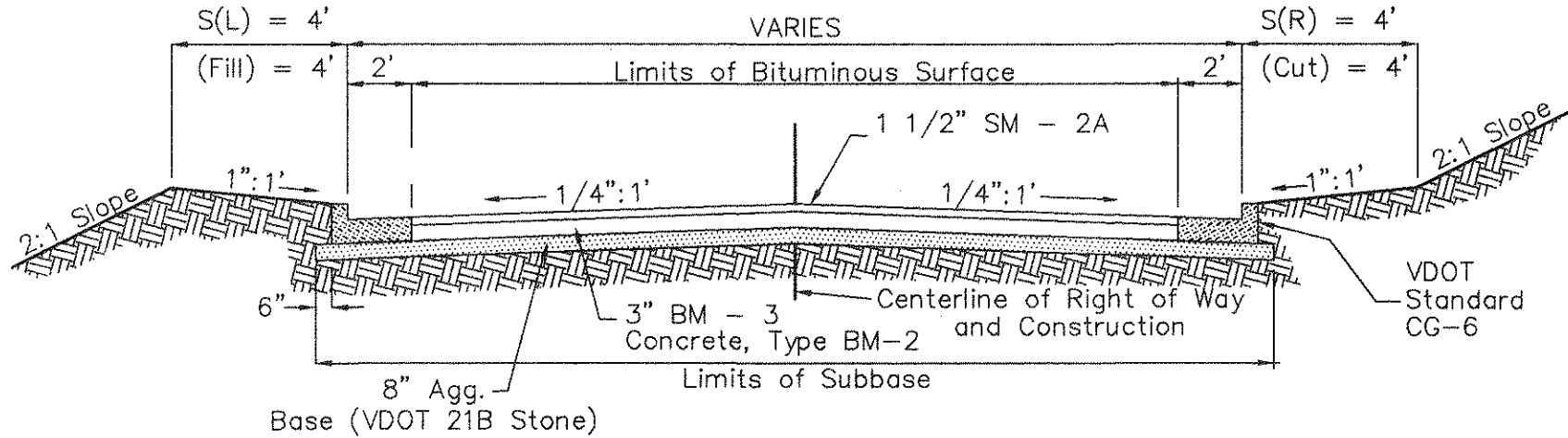
REVISIONS			
DATE	SHEETS	DATE	SHEETS
5-3-99	3, 4, 6, 7 & 10		

VIRGINIA DEPARTMENT OF TRANSPORTATION NOTES.

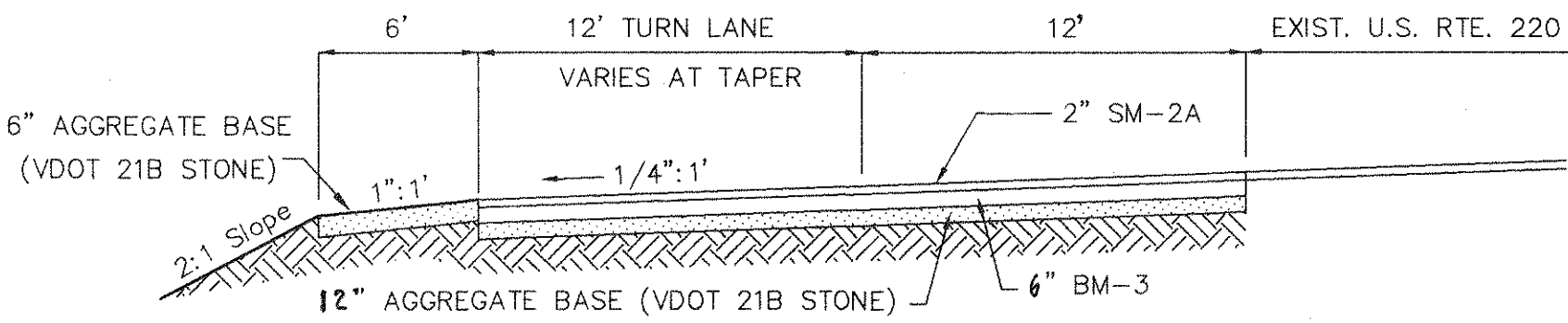
1. QUALITY CONTROL
STREETS TO BE GRADED, PAVED AND ALL STRUCTURAL COMPONENTS ERECTED IN ACCORDANCE WITH CURRENT VIRGINIA DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE SPECIFICATIONS AND ROAD DESIGN STANDARDS AND BOTETOURT COUNTY DESIGN STANDARDS AND SPECIFICATIONS. ALL MATERIALS USED SHALL BE TESTED IN ACCORDANCE WITH STANDARD POLICIES. THE DEVELOPER MUST CONTACT THE OFFICE OF THE RESIDENT ENGINEER, PRIOR TO BEGINNING ANY CONSTRUCTION AT WHICH TIME AN INSPECTION AND TESTING PROCEDURE POLICY WILL BE DRAWN. THE DEVELOPER WILL PRODUCE TEST REPORTS FROM APPROVED INDEPENDENT LABORATORIES AT THE DEVELOPER'S EXPENSE.
- THE PAVEMENT DESIGNS SHOWN ARE BASED ON A SUBGRADE CBR VALUE OF 10 OR GREATER. THE SUBGRADE SOIL IS TO BE TESTED BY AN INDEPENDENT LABORATORY AND THE RESULTS SUBMITTED TO THE VIRGINIA DEPARTMENT OF TRANSPORTATION PRIOR TO BASE CONSTRUCTION. SHOULD THE SUBGRADE CBR VALUES BE LESS THAN 10, THEN ADDITIONAL BASE MATERIAL WILL BE REQUIRED IN ACCORDANCE WITH DEPARTMENTAL SPECIFICATIONS.
- THE SUBGRADE MUST BE APPROVED BY VIRGINIA DEPARTMENT OF TRANSPORTATION PRIOR TO PLACEMENT OF THE BASE. BASE MUST BE APPROVED BY VIRGINIA DEPARTMENT OF TRANSPORTATION FOR DEPTH, TEMPLATE AND COMPACTION BEFORE SURFACE IS APPLIED.
2. UTILITIES
ALL NECESSARY UTILITY LATERALS ALONG WITH PROVISIONS FOR CONDUITS (I.E. WATER, SEWER, STORM, GAS AND TELEPHONE) WILL BE CONSTRUCTED PRIOR TO PLACEMENT OF BASE MATERIAL.
- GAS OR PETROLEUM TRANSMISSION LINES WILL NOT BE PERMITTED WITHIN THE PAVEMENT OR SHOULDER ELEMENT (BACK OF CURB TO BACK OF CURB) OF THIS DEVELOPMENT. SERVICE LATERALS CROSSING AND PIPE LINES LOCATED OUTSIDE THE PAVEMENT BUT INSIDE THE RIGHT OF WAY WILL BE CONSTRUCTED IN CONFORMITY WITH ASA B 31.8 SPECIFICATIONS AND SAFETY REGULATIONS. DISTRIBUTION LINES WITH PRESSURES LESS THAN 120 LBS. ARE UNAFFECTED BY THE ABOVE.
- PERMITS WILL BE REQUIRED FOR ALL UTILITIES WITHIN STREET RIGHT OF WAY PRIOR TO ACCEPTANCE INTO THE SECONDARY HIGHWAY SYSTEM.
- ANY EASEMENTS GRANTED TO A UTILITY COMPANY FOR PLACEMENT OF POWER, TELEPHONE, ETC. MUST BE RELEASED PRIOR TO ACCEPTANCE.
3. PRIVATE ENTRANCES
MODIFIED CG-9D GUTTER WILL BE PROVIDED AT ALL ENTRANCES TO PRIVATE LOTS WHERE STANDARD CG-6 CURB AND GUTTER IS APPROVED FOR USE.
- DRIVEWAYS CONNECTING TO ROADS WITHOUT CURB & GUTTER SHALL CONFORM TO THE PAVEMENT, SHOULDER & SLOPE.
- PERMITS WILL BE REQUIRED FOR ALL PRIVATE ENTRANCES CONSTRUCTED ON STREET RIGHTS OF WAY AFTER ACCEPTANCE INTO THE SECONDARY HIGHWAY SYSTEM.
4. EROSION CONTROL AND LANDSCAPING
CARE MUST BE TAKEN DURING CONSTRUCTION TO PREVENT EROSION, DUST AND MUD FROM DAMAGING ADJACENT PROPERTY, CLOGGING DITCHES, TRACKING PUBLIC STREETS AND OTHERWISE CREATING A PUBLIC OR PRIVATE NUISANCE TO SURROUNDING AREAS.
- THE ENTIRE CONSTRUCTION AREA INCLUDING DITCHES, CHANNELS, BACK OF CURBS AND OR PAVEMENT ARE TO BE BACKFILLED AND SEEDED AT THE EARLIEST POSSIBLE TIME AFTER FINAL GRADING.
- DRAINAGE EASEMENTS MUST BE DEFINED BY EXCAVATED DITCHES OR CHANNELS FOR THEIR FULL LENGTH TO WELL DEFINED EXISTING NATURAL WATERCOURSES.
- THE ROAD WILL BE REVIEWED DURING CONSTRUCTION FOR THE NEED OF PAVED DITCHES. IF EROSION IS ENCOUNTERED IN ANY DRAINAGE EASEMENT, IT WILL BE THE RESPONSIBILITY OF THE DEVELOPER TO SOD, RIP RAP, GROUT, PAVE OR TO DO WHATEVER IS NECESSARY TO CORRECT THE PROBLEM.
- ALL VEGETATION AND OVERBURDEN TO BE REMOVED FROM SHOULDER TO SHOULDER PRIOR TO THE CONDITIONING (CUTTING AND/OR PREPARATION) OF THE SUBGRADE.
5. INTERSECTION PAVEMENT RADIUS
MINIMUM PAVEMENT RADIUS OF 25 FEET IS REQUIRED AT ALL STREET INTERSECTION.
6. CONNECTIONS TO STATE-MAINTAINED ROADS
WHILE THESE PLANS HAVE BEEN APPROVED, SUCH APPROVAL DOES NOT EXEMPT CONNECTIONS WITH EXISTING STATE-MAINTAINED ROADS FROM CRITICAL REVIEW AT THE TIME PERMIT APPLICATIONS ARE MADE. THIS IS NECESSARY IN ORDER THAT THE PREVAILING CONDITIONS BE TAKEN INTO CONSIDERATION REGARDING SAFETY ACCOMPANIMENTS SUCH AS TURNING LANES.
7. GUARDRAILS
STANDARD GUARDRAIL WITH SAFETY END SECTIONS MAY BE REQUIRED ON FILLS AS DEEMED NECESSARY BY THE RESIDENT ENGINEER. AFTER COMPLETION OF ROUGH GRADING OPERATIONS, THE OFFICE OF THE RESIDENT ENGINEER, SHALL BE NOTIFIED SO THAT A FIELD REVIEW MAY BE MADE OF THE PROPOSED LOCATIONS.
- WHERE GUARDRAILS ARE TO BE INSTALLED THE SHOULDER WIDTH SHALL BE INCREASED IN ACCORDANCE WITH VDOT ROAD AND BRIDGE STANDARDS.
8. STORM DRAINAGE
FIELD REVIEW WILL BE MADE DURING CONSTRUCTION TO DETERMINE THE NEED AND LIMITS OF PAVED DITCHES AND/OR DITCH STABILIZATION TREATMENTS, AND TO DETERMINE THE NEED AND LIMITS OF ADDITIONAL DRAINAGE EASEMENTS. ALL DRAINAGE EASEMENTS MUST BE CUT AND MADE TO FUNCTION TO A NATURAL WATERCOURSE. ANY EROSION PROBLEMS ENCOUNTERED IN AN EASEMENT MUST BE CORRECTED BY WHATEVER MEANS NECESSARY PRIOR TO SUBDIVISION ACCEPTANCE.
- DITCH SLOPES ARE TO BE FOUR TO ONE (4:1) FOR SHOULDER WIDTHS OF SIX FEET (6') OR GREATER AND THREE TO ONE (3:1) FOR SHOULDER WIDTHS OF FOUR FEET (4') OR FIVE FEET (5'), UNLESS OTHERWISE SPECIFIED IN THE PLANS.
9. ENTRANCE PERMIT
CONTRACTOR SHALL OBTAIN ENTRANCE PERMIT TO THE EXISTING VIRGINIA DEPARTMENT OF TRANSPORTATION RIGHT OF WAY FROM RESIDENT ENGINEER PRIOR TO ROAD CONSTRUCTION.
10. INSPECTION
AN INSPECTOR WILL NOT BE FURNISHED EXCEPT FOR PERIODIC PROGRESS INSPECTION, THE ABOVE MENTIONED FIELD REVIEWS AND CHECKING FOR REQUIRED STONE DEPTHS. THE DEVELOPER WILL BE REQUIRED TO POST A SURETY TO GUARANTEE THE ROAD FREE OF DEFECTS FOR ONE YEAR AFTER ACCEPTANCE BY THE DEPARTMENT OF TRANSPORTATION.
11. STREET MAINTENANCE
THE STREETS MUST BE PROPERLY MAINTAINED UNTIL ACCEPTANCE. AT SUCH TIME AS ALL REQUIREMENTS HAVE BEEN MET FOR ACCEPTANCE, ANOTHER INSPECTION WILL BE MADE TO DETERMINE THAT THE STREET HAS BEEN PROPERLY MAINTAINED.
12. UNDERGROUND UTILITIES
CONTRACTOR SHALL VERIFY LOCATION AND ELEVATION OF ALL UNDERGROUND UTILITIES SHOWN ON THE PLANS IN AREAS OF CONSTRUCTION PRIOR TO STARTING WORK BY CONTACTING MISS UTILITY. CONTACT SITE ENGINEER IMMEDIATELY IF LOCATION OR ELEVATION IS DIFFERENT FROM THAT SHOWN ON THE PLANS. IF THERE APPEARS TO BE A CONFLICT, AND UPON DISCOVERY OF ANY UTILITY NOT SHOWN ON THIS PLAN, CALL "MISS UTILITY" OF CENTRAL VIRGINIA AT 1-800-552-7001.
13. REVISIONS OF SPECIFICATIONS AND STANDARDS
APPROVAL OF THESE PLANS WILL BE BASED ON SPECIFICATIONS AND STANDARDS IN EFFECT AT THE TIME OF APPROVAL AND WILL BE SUBJECT, UNTIL COMPLETION OF THE ROADWAY AND ACCEPTANCE BY THE DEPARTMENT, TO FUTURE REVISIONS OF THE SPECIFICATIONS AND STANDARDS.



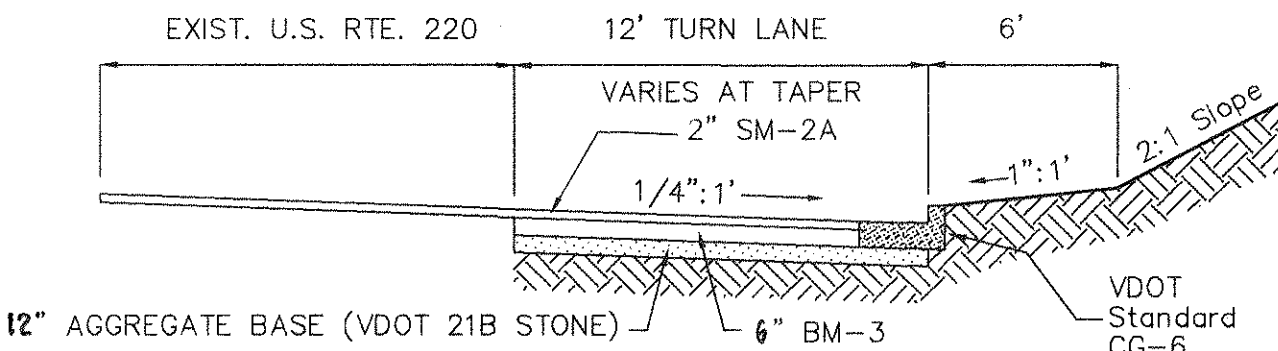
TYPICAL SECTION
COMMONS PARKWAY
10 + 30.00 TO 15 + 50.00



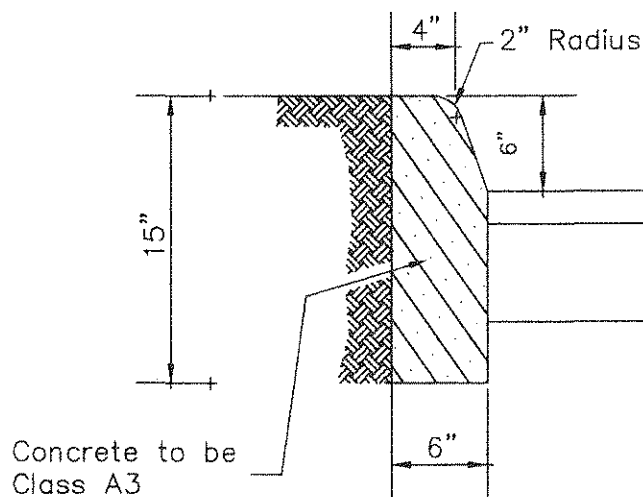
TYPICAL SECTION
COMMONS PARKWAY
15 + 50.00 TO CUL DE SAC



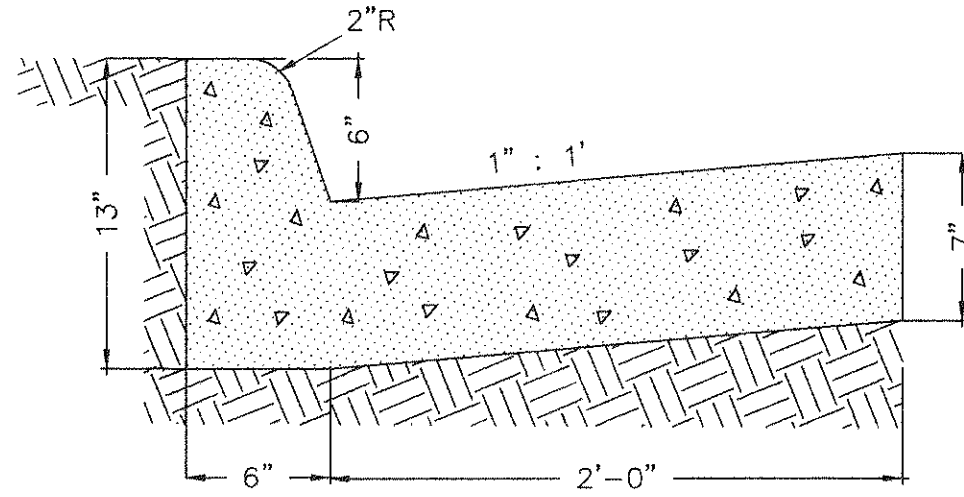
NORTHBOUND - U.S. RTE. 220
LEFT TURN LANE



SOUTHBOUND - U.S. RTE. 220
RIGHT TURN LANE



CONCRETE CURB (CG-2)



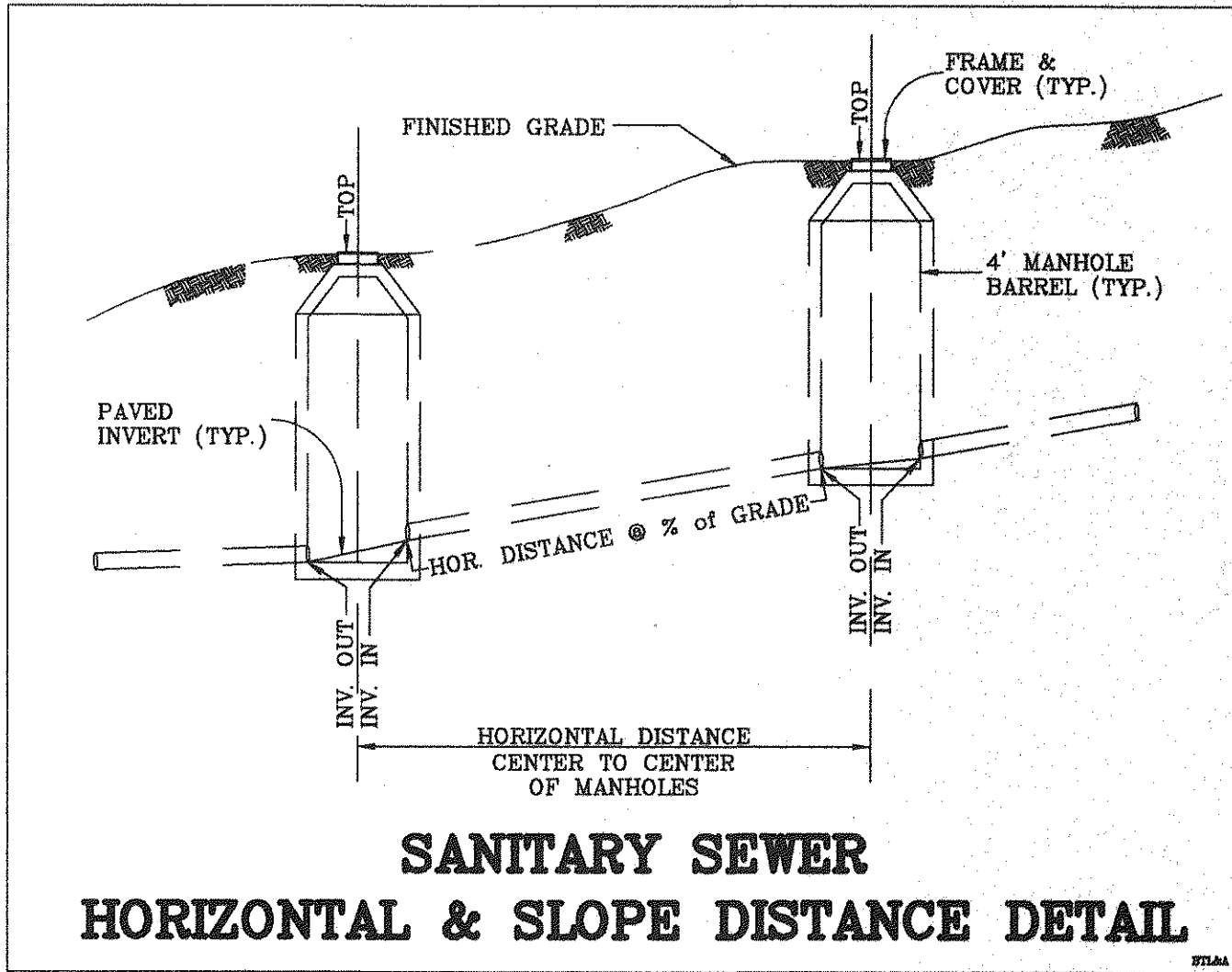
CONCRETE CURB & GUTTER (CG-6)

CONSTRUCTION NOTES

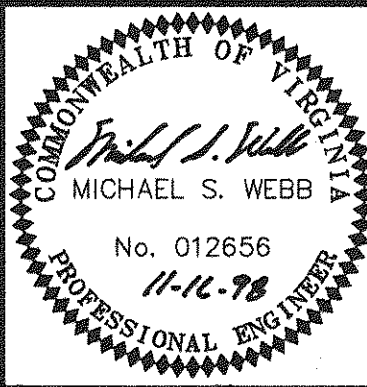
1. ALL CONSTRUCTION SHALL CONFORM TO THE CURRENT COUNTY OF BOTETOURT STANDARDS AND SPECIFICATIONS.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE OWNER AND THE ENGINEER OF ANY CHANGES OR CONDITIONS ATTACHED TO PERMITS OBTAINED FROM ANY AUTHORITY ISSUING PERMITS.
3. NO SUBSOIL INVESTIGATIONS HAVE BEEN MADE BY THE DESIGNING ENGINEER.
4. THE CONTRACTOR SHALL VISIT THE SITE AND VERIFY EXISTING CONDITIONS PRIOR TO STARTING CONSTRUCTION.
5. THE CONTRACTOR SHALL VERIFY THE LOCATION AND DEPTH OF ALL UNDERGROUND UTILITIES PRIOR TO CONSTRUCTION.

GRADING NOTES

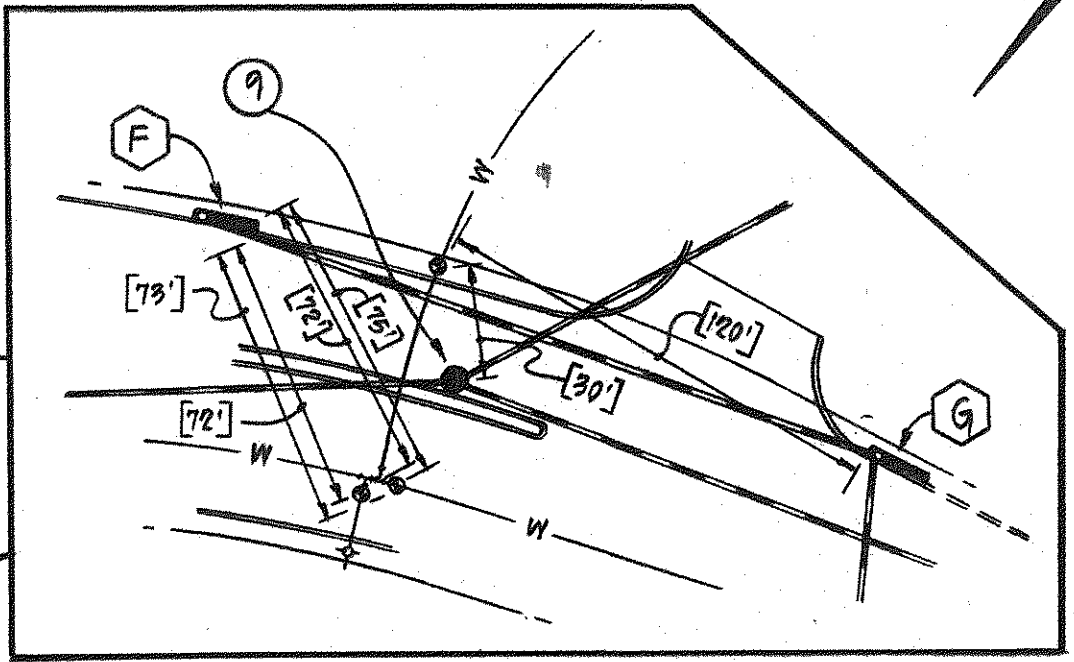
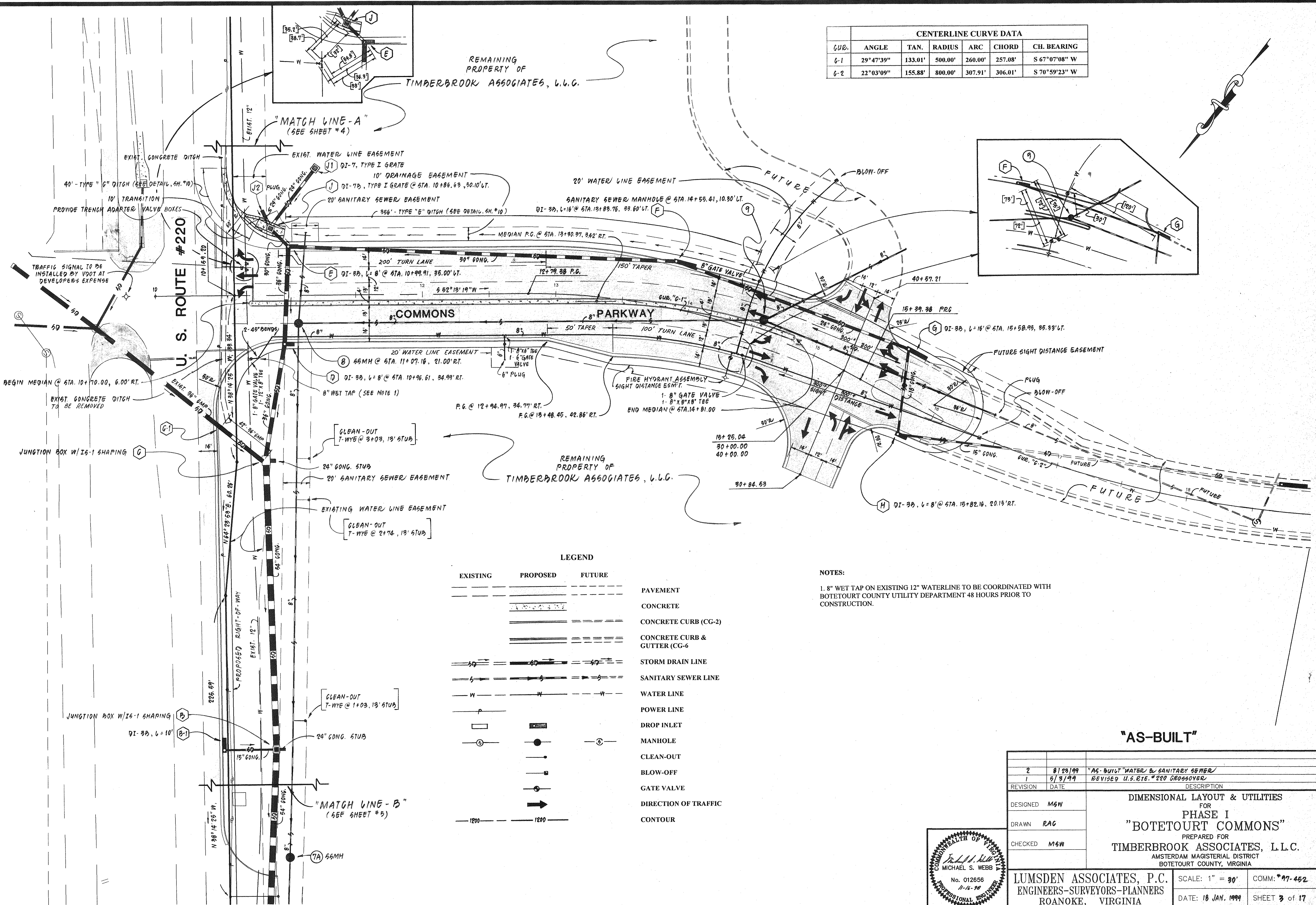
1. ALL AREAS TO BE GRADED SHALL BE STRIPPED OF PAVEMENT AND ORGANIC MATTER. ALL MATERIAL NOT SUITED FOR USE AS FILL MATERIAL SHALL BE REMOVED FROM THE SITE AND DISPOSED OF PROPERLY.
2. ANY BORROW OR WASTE SITE MUST HAVE AN APPROVED EROSION AND SEDIMENT CONTROL PLAN PRIOR TO CONSTRUCTION.
3. TOPSOIL SHALL BE REMOVED FROM THE CLEARED AREA AND STOCKPILED FOR FUTURE USE.
4. FILL MATERIAL SHALL BE FREE OF ORGANIC MATTER AND ROCKS LARGER THAN 6 INCHES IN DIAMETER. FILL MATERIAL SHALL BE PLACED AND COMPACTED IN 8 INCH LIFTS TO 95% MINIMUM DENSITY, STANDARD PROCTOR.



REVISION	DATE	DESCRIPTION
DESIGNED	MSW	
DRAWN	RAC	
CHECKED	MSW	
NOTES & DETAILS FOR PHASE I "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA		
LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA		SCALE: AS SHOWN DATE: 10 JAN. 1999 COMM: #97-452 SHEET 2 of 17



CENTERLINE CURVE DATA						
CUR.	ANGLE	TAN.	RADIUS	ARC	CHORD	CH. BEARING
6-1	29°47'39"	133.01'	500.00'	260.00'	257.08'	S 67°07'08" W
6-2	22°03'09"	155.88'	800.00'	307.91'	306.01'	S 70°59'23" W



LEGEND

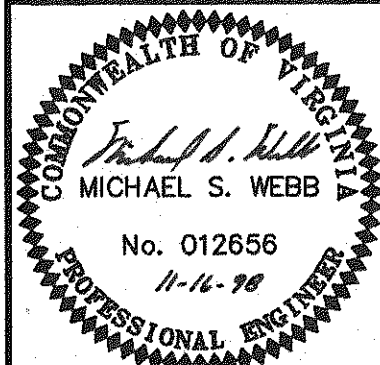
EXISTING	PROPOSED	FUTURE	
---	---	---	PAVEMENT
---	---	---	CONCRETE
---	---	---	CONCRETE CURB (CG-2)
---	---	---	CONCRETE CURB & GUTTER (CG-6)
---	---	---	STORM DRAIN LINE
---	---	---	SANITARY SEWER LINE
---	---	---	WATER LINE
---	---	---	POWER LINE
---	---	---	DROP INLET
---	---	---	MANHOLE
---	---	---	CLEAN-OUT
---	---	---	BLOW-OFF
---	---	---	GATE VALVE
---	---	---	DIRECTION OF TRAFFIC
---	---	---	CONTOUR

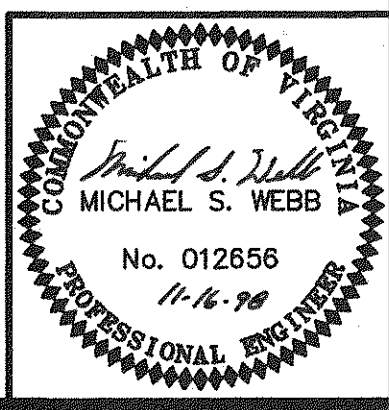
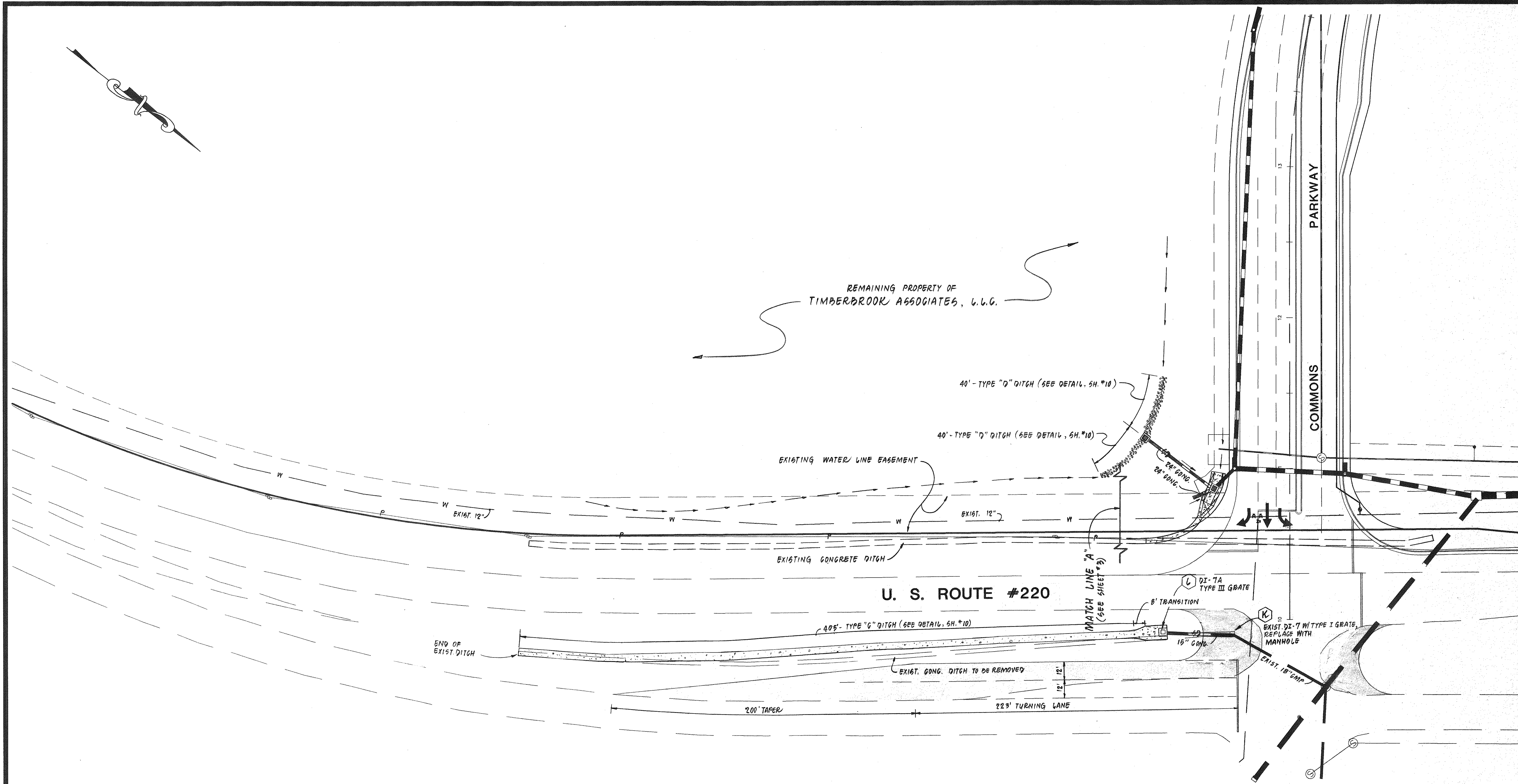
NOTES:

1. 8" WET TAP ON EXISTING 12" WATERLINE TO BE COORDINATED WITH BOTETOURT COUNTY UTILITY DEPARTMENT 48 HOURS PRIOR TO CONSTRUCTION.

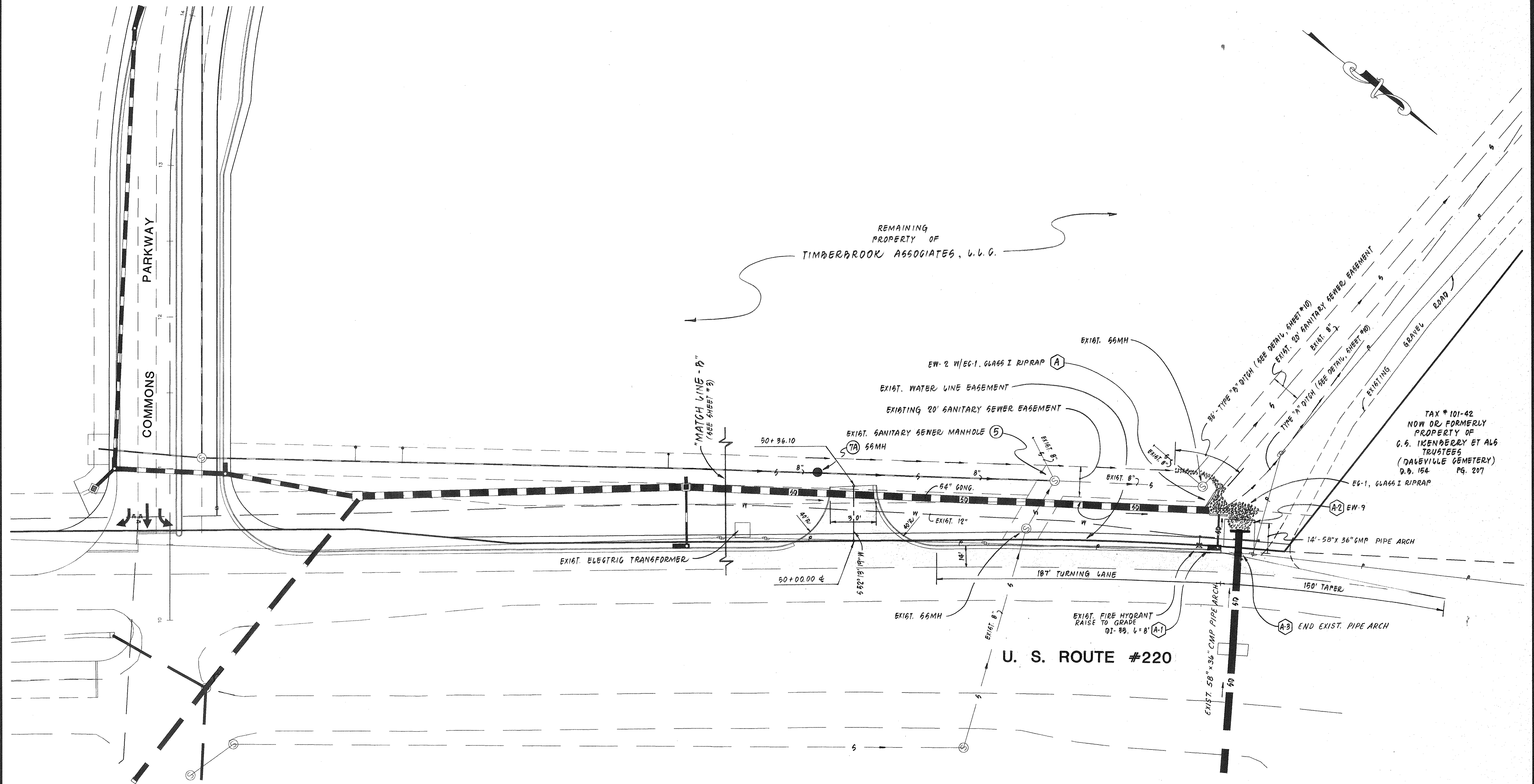
"AS-BUILT"

REVISION	DATE	DESCRIPTION
2	8/23/99	"AS-BUILT" WATER & SANITARY SEWER
1	6/3/99	REVISED U.S. RTE. #220 CROSSOVER
DESIGNED: MSW		
DRAWN: RAG		
CHECKED: MSW		
DIMENSIONAL LAYOUT & UTILITIES FOR PHASE I "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA		
LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA		SCALE: 1" = 30' DATE: 18 JAN. 1999 COMM: 97-462 SHEET 3 of 17



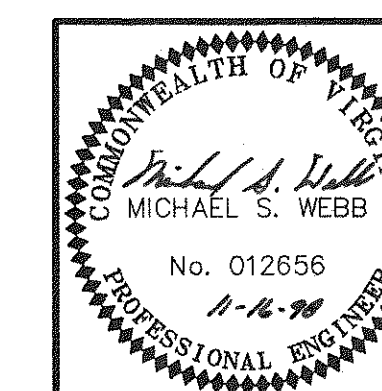


	5-3-99	REVISED U.S. RTE 220 CROSSOVER & DI-7
	2-26-99	REVISED MEDIAN DI-7
REVISION	DATE	DESCRIPTION
DESIGNED	MSW	DIMENSIONAL LAYOUT & UTILITIES FOR PHASE I "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA
DRAWN	RAC	
CHECKED	MSW	
LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA		SCALE: 1" = 30' COMM: *97-452
		DATE: 18 JAN. 1999 SHEET 4 of 17

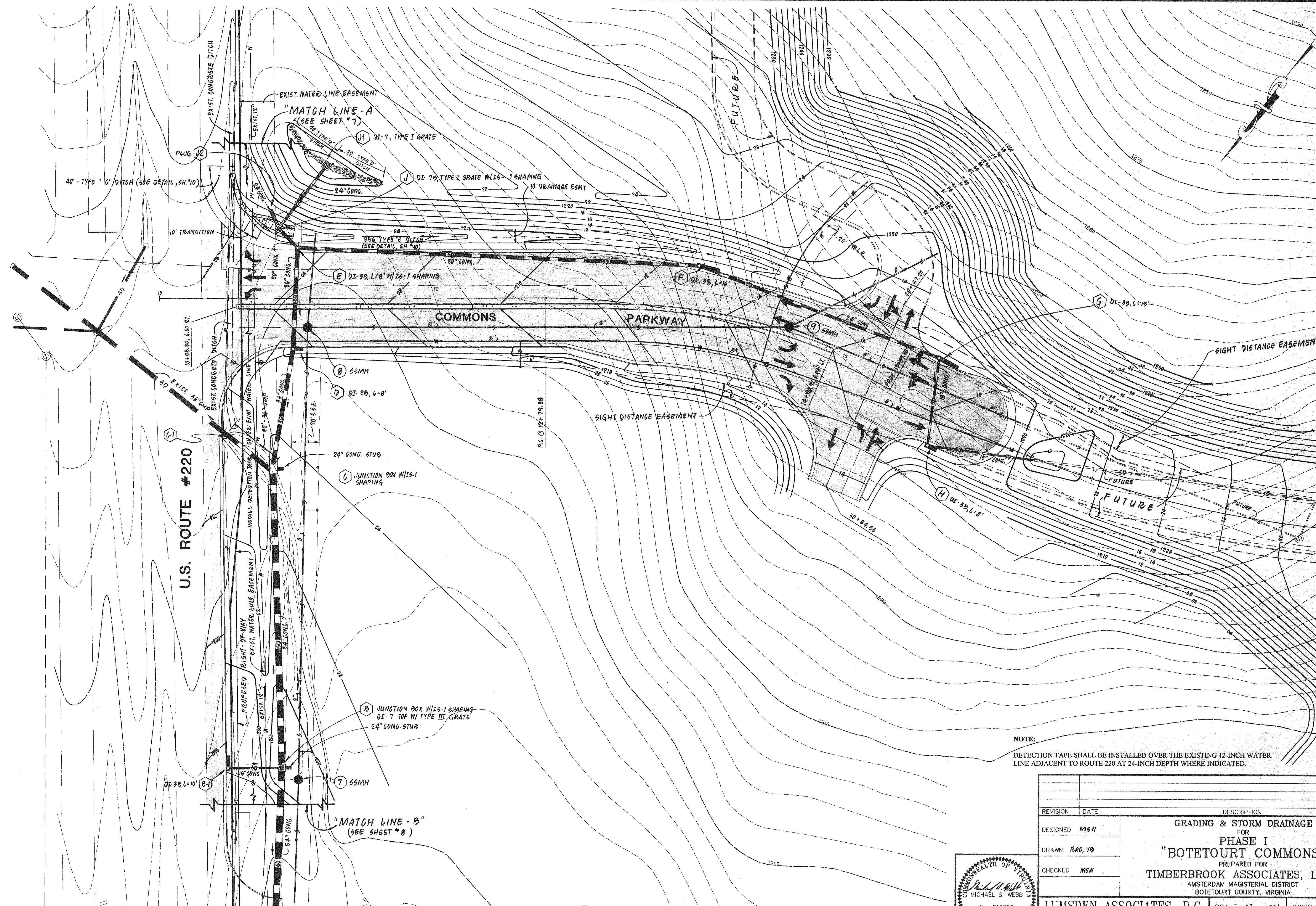


"AS-BUILT"

REVISION	DATE	DESCRIPTION
1	8/23/99	"AS-BUILT" WATER & SANITARY SEWER
DESIGNED	MSW	DIMENSIONAL LAYOUT & UTILITIES FOR "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA
DRAWN	RAC	
CHECKED	MSW	
LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA		SCALE: 1" = 30' DATE: 10 JAN. 1999 COMM: *97-452 SHEET 5 of 17

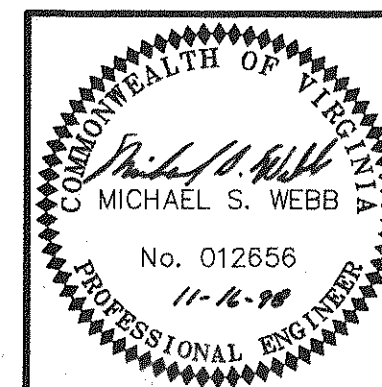


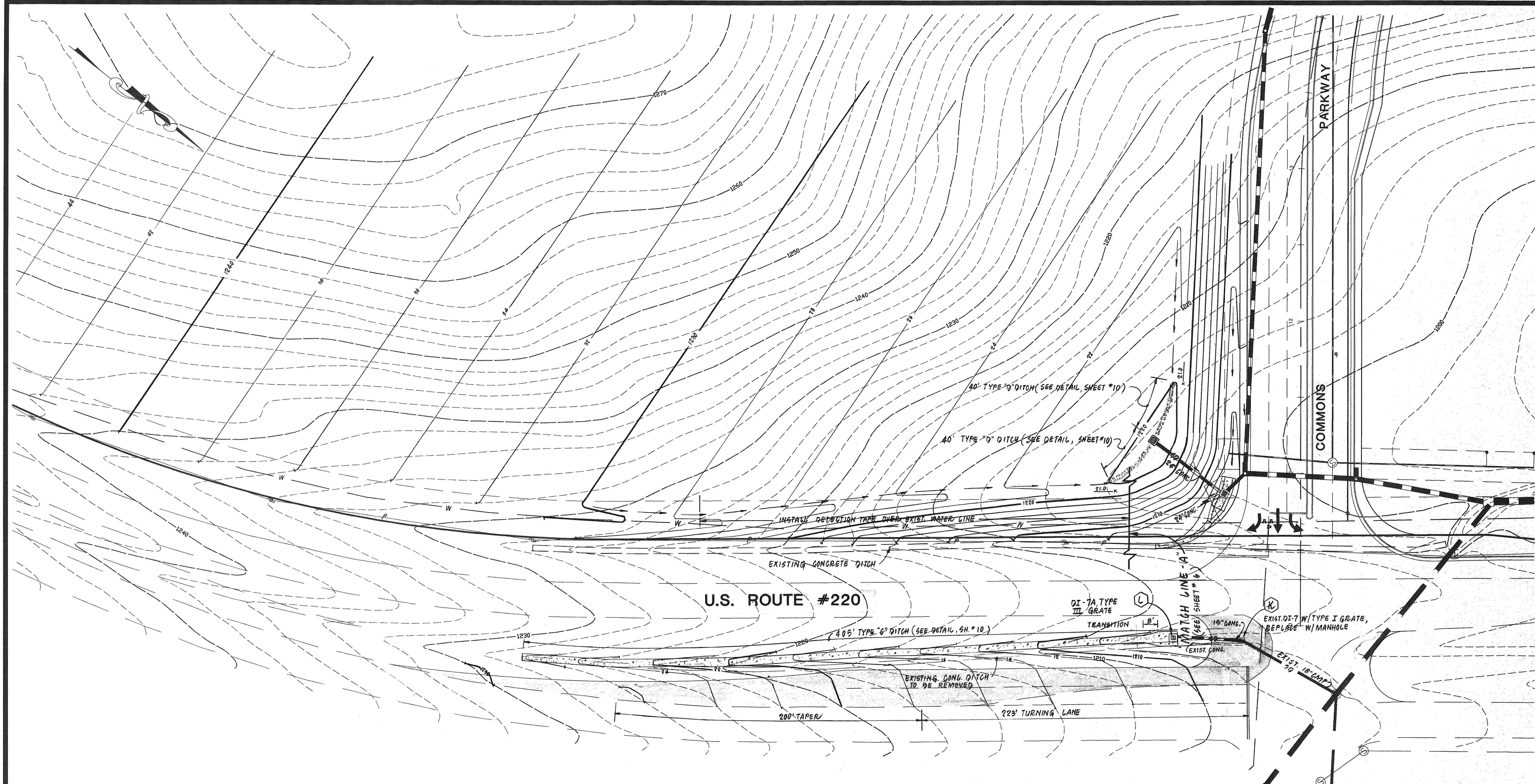
LUMSDEN ASSOCIATES, P.C. 1000 N. 10TH ST. SUITE 100 ROANOKE, VA 24011



NOTE:
DETECTION TAPE SHALL BE INSTALLED OVER THE EXISTING 12-INCH WATER LINE ADJACENT TO ROUTE 220 AT 24-INCH DEPTH WHERE INDICATED.

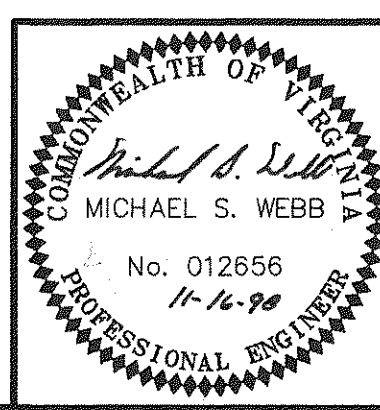
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DESIGNED		MSW	GRADING & STORM DRAINAGE FOR PHASE I "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA
DRAWN		RAC, VB	
CHECKED		MSW	
LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA			SCALE: 1" = 30'
			DATE: 10 JAN. 1999
			SHEET 6 of 17

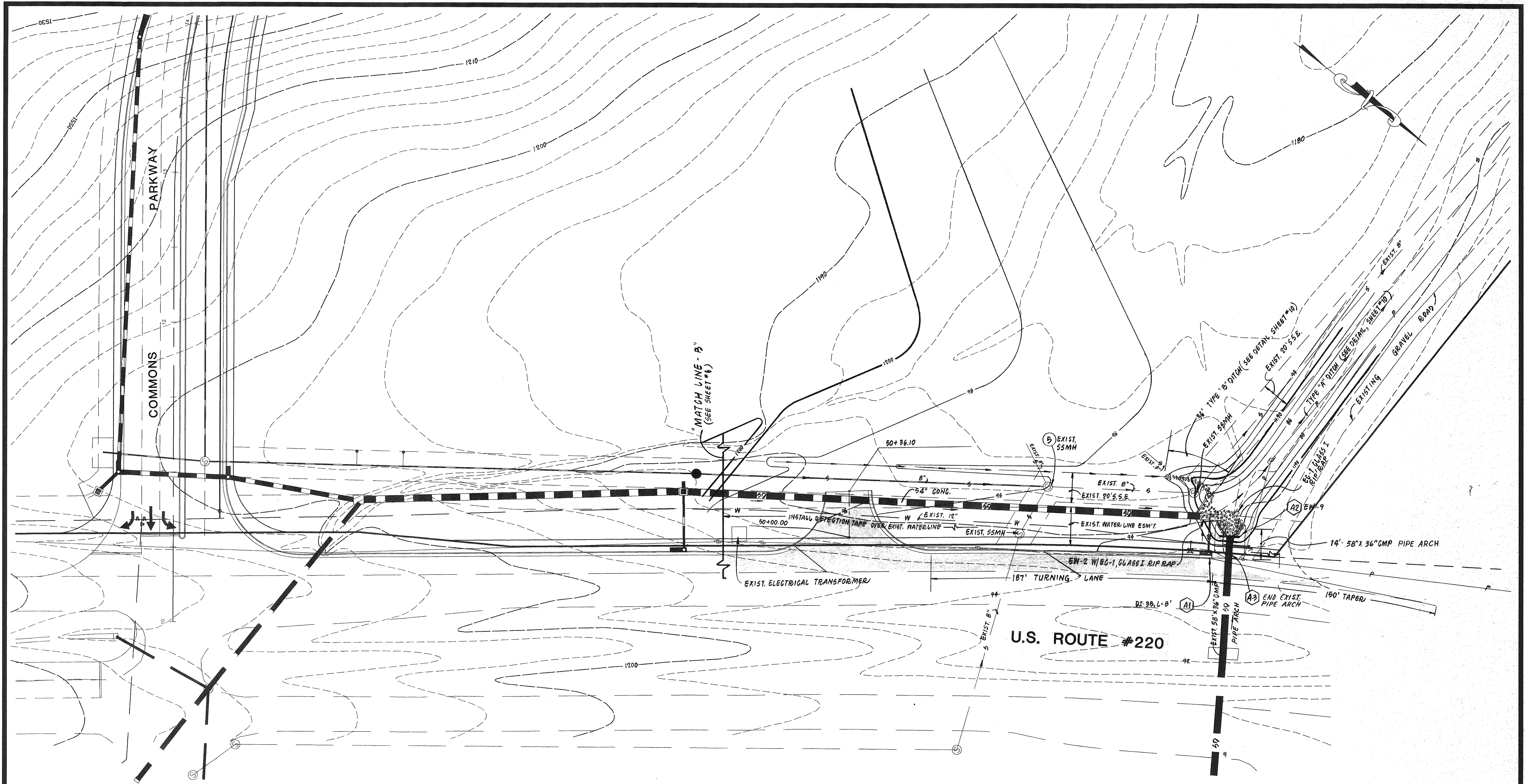




NOTE:
DETECTION TAPE SHALL BE INSTALLED OVER THE EXISTING 12-INCH WATER LINE ADJACENT TO ROUTE 220 AT 24-INCH DEPTH WHERE INDICATED.

5-3-99		REVISED U.S. RTE #220 CROSSOVER
2-26-99		REVISED MEDIAN DI-7
REVISION	DATE	DESCRIPTION
DESIGNED	MSW	GRADING & STORM DRAINAGE FOR PHASE I "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA
DRAWN	MSW, VB	
CHECKED	MSW	
LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA		SCALE: 1" = 30' DATE: 10 JAN. 1999
		COMM: #97-452 SHEET 7 of 17

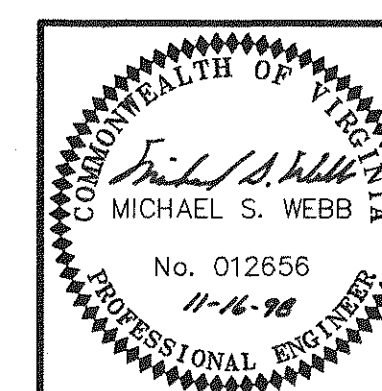




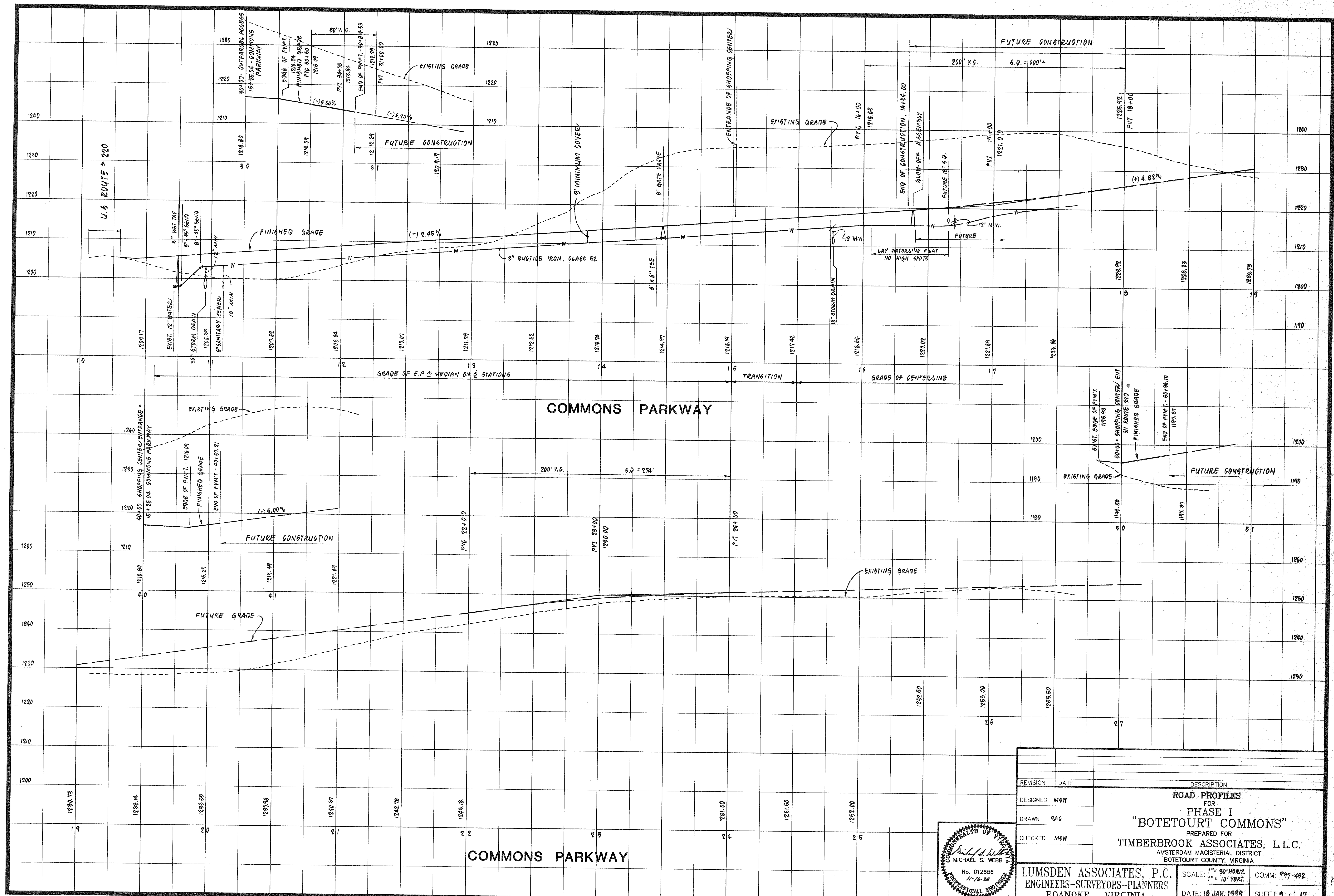
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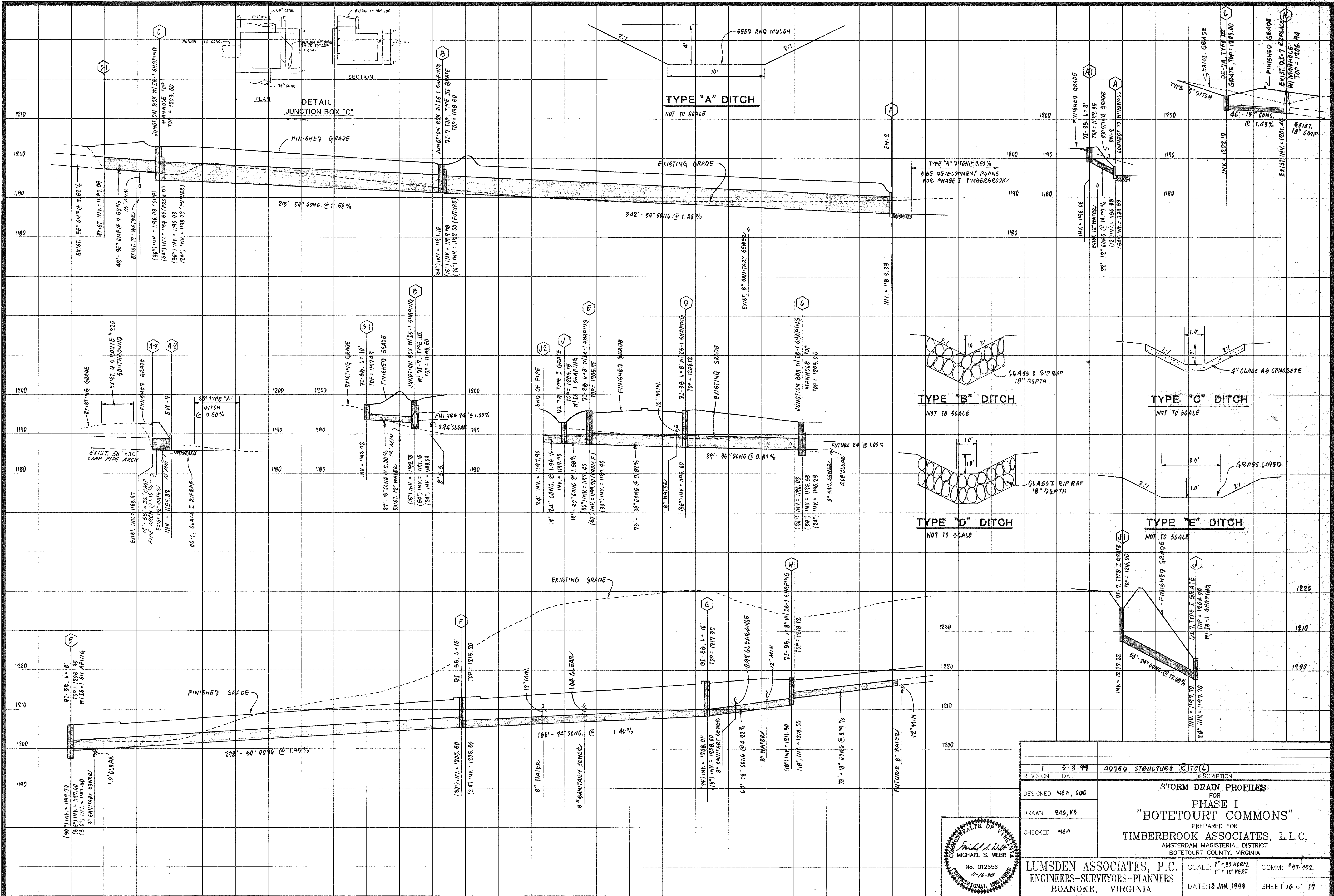
DETECTION TAPE SHALL BE INSTALLED OVER THE EXISTING 12-INCH WATER LINE ADJACENT TO ROUTE 220 AT 24-INCH DEPTH WHERE INDICATED.

REVISION	DATE	DESCRIPTION
DESIGNED	MSW	GRADING & STORM DRAINAGE FOR PHASE I "BOTETOURT COMMONS"
DRAWN	MSW, VCB	PREPARED FOR
CHECKED	MSW	TIMBERBROOK ASSOCIATES, L.L.C.
		AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA
		LUMSDEN ASSOCIATES, P.C.
		ENGINEERS-SURVEYORS-PLANNERS
		ROANOKE, VIRGINIA
		SCALE: 1" = 30'
		DATE: 10 JAN. 1999
		COMM: #97-452
		SHEET 8 of 17

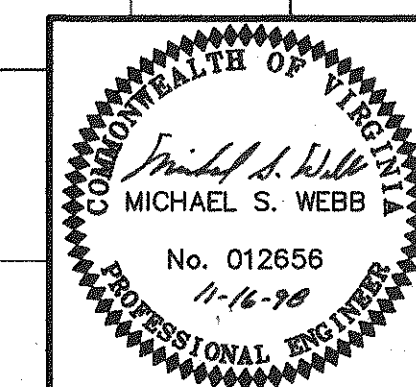


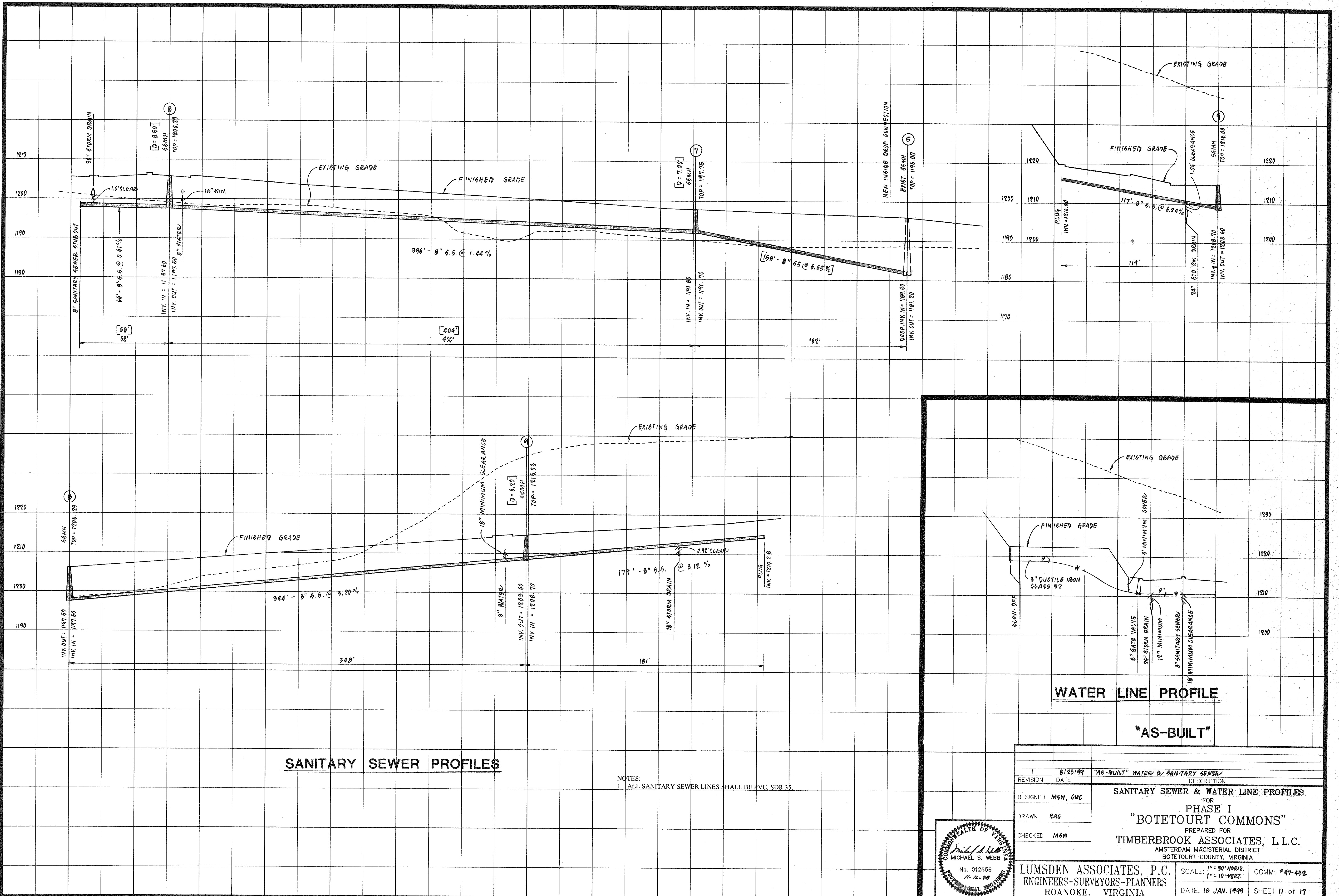
2025 RELEASE UNDER E.O. 14176

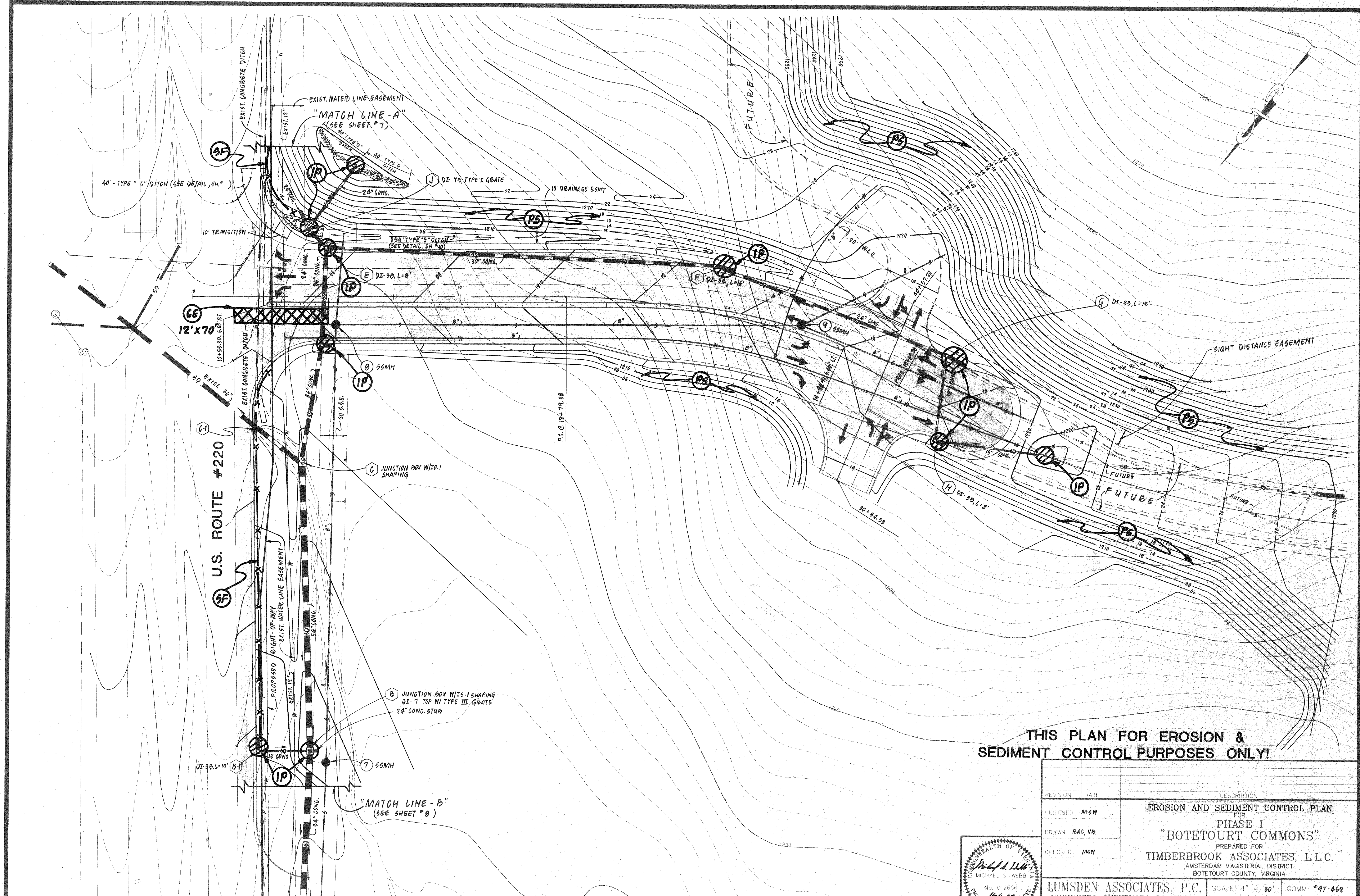




REVISION	5-3-99	ADDED STRUCTURE (K) TO (L)
DESIGNED	MSW, CDC	
DRAWN	RAG, VB	
CHECKED	MSW	
STORM DRAIN PROFILES FOR PHASE I "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA		
LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA		SCALE: 1" = 30' HORIZ. 1" = 10' VERT. DATE: 10 JAN. 1999
COMM: *97.452		SHEET 10 of 17



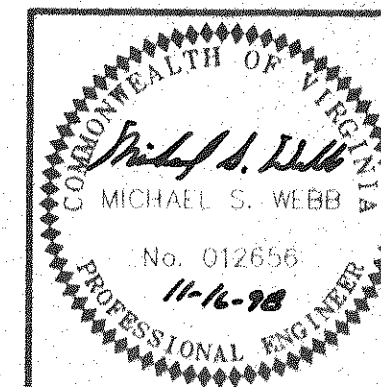


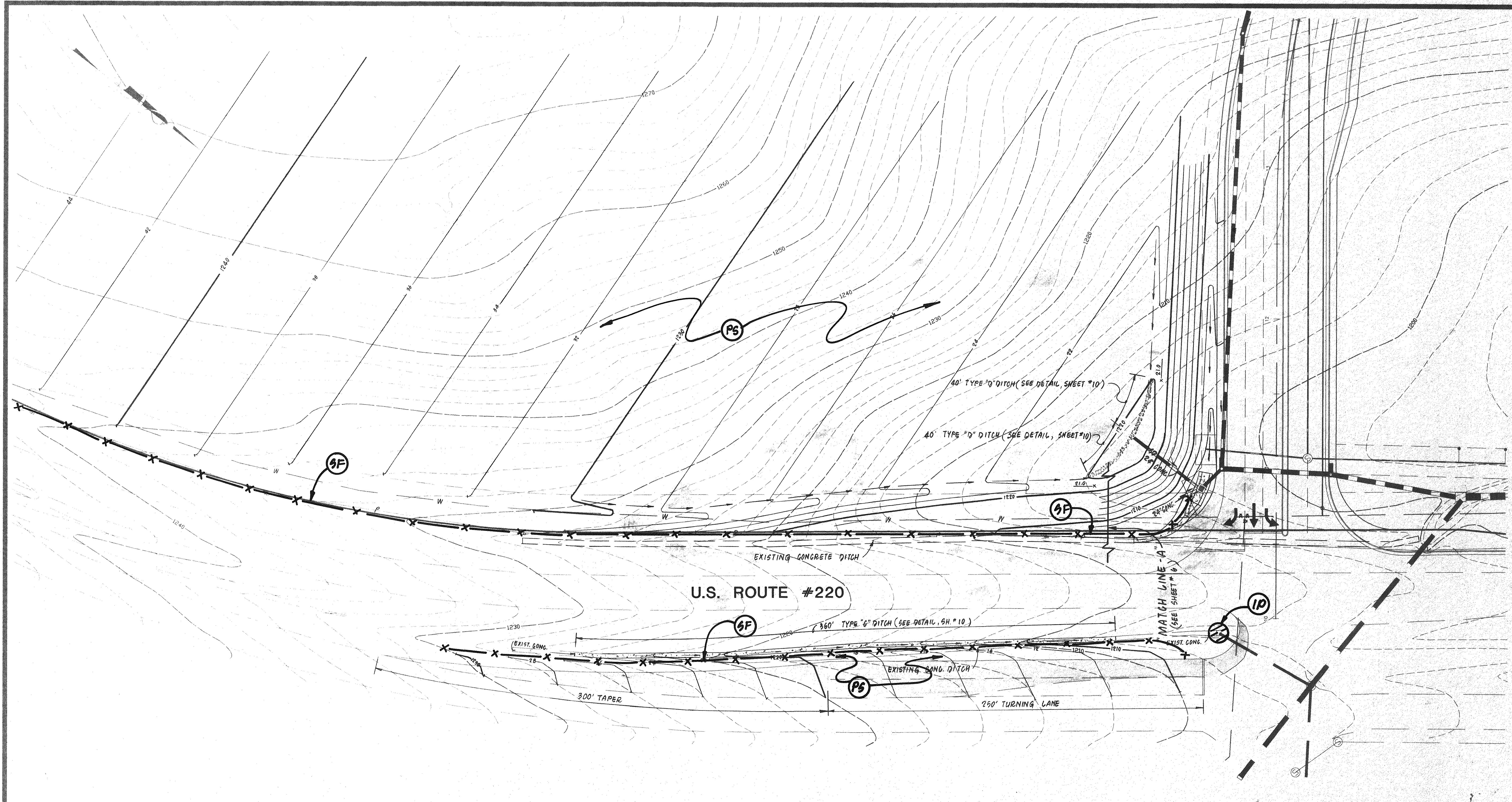


THIS PLAN FOR EROSION & SEDIMENT CONTROL PURPOSES ONLY!

REVISION	DATE	DESCRIPTION
DESIGNED	MSW	EROSION AND SEDIMENT CONTROL PLAN FOR PHASE I "BOTETOURT COMMONS"
DRAWN	RAC, VP	PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA
CHECKED	MSW	

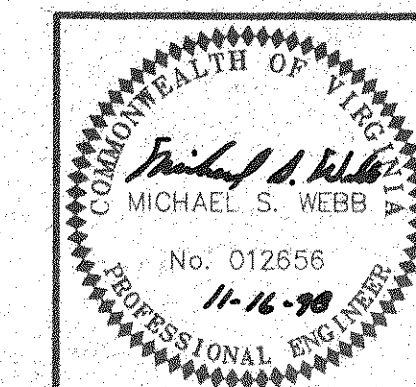
LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA	SCALE: 1" = 30'	COMM: *97-452
	DATE: 18 JAN. 1999	SHEET 12 of 17





THIS PLAN FOR EROSION & SEDIMENT CONTROL PURPOSES ONLY!

REVISION	DATE	DESCRIPTION
DESIGNED	MSW	EROSION AND SEDIMENT CONTROL PLAN FOR PHASE I "BOTETOURT COMMONS"
DRAWN	MSW, VD	PREPARED FOR
CHECKED	MSW	TIMBERBROOK ASSOCIATES, L.L.C.
		AMSTERDAM MAGISTERIAL DISTRICT
		BOTETOURT COUNTY, VIRGINIA



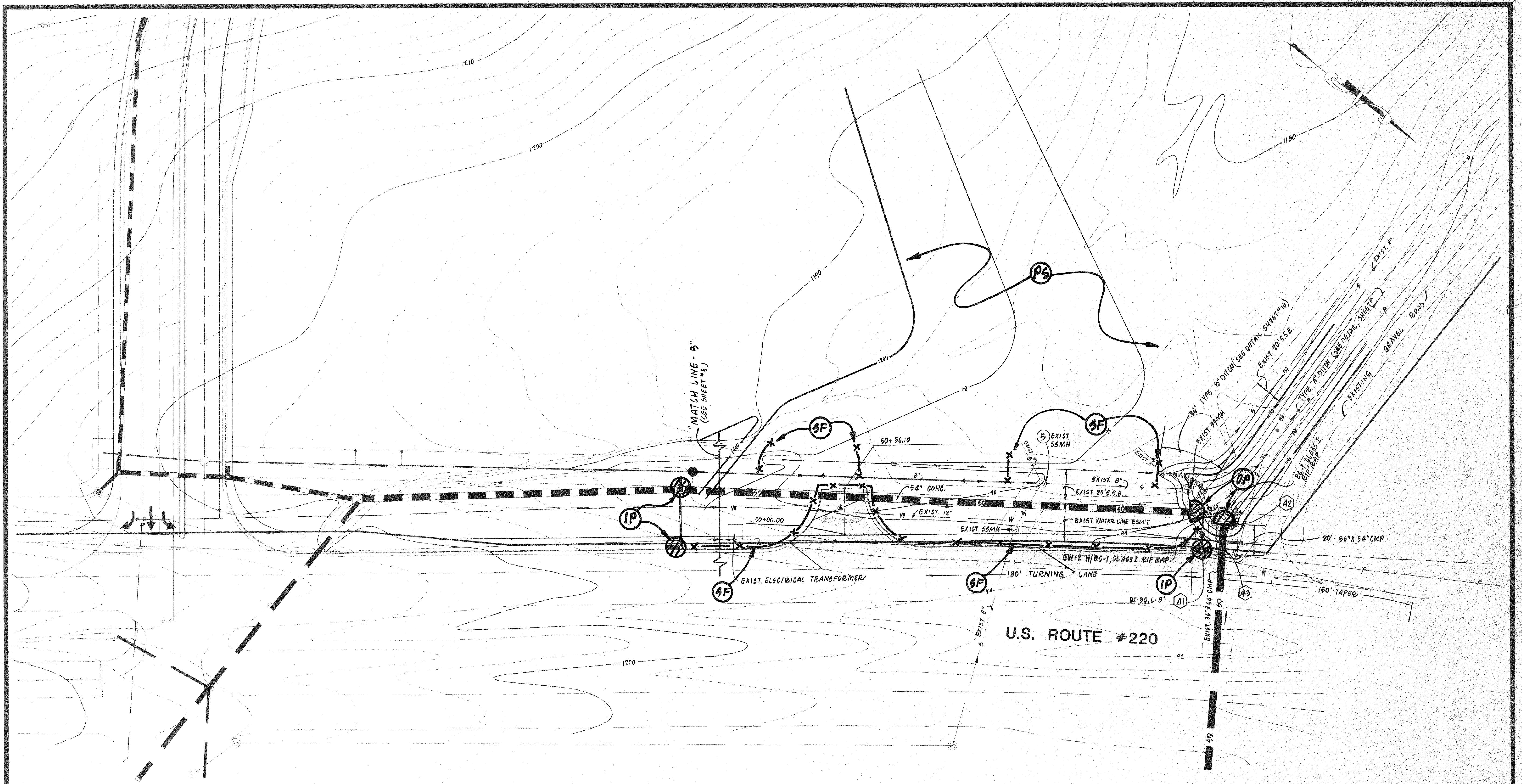
LUMSDEN ASSOCIATES, P.C.
ENGINEERS-SURVEYORS-PLANNERS
ROANOKE, VIRGINIA

SCALE: 1" = 30'

DATE: 18 JAN. 1999

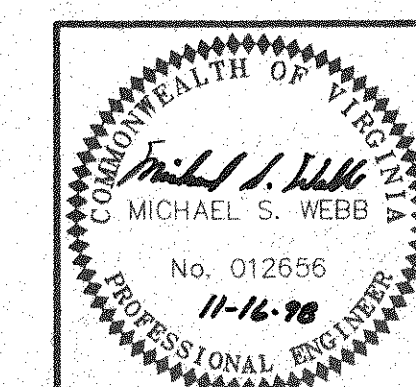
COMM: #97-452

SHEET 13 of 17



THIS PLAN FOR EROSION & SEDIMENT CONTROL PURPOSES ONLY!

REVISION	DATE	DESCRIPTION
DESIGNED	M&W	EROSION AND SEDIMENT CONTROL PLAN FOR PHASE I "BOTETOURT COMMONS"
DRAWN	M&W, V&B	PREPARED FOR
CHECKED	M&W	TIMBERBROOK ASSOCIATES, L.L.C.
		AMSTERDAM, MAGISTERIAL DISTRICT
		BOTETOURT COUNTY, VIRGINIA
LUMSDEN ASSOCIATES, P.C.		SCALE: 1" = 30'
ENGINEERS-SURVEYORS-PLANNERS		DATE: 10 JAN, 1999
ROANOKE, VIRGINIA		COMM: #97-452
		SHEET 14 OF 17



10-10-11-12-13-14-15-16-17-18-19-20-21-22-23-24-25-26-27-28-29-30-31-32-33-34-35-36-37-38-39-40-41-42-43-44-45-46-47-48-49-50-51-52-53-54-55-56-57-58-59-60-61-62-63-64-65-66-67-68-69-70-71-72-73-74-75-76-77-78-79-80-81-82-83-84-85-86-87-88-89-90-91-92-93-94-95-96-97-98-99-100-101-102-103-104-105-106-107-108-109-110-111-112-113-114-115-116-117-118-119-120-121-122-123-124-125-126-127-128-129-130-131-132-133-134-135-136-137-138-139-140-141-142-143-144-145-146-147-148-149-150-151-152-153-154-155-156-157-158-159-160-161-162-163-164-165-166-167-168-169-170-171-172-173-174-175-176-177-178-179-180-181-182-183-184-185-186-187-188-189-190-191-192-193-194-195-196-197-198-199-200-201-202-203-204-205-206-207-208-209-210-211-212-213-214-215-216-217-218-219-220-221-222-223-224-225-226-227-228-229-230-231-232-233-234-235-236-237-238-239-240-241-242-243-244-245-246-247-248-249-250-251-252-253-254-255-256-257-258-259-260-261-262-263-264-265-266-267-268-269-270-271-272-273-274-275-276-277-278-279-280-281-282-283-284-285-286-287-288-289-290-291-292-293-294-295-296-297-298-299-300-301-302-303-304-305-306-307-308-309-310-311-312-313-314-315-316-317-318-319-320-321-322-323-324-325-326-327-328-329-330-331-332-333-334-335-336-337-338-339-340-341-342-343-344-345-346-347-348-349-350-351-352-353-354-355-356-357-358-359-360-361-362-363-364-365-366-367-368-369-370-371-372-373-374-375-376-377-378-379-380-381-382-383-384-385-386-387-388-389-390-391-392-393-394-395-396-397-398-399-400-401-402-403-404-405-406-407-408-409-410-411-412-413-414-415-416-417-418-419-420-421-422-423-424-425-426-427-428-429-430-431-432-433-434-435-436-437-438-439-440-441-442-443-444-445-446-447-448-449-450-451-452-453-454-455-456-457-458-459-460-461-462-463-464-465-466-467-468-469-470-471-472-473-474-475-476-477-478-479-480-481-482-483-484-485-486-487-488-489-490-491-492-493-494-495-496-497-498-499-500-501-502-503-504-505-506-507-508-509-510-511-512-513-514-515-516-517-518-519-520-521-522-523-524-525-526-527-528-529-530-531-532-533-534-535-536-537-538-539-540-541-542-543-544-545-546-547-548-549-550-551-552-553-554-555-556-557-558-559-560-561-562-563-564-565-566-567-568-569-570-571-572-573-574-575-576-577-578-579-580-581-582-583-584-585-586-587-588-589-590-591-592-593-594-595-596-597-598-599-600-601-602-603-604-605-606-607-608-609-610-611-612-613-614-615-616-617-618-619-620-621-622-623-624-625-626-627-628-629-630-631-632-633-634-635-636-637-638-639-640-641-642-643-644-645-646-647-648-649-650-651-652-653-654-655-656-657-658-659-660-661-662-663-664-665-666-667-668-669-670-671-672-673-674-675-676-677-678-679-680-681-682-683-684-685-686-687-688-689-690-691-692-693-694-695-696-697-698-699-700-701-702-703-704-705-706-707-708-709-710-711-712-713-714-715-716-717-718-719-720-721-722-723-724-725-726-727-728-729-730-731-732-733-734-735-736-737-738-739-740-741-742-743-744-745-746-747-748-749-750-751-752-753-754-755-756-757-758-759-760-761-762-763-764-765-766-767-768-769-770-771-772-773-774-775-776-777-778-779-780-781-782-783-784-785-786-787-788-789-790-791-792-793-794-795-796-797-798-799-800-801-802-803-804-805-806-807-808-809-810-811-812-813-814-815-816-817-818-819-820-821-822-823-824-825-826-827-828-829-830-831-832-833-834-835-836-837-838-839-840-841-842-843-844-845-846-847-848-849-850-851-852-853-854-855-856-857-858-859-860-861-862-863-864-865-866-867-868-869-870-871-872-873-874-875-876-877-878-879-880-881-882-883-884-885-886-887-888-889-890-891-892-893-894-895-896-897-898-899-900-901-902-903-904-905-906-907-908-909-910-911-912-913-914-915-916-917-918-919-920-921-922-923-924-925-926-927-928-929-930-931-932-933-934-935-936-937-938-939-940-941-942-943-944-945-946-947-948-949-950-951-952-953-954-955-956-957-958-959-960-961-962-963-964-965-966-967-968-969-970-971-972-973-974-975-976-977-978-979-980-981-982-983-984-985-986-987-988-989-990-991-992-993-994-995-996-997-998-999-1000-1001-1002-1003-1004-1005-1006-1007-1008-1009-1010-1011-1012-1013-1014-1015-1016-1017-1018-1019-1020-1021-1022-1023-1024-1025-1026-1027-1028-1029-1030-1031-1032-1033-1034-1035-1036-1037-1038-1039-1040-1041-1042-1043-1044-1045-1046-1047-1048-1049-1050-1051-1052-1053-1054-1055-1056-1057-1058-1059-1060-1061-1062-1063-1064-1065-1066-1067-1068-1069-1070-1071-1072-1073-1074-1075-1076-1077-1078-1079-1080-1081-1082-1083-1084-1085-1086-1087-1088-1089-1090-1091-1092-1093-1094-1095-1096-1097-1098-1099-1100-1101-1102-1103-1104-1105-1106-1107-1108-1109-1110-1111-1112-1113-1114-1115-1116-1117-1118-1119-1120-1121-1122-1123-1124-1125-1126-1127-1128-1129-1130-1131-1132-1133-1134-1135-1136-1137-1138-1139-1140-1141-1142-1143-1144-1145-1146-1147-1148-1149-1150-1151-1152-1153-1154-1155-1156-1157-1158-1159-1160-1161-1162-1163-1164-1165-1166-1167-1168-1169-1170-1171-1172-1173-1174-1175-1176-1177-1178-1179-1180-1181-1182-1183-1184-1185-1186-1187-1188-1189-1190-1191-1192-1193-1194-1195-1196-1197-1198-1199-1200-1201-1202-1203-1204-1205-1206-1207-1208-1209-1210-1211-1212-1213-1214-1215-1216-1217-1218-1219-1220-1221-1222-1223-1224-12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CONSTRUCTION SPECIFICATIONS

SPECIAL CONDITIONS

1. A minimum cover of three (3) feet over the proposed lines is required.
2. A preconstruction meeting shall be scheduled with Botetourt County personnel, the owner, engineer and contractor.
3. No work shall begin without written approval of construction plans.
4. Work shall be subject to inspection by the County inspectors and design engineer.
5. Contractor shall be responsible for locating and uncovering all valve boxes after surface treatment of roads and adjusting boxes to final road grades, if necessary.
6. All existing utilities may be shown or may not be shown in the exact location. The contractor shall comply with the State Water Works regulations, Section 12.05.03 where lines cross.
7. The contractor shall notify the County of any field corrections to the approved plans prior to such construction.
8. All trenches within the existing or future Virginia State Department of Highways and Transportation right-of-way must be compacted in six inch layers.
9. All lines to be staked prior to construction.
10. Contractor shall maintain a set of red-line plans showing as-built data on all water and sewer lines. The contractor shall have the engineer field verify the grade on sewer lines which have grades of less than 1.0 percent. The contractor shall provide three reference distances to all water valves. Accessions to be made at one time during the laying of pipe, unless approved by the engineer prior to acceptance by Botetourt County.
11. All construction shall be in accordance to approved construction practices of the applicable trades.
12. Unless noted otherwise herein all construction shall be in accordance to the latest edition of AWWA standards.
13. Shop drawings for fabricated materials shall be submitted to Botetourt County for approval.

EXCAVATION, STABILIZATION AND BEDDING

A. TRENCHING

1. Excavation for trenches shall include the removal of all material encountered regardless of classification in accordance with the elevations and grades at the locations and stations indicated on the plans or specified herein.
2. Excavation, unless otherwise specified, shall be open cut. The Contractor shall open no more than two hundred (200) feet of trench at one time during the laying of pipe, unless approved by the Engineer.
3. Trenches shall be excavated in straight lines and shall be accurately graded in order to establish a true elevation for the invert of the pipe.
4. The width of trenches, from existing grade to one (1) foot above the top of the pipe shall be of sufficient width to permit the proper installation of bracing, shoring or shoring.
5. The sides of the trenches shall be as vertical as practical.
6. Excavation for structures shall allow a minimum of twelve (12) inches clear between the structure and the sides of the trench or any required bracing, shoring or shoring.
7. Excavated materials suitable for backfill shall be stockpiled in an orderly manner at a sufficient distance from the sides of the trench in order to avoid overloading the banks of the trench and to prevent slides or cave-ins.
8. Excavated materials which are not required or approved or backfill shall be removed from the site and disposed of by the Contractor, at his expense.
9. Contractor to adhere to all local, state and federal construction laws, including OSHA Trench Safety Regulations.

B. TRENCH STABILIZATION

1. Trench stabilization material shall be coarse aggregate size number 2 and shall conform with VDOT Section 203 and/or ASTM C 33.
2. Whenever excessively wet or unstable material is encountered in the bottom of the trench, which in the opinion of the Engineer is incapable of properly supporting the pipe or structures, such material shall be removed and backfilled with trench stabilization material and shall be graded to allow for the compacted bedding material.

3. All unauthorized overdepths of excavation shall be backfilled, at the Contractor's expense, with trench stabilization material and shall be graded to allow for the compacted bedding material.

C. COMPACTED BEDDING MATERIAL

1. Bedding material shall be coarse aggregate size Number 57 and shall conform with VDOT Section 203 and/or ASTM C 33.
2. The bottom of the pipe trench shall be excavated to a minimum overdepth of six (6) inches below the bottom of the pipe, to provide for the compacted bedding material. Bedding material shall be placed, shaped and compacted. Bedding material shall extend a minimum of six inches above the top of the pipe.
3. Bell holes and depressions required for the jointing of the pipe shall be dug after the compacted bedding material has been graded and shaped and shall be only of the length, depth and width required to make the joint properly.

PIPE, JOINTS AND FITTINGS

A. SCOPE OF WORK

1. All materials and appurtenances required for the work shall be new, or first class quality and shall be furnished, delivered, erected, connected and finished in every detail as specified or indicated. All materials found defective, regardless of the circumstances, shall be replaced with new material at the expense of the Contractor.

2. The materials specified for the construction shall comply with the latest revisions of the applicable American Society for Testing Materials (ASTM), American National Standards Institute (ANSI) and/or the Virginia Department of Transportation (VDOT) standards.

B. OPTIONAL PIPE SELECTIONS

1. The Contractor shall install only one (1) type of pipe between structures. Where existing pipe is to be replaced or extended the same type of pipe shall be installed, unless specified or indicated otherwise.
2. Water line shall be Class 52 ductile iron.
3. Sanitary sewers with an inside diameter less than or equal to twelve (12) inches shall be either polyvinyl chloride or ductile iron pipe, at the Contractor's option, unless specified or indicated otherwise. Contractor shall obtain approval of pipe material by Botetourt County Engineer prior to beginning construction.
4. Service laterals shall be either ductile iron or polyvinyl chloride pipe, at the Contractor's option, unless specified or indicated otherwise.

C. TYPES OF PIPE

1. Ductile iron pipe shall conform with AWWA C 151/ANSI 21.51 and fittings shall conform with AWWA C 110/ANSI 21.10. The pipe and fittings shall be bituminous coated and cement lined in accordance with AWWA C 104/ANSI 21.40. The pipe thickness shall conform with AWWA C 150/ANSI 21.50 and shall be Class 50, as a minimum, unless specified or indicated otherwise.
2. PVC sewer pipe and fittings shall be SDR 35 (ASTM D 3034).

D. JOINTS COUPLINGS AND APPURTENANCES

1. PVC pipe and fittings shall be bell and spigot type joints. The bell and spigot joint shall be sealed with elastomeric gaskets conforming to ASTM D 3212. The joints shall be made in strict accordance with the recommendation of the pipe manufacturer.
2. Ductile iron pipe and fittings shall be either mechanical or bell and spigot type joints as specified or indicated. Joints shall be made with a single watertight rubber gasket manufactured in accordance with AWWA C 111/ANSI 21.11. The joints shall be made in strict accordance with the recommendations of the pipe manufacturer.
3. Gate Valves shall be iron-body, bronze-mounted, double-disc, parallel-seal, O-ring sealed, inside-screw, non-rising stem, fitting with 2 inch square operating nut for valve vault service, all in accordance with AWWA Standard C500 (latest revision). Connections shall be suitable for the pipe with which it is used. The valves shall be suitable for 200 p.s.i. working pressure and shall be tested at twice the rated working pressure. All gate valves shall be installed in valve vaults and equipped with a 2-inch square operating nut. The nut shall be marked with an arrow and the word "OPEN" and shall open by turning to the right (clockwise).
4. All other materials and appurtenances to be in accordance with details shown on plans.

PIPE INSTALLATION

A. GENERAL

1. The Contractor shall not lay pipe or place manholes until all water has been removed from the trench, or when in the opinion of the Engineer, the trench or the weather conditions are unsuitable for work.
2. Pipe that may require field cutting shall be done so in a neat and workmanlike manner, so as to leave a smooth end at right angles to the axis of the pipe. Care shall be taken to avoid damaging the pipe and any coatings or linings. Ductile iron pipe shall not be cut with an oxyacetylene torch.
3. The materials shall be visually inspected for defects before lowering the pipe or placing the manholes into the trench. During the laying operation no tools, clothing or other material shall be placed in the pipe or manhole. The interior of the pipe shall be clear of all soil, debris and superfluous materials prior to and during the installation.
4. The Contractor shall exercise every precaution to prevent foreign material from entering the pipe while it is being placed in the trench. Failure by the contractor to take such precautions may result in the Engineer requiring a heavy, tightly woven canvas bag of suitable size be placed over each end of the pipe and removed only when the joint can be made properly.
5. The pipe and manholes shall be lowered carefully into the trench by suitable means and handled with care at all times to avoid damage. Under no circumstances shall the materials be dropped or dumped into the trenches.
6. When work is not in progress, the Contractor shall plug the open ends of the pipe to prevent trench water or other substances from entering the pipe. The plug shall be watertight and shall remain in place until any required dewatering has been completed.
7. Parallel Installation - Water lines shall be laid at least ten feet horizontally from a sewer or sewer manhole whenever possible. When local conditions prevent a horizontal separation of ten feet, the water line may be laid closer to a sewer or sewer manhole provided that:
 - I. The invert of the water main shall be at least 18 inches above the crown of the sewer.
 - II. Where this vertical separation cannot be obtained, the sewer shall be constructed of AWWA approved water pipe, pressure tested in place without leakage prior to backfilling.
 - III. The sewer manhole shall be of water-tight construction and tested in place.Crossing - Water lines crossing sewers shall be laid to provide a separation of at least 18 inches between the bottom of the water line and the top of the sewer whenever possible. When local conditions prevent this vertical separation, the following construction shall be used:
 - I. Sewers passing over or under water lines shall be constructed of AWWA approved water pipe, pressure tested in place without leakage prior to backfilling.
 - II. Water lines passing under sewers shall, in addition, be protected by providing:

- a) A vertical separation of at least 18 inches between the bottom of the sewer and the top of the water line.
- b) Adequate structural support for the sewers to prevent excessive deflection of the joints and the settling on and breaking of the waterline, and
- c) That the length of the water line be centered at the point of the crossing so that joints shall be equal distance and as far as possible from the sewer.

8. Before joints are made the pipe shall be well bedded on a firm foundation and no pipe shall be brought into position until the preceding length has been thoroughly embedded and secured in place. Any defects due to settlement shall be made good by the Contractor at his expense. Bell holes shall be dug sufficiently large to insure the making of proper joints.
9. Pipe shall be jointed in full accordance with manufacturer's recommendations. Push-on joints shall be thoroughly cleaned, the rubber gasket inserted in the bell socket, a thin film of approved gasket lubricant applied, the spigot end of the pipe centered into the socket and the joint completed by forcing the spigot end to the bottom of the socket by a jack-type tool or other device approved by the Engineer. Mechanical joints shall be thoroughly cleaned, the gland slipped over the spigot end of the pipe, the rubber gasket painted with soap solution and placed on the spigot end, the spigot end of the pipe seated in the bell, the gasket pressed into place within the bell, the gland moved into position, and bolts and nuts assembled by hand and tightened with an approved torque-limiting wrench.

B. INSTALLING WATER MAINS

1. The water main shall be laid and maintained at the required lines and grades with fittings and valves at the required locations.
2. Deflection of the line of pipe, in either, the vertical or horizontal plane to avoid obstructions, or in locations where long-radius curves are required, the amount of deflection shall not exceed approved AWWA standards. Alignment that may require deflections in excess of the recommended limitations, special bends, or a sufficient number of shorter lengths of pipe to provide the angular deflections within the limits as set forth, shall be approved by the Engineer.
3. All plugs, except mechanical joint plugs at connections for future lines, all tees, and all bends in water mains under pressure shall be provided with reaction backing consisting of concrete thrust blocks. Valves for connections to future lines and fire hydrants shall be anchored to the water main with the rods.
4. Water line valve delineators shall be installed by the contractor six feet off of and facing water line valves, or at other distance where necessary. Delineators will be supplied by Botetourt County Utility Department.

C. DISINFECTION OF WATER MAINS

1. All pipe shall be disinfected, tested and flushed in accordance with AWWA Standard C651 (latest revision).
2. Contractor shall provide all materials, equipment, necessary labor and perform all work required for the sterilization, testing and flushing of the water main.
3. No tested section of water line shall be approved to deliver water service until a favorable laboratory report has been achieved. Any tested section of water line failing to meet the requirements specified shall be repaired by the Contractor and retested until the results are within the limits specified.
4. The water main or valved off section that has been completed shall be filled, tested and flushed. Test locations shall be subject to the discretion of the Engineer and as valves and blow-offs permit.
5. After testing and before final inspection of the completed systems, water mains and service laterals shall be flushed and disinfected in accordance with AWWA Specifications C601 (latest revision). Flushing shall be accomplished at a flow velocity of not less than 2.5 feet per second.

Disinfection as described in AWWA C651 - "Placing of calcium hypochlorite tablets" shall be used. 5 gram calcium hypochlorite tablets with 3.25 gram available chlorine per tablet shall be attached at the inside top of the pipe by an adhesive such as Ferramex No. 1 or equal. The following number of tablets for the given pipe size shall be used for an initial dose of 25 mg/l (ppm) chlorine:	Number Tablets Per 18-20 Ft. Pipe Section
Pipe Diameter	
6"	1
8"	2
10"	3
12"	4
16"	7

or the number of tablets equal to 0.0012dL2 rounded to the next higher integer, where d is the inside diameter, in inches and L is the length of the pipe section, in feet. Use of the continuous feed or slug method of disinfecting may only be used to re-chlorinate a water pipe after the initial disinfection or in other specific cases approved by the Design Engineer. When filling the pipeline for disinfection, the rate of filling must result in a velocity of less than 1 ft./sec.

The disinfection solution shall remain in the pipe line for not less than twenty-four (24) hours, after which time a chlorine residual of 10 ppm at all parts of the line shall be required.

Following chlorination, the piping shall be thoroughly flushed. The Virginia Waterworks Regulations require at least two consecutive satisfactory bacteriological samples at 24 hour intervals from the distribution system at maximum spacing of 2000 feet before the system can be placed in service. If the initial testing is not satisfactory the new lines will be retested until satisfactory results are achieved. The Contractor shall pay all costs associated with disinfection and testing of installed facilities including any bacteriological samples and retesting if required. Samples will be collected in accordance with the Virginia Waterworks Regulations.

D. INSTALLING SEWER PIPE & MANHOLES

1. The installation of the sanitary sewer system shall begin at the downstream manhole and proceed upstream. The downstream sections shall be completed, tested and approved prior to allowing sanitary sewage to enter the system.

2. The pipe shall be installed in accordance with the pipe manufacturer's recommendations and as directed by the Engineer. The pipe shall be laid in a true straight line with the bell ends upstream and with the invert of the pipe being the true elevation and grade of the system.
3. The Contractor shall be responsible for establishing and maintaining the horizontal alignment and vertical elevation and grade of the system in accordance with the survey information indicated on the plans.
4. The horizontal alignment of the pipe shall be maintained by a transit or theodolite plumbed over the center of the downstream manhole. The vertical elevation and grade shall be maintained by not less than three (3) batter boards placed between manholes or by an adjustable laser level mounted at the invert of the downstream manhole with target(s) placed in the bell and of the pipe being laid.
5. Sewer pipe shall be installed in 6 inch gravel bedding extending a minimum of six inches above the top of the pipe and in accordance with manufacturers recommendations.
6. The sanitary sewer system shall be laid and joined complete-in-place so that each length and section of pipe between the manholes shall have a smooth and uniform invert.
7. The pipe shall be connected to manholes through precast openings and joined with either a flexible boot adapter or a pipe seal gasket.
8. Detection tape to be installed 12" to 18" above all new pipe mains and laterals.

E. CONNECTION TO EXISTING SYSTEMS

1. The new pipe connection to be made to an existing manhole where no shut or opening exists, shall be made through an opening of maximum diameter cut into the manhole wall at the required location and elevation.
2. The existing invert channels and benches shall be reworked as required to form a new flow channel from the new connection to the existing flow channel.
3. The new pipe connected to an existing manhole shall be secured in position and the remaining opening shall be filled and sealed with brick and mortar. The outer surface of the connection shall be given a coat of heavy bituminous waterproofing compound.

E. SERVICE CONNECTIONS

1. The Contractor shall make all service connections to the sewer pipe and from manholes where shown on the plans and/or where located in the field. The service connections to the sewer pipe shall be made with a wye branch fitting.
2. The wye branch fittings for service connections shall be commercially manufactured and installed in strict accordance with the recommendations of the pipe manufacturer.
3. The sewer pipe shall not be cut or tapped for service connections except when and where permitted by the Engineer.
4. All service connections shall be made with four (4) inch pipe as a minimum, unless the size of an existing service connection dictates otherwise, and shall be installed at a minimum grade of one-quarter (1/4) inch per one (1) foot from the sewer pipe or manhole to the property or easement line.
5. Future service connections shall extend to the property or easement line with cleanout and be properly capped with a watertight fitting to prevent infiltration into the sewerage system. The fitting shall be installed in strict accordance with the recommendations of the pipe manufacturer.
6. Future service connections shall be field marked by a treated, solid wooden (2 x 4) marker set vertically plumb with the end of the capped extension. The tops of the markers shall be painted green and set 24" above the finished grade. The location and invert depth of the service connection shall be shown on the as-built plans.

BACKFILLING

A. JOB CONDITIONS

1. Prior to placing backfill, all organic, rubbish debris or other unsuitable or objectionable material within the trench shall be removed. All concrete forms shall be removed. All shoring or sheeting shall be removed or cut off at the depth stipulated by the Engineer.
2. Prior to placing backfill, the trench box shall be removed. All concrete forms shall be removed. All shoring or sheeting shall be removed or cut off at the depth stipulated by the Engineer.
3. Backfill material shall be placed in uniform horizontal layers and thoroughly compacted with proper mechanical or hand operated tampers or other equipment as approved by the Engineer to perform such work. The degree of compaction of backfill material shall be tested by a soils engineer. The cost of testing to be paid by the owner. At least two compaction tests shall be performed for every 1000 feet of installed utility line.
4. Backfill material shall be placed and compacted so as to not unevenly support, damage or displace the alignment of the pipe or structures.
5. Backfill shall not be placed or compacted against cast in place concrete until it has obtained sufficient strength to withstand the backfilled pressure placed upon it.
6. Upon the completion of backfilling, all excess soil, stones and debris shall be removed from the site and disposed of by the Contractor.

B. BACKFILL MATERIAL

1. Materials for backfill shall be approved excavated material or approved suitable material obtained from other sources. All material shall be approved by a Soils Engineer.
2. Material shall consist of durable natural granular material or granular aggregates free of organic material, loam, debris, or other objectionable material which cannot be thoroughly compacted.
3. Material shall not contain stones larger in diameter than those specified herein, granite, broken concrete, masonry rubble or other material which in the opinion of the Engineer is unsuitable for backfill.

4. Excessively wet excavated material shall not be used as backfill. Frozen material shall not be placed in the trench, nor shall approved backfill be placed upon frozen material. However, backfilling may be allowed in freezing weather with prior approval of the Engineer.

C. BACKFILL BELOW UNPAVED AREAS

1. Backfill from the top of the pipe bedding or bottom of the pipe trench to one (1) foot above the top of the pipe shall be free of stones larger than two (2) inches in diameter and shall be placed in layers not to exceed six (6) inches and compacted with hand operated tampers.
2. Backfill from one (1) foot above the top of the pipe to the topsoil subgrade shall be free of stones larger than six (6) inches in diameter and shall be placed in layers not to exceed twelve (12) inches and compacted with mechanical tampers.
3. Drainage channels to be constructed of fill material shall be graded and shaped to the topsoil subgrade with material free of stones larger than four (4) inches in diameter and shall be placed in layers not to exceed eight (8) inches and compacted with mechanical tampers.

D. BACKFILL BELOW EXISTING OR NEW PAVED AREAS AND SIDEWALKS

1. Backfill from the top of the pipe bedding or bottom of the pipe trench to one (1) foot above the top of the pipe shall be free of stones larger than two (2) inches in diameter and shall be placed in layers not to exceed six (6) inches and compacted with hand tampers.
2. Backfill from one (1) foot above the top of the pipe to the pavement subgrade shall be free of stones larger than four (4) inches in diameter and shall be placed in layers not to exceed eight (8) inches and compacted with mechanical tampers.

INSPECTION AND TESTS

A. TESTING OF SANITARY SEWER

1. The Contractor shall prove the watertightness of the sewer system or portions thereof by one of the following tests, at such times as the Engineer may direct. Tests shall be made only in the presence of the Engineer. The Contractor shall furnish all labor and equipment required for the test and shall make repairs necessary until test results are satisfactory. Botetourt County Engineer shall be notified of all tests 48 hours prior to conducting such tests. All tests shall be coordinated with the Design Engineer for his attendance, observation and proper documentation.

B. AIR TEST - SANITARY SEWER LINES

The testing equipment, procedure, and results will all be subject to the strict approval of the Engineer. Results of the air test will be reviewed for compliance with ASTM designation C-828, current revision. The air test is to be conducted between two (2) consecutive manholes. The test equipment shall consist of two (2) plugs (one tapped and equipped for air inlet connection), a shut-off valve, a pressure regulating valve, a pressure reduction valve, and a monitoring pressure gauge having a pressure range from 0 to 5 psi, graduated in 0.10 psi with an accuracy of plus/minus .04 psi. The test equipment shall be set up outside the manhole for easy access and reading. Air shall be supplied to the test slowly and shall be regulated to prevent the pressure inside the pipe from exceeding 5.0 psig. The pipeline shall be filled until a constant internal pressure of 3.5 psig is maintained. The internal pressure shall be maintained at 3.5 psig or slightly above for a five (5) minute stabilization period, after which the internal pressure shall be adjusted to 3.5 psig, the air supply shut off and the test begun. No person shall remain in the manhole while pipe is being pressurized or throughout the test for safety purposes. A pressure drop of 1.0 psi from 3.5 to 2.5 psig shall be allowed for the test times specified in the following table, based upon the designated pipe size and test segment length.

AIR TEST TABLE

BASED ON EQUATIONS FROM ASTM C-828-80 SPECIFICATIONS TIME (MIN-SEC) REQUIRED FOR PRESSURE DROP FROM 3.5 TO 2.5 PSI WHEN TESTING ONE PIPE DIAMETER ONLY.

LENGTH OF TEST SEGMENT	PIPE DIAMETER, INCHES	4	6	8	10	12	15	18
25	0:04	0:10	0:18	0:28	0:40	1:02	1:29	
50	0:09	0:20	0:35	0:55	1:19	2:04	2:58	
75	0:13	0:30	0:53	1:23	1:59	3:06	4:27	
100	0:18	0:40	1:10	1:50	2:28	4:08	5:56	
125	0:22	0:50	1:28	2:18	3:18	5:09	7:26	
150	0:28	0:59	1:46	2:45	3:58	6:11	8:30	
175	0:31	1:09	2:03	3:13	4:37	7:05		
200	0:35	1:19	2:21	3:40	5:17			
225	0:40	1:29	2:38	4:08	5:40			
250	0:44	1:39	2:56	4:35			8:31	
275	0:48	1:49	3:14	4:43			9:21	
300	0:53	1:59	3:31				10:12	
350	1:02	2:19	3:47				6:16	11:54
400	1:10	2:38				6:03	9:27	13:36
450	1:19	2:50				6:48	9:38	15:19
500	1:28				5:14	7:34	1:49	17:01

Should the 1.0 psi drop occur in less time than that specified in the table, the sewer segment shall have failed. If the time required for the pressure to drop 1.0 psi is greater than that shown in the table, the sewer segment shall have passed.

For a more detailed description of the air test method refer to ASTM designation C-828, current revision. An air pressure correction shall be required when the prevailing ground water is above the sewer line being tested and shall be calculated as follows:

Ground Water Depth (H) + 3.5 = Starting Test Pressure
2.31
Ending Test Pressure = Starting Pressure - 1.0 psi

There is no change from time requirements established for the basic air test.

C. VACUUM TEST - SANITARY SEWER MANHOLES

1. All sanitary sewer manholes shall be vacuum tested.
2. Installation and operation of vacuum equipment and indicating devices shall be in accordance with equipment specifications.
3. A measured vacuum of 10 inches of mercury shall be established in the manhole. The time for the vacuum to drop to nine inches of mercury shall be recorded.
4. The elapsed time for the vacuum pressure to drop from 10 to 9 inches of mercury shall exceed the following times:

4 Ft. Dia. Manhole Depth	Minimum Time
10 ft. or less	60 seconds
>10 ft. but < 15 ft.	75 seconds
> 15 ft.	90 seconds

For 5 foot diameter manholes, add an additional 15 seconds to the listed minimum time. For 6 foot diameter manholes, add an additional 30 seconds to the listed minimum time.

5. If a manhole fails a test, repairs shall be made and the vacuum test shall be repeated.

D. MANDREL TEST

All sewer lines shall be tested by pulling a standard test mandrel between test sections.

E. TESTING OF WATER LINES

1. After placing all haunching and all valve support concrete, sufficient backfill shall be placed prior to filling the pipe with water and field testing to prevent lifting of the pipe. When local conditions require that the trenches be backfilled immediately after the pipe has been laid, the testing shall be carried out after backfilling has been completed but prior to placement of the permanent surface. At least fourteen (14) days shall elapse after the last valve support or hydrant block has been cast (Type I Portland Cement) prior to testing, unless early early strength concrete (Type III) is used, in which case three (3) days shall elapse.
2. All testing will be performed in accordance with the AWWA C600-82 or current revision.
3. Pressure Test: After the pipe has been laid, all newly laid pipe or any valved section thereof shall be subjected to a hydrostatic pressure of at least 1.5 times the working pressure at the point of testing.

Test pressure restrictions. Test pressures shall:

- a. not be less than 1.50 times the working pressure at the highest point along the test section;
- b. not exceed pipe or thrust restraint design pressures;
- c. be of at least 2-hour duration;
- d. not vary by more than + 5 psi;
- e. not exceed twice the rated pressure of the valves or hydrants when the pressure boundary of the test section includes closed gate valves or hydrants;
- f. not exceed the rated pressure of the valve.

Each valved section of pipe shall be filled with properly disinfected water slowly and the specified test pressure shall be applied by means of a pump connected to the pipe in a manner satisfactory to the Engineer.

Before applying the specified test pressure, air shall be expelled completely from the pipe, valves, and hydrants.

All exposed pipe, fittings, valves, hydrants and joints shall be examined carefully during the test. Any damaged or defective pipe, fittings, valves, or hydrants that are discovered following the pressure test shall be repeated until it is satisfactory to the Engineer.

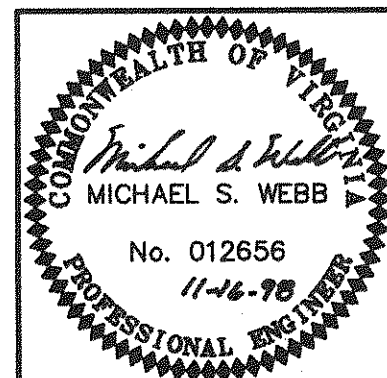
4. A leakage test shall be conducted concurrently with the pressure test. Leakage shall be defined as the quantity of water that must be supplied into the newly laid pipe, or any valved section thereof, to maintain pressure within 5 psi of the specified test pressure after the air in the pipeline has been expelled and the pipe has been filled with water. No pipe installation will be accepted if the leakage is greater than that determined by the following formula:

$$L = \frac{S \cdot D \cdot \sqrt{P}}{133,200}$$

in which L is the allowable leakage, in gallons per hour; S is the length of pipeline tested in feet; D is the nominal diameter of the pipe, in inches; and P is the average test pressure during the leakage test, in pounds per square inch gauge. When testing against closed metal-sealed valves, an additional leakage per enclosed valve of 0.0078 gal/hr/in. of nominal valve size shall be allowed. When hydrants are in the test section, the test shall be made on the basis of allowable leakage. If any test of pipe laid discloses leakage greater than the allowable amount, the Contractor shall, at his own expense, locate and repair the defective material until the leakage is within the specified allowance. All visible leaks are to be repaired regardless of the amount of leakage.

REVISION	DATE	DESCRIPTION
DESIGNED	M6W	
DRAWN	RAC	
CHECKED	M5W	
CONSTRUCTION SPECIFICATIONS FOR PHASE I "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTRAL DISTRICT BOTETOURT COUNTY, VIRGINIA		

LUMSDEN ASSOCIATES, P.C. ENGINEERS-SURVEYORS-PLANNERS ROANOKE, VIRGINIA	SCALE: NONE	COMM: 97-452
	DATE: 18 JAN. 1999	SHEET 15 of 17



PRIVATE PROPERTY

STATE RIGHT OF WAY OR EASEMENT LINE

2" X 4" PRESSURE TREATED MARKER POST

FINISHED GRADE

PAINT GREEN

24"

BRASS OR CAST IRON PLUG WITH COUNTERSUNK HEAD

PLASTIC BODY CLEAN-OUT

CLEAN-OUT TO EXTEND ABOVE GROUND A MIN. OF 1 FOOT DURING INSTALLATION OF MAIN LINE. FLUMBER SHALL CUT STACK TO FIN. GRADE & REPLACE C.O. & PLUG WHEN BUILDING SEWER IS CONNECTED TO LATERAL.

CLEAN-OUT STACK (4" MINIMUM)

45° OR 22 1/2° BELL & SPIGOT BEND (IF NEEDED)

45° WYE & REQUIRED BEND (TEE/WYE NOT ACCEPTABLE)

MINIMUM 5' STUB W/ TEMPORARY CONSTRUCTION CAP

4" MIN. SAN. SEWER LATERAL

1/4" PER 10" MINIMUM GRADE

4" MINIMUM GRAVEL BEDDING

CONNECTION TO BE MADE WITH ADAPTOR COUPLING

45° WYE FITTINGS SHALL BE SIZED AS PIPES REQUIRE

MAIN LINE

1. IN TRAFFIC AREA
2. IN TRAFFIC AREA
3. IN TRAFFIC AREA
4. IN TRAFFIC AREA
5. IN TRAFFIC AREA

The drawing consists of three views of a mechanical part:

- Isometric View (Top Left):** Shows a rectangular block with a cylindrical hole of diameter $\phi 10$ and a rectangular slot of width 10. A section line A-A is indicated.
- Side View (Top Right):** Shows the profile of the part with a total width of 45 MAX. and a section line A-A.
- Section A-A (Bottom):** A cross-sectional view showing a 45° MIN. fillet at the corner of the hole and a 45° MIN. fillet at the corner of the slot. The dimensions 2" MIN, 3", and 3" are shown.

PIPE SIZE	TEE OR DEAD END			90° BEND			45° BEND			22.5° LESS BEND		
6" OR LESS	A	B	C	A	B	C	A	B	C	A	B	C
6	1.2	5.0	1.0	1.5	3.3	2.0	1.2	2.5	1.0	1.2	2.0	1.0
8	1.7	3.8	1.5	2.0	4.5	2.0	1.3	3.6	1.5	1.3	2.0	1.0
10	2.0	4.6	1.8	2.3	4.7	2.0	1.4	3.8	1.5	1.5	2.6	1.5
12	2.5	5.8	2.0	2.8	5.5	2.5	1.7	4.6	1.8	1.8	3.0	1.8
14	3.0	6.8	2.5	3.3	6.5	3.0	2.0	5.4	2.2	2.2	3.8	2.2
16	3.5	7.0	3.0	4.5	8.0	3.5	3.0	6.8	3.0	2.5	4.0	2.5
24							3.8	7.0	4.2	4.0	4.5	2.8
30							4.0	7.8	4.5	4.2	5.0	3.0
36							4.2	8.2	4.8	4.5	5.4	3.2

NOTES:

1. THE ABOVE NOTED DIMENSIONS ARE MINIMUM BASED ON CONCRETE ANCHOR EXTENDING TO UNDISTURBED SOIL. LINE PRESSURE NOT TO EXCEED 150 PSI AND, MINIMUM SOIL BEARING CAPACITY OF 2500 PSF; UNSATISFACTORY SOIL BEARING OR EXCESS PRESSURE WILL REQUIRE ADDITIONAL DESIGN.
2. CONCRETE SHALL NOT BE POURED ON JOINTS OR BOLTS.
3. CONCRETE TO BE MINIMUM 3000 PSI - 28 DAY COMPRESSIVE STRENGTH.

SECTION A-A

Diagram illustrating the construction details of a manhole assembly, showing a cross-section (Section A-A) and a detail of the flexible boot.

Section A-A Details:

- Top: JT MATERIAL (ND MORTAR) BETWEEN F&C AND RH. TOP
- Top Ring: 2.00' diameter, 5' height, 3/4" THREADED RODS, GRADE RING IF NECESSARY
- Inner Wall: 12" MIN. height, 2.0' OR GREATER diameter
- Inner Wall Material: SDR 35
- Inner Wall Thickness: 1/2" MAX.
- Inner Wall Slope: 2'-0" CONE ECCENTRIC RISER REQUIRED
- Inner Wall Material: JT MATERIAL (ND MORTAR)
- Inner Wall Detail: CUT OUT (with 1/2" diameter hole)
- Inner Wall Detail: THREADED END CAP WITH PLUG
- Inner Wall Detail: 1" STAINLESS STEEL BAND # 24' D.C. W/ 5/8" S.S. ANCHOR BOLTS
- Inner Wall Detail: SDR 35
- Inner Wall Detail: 6" MIN. STONE BASE #57 OR EQUIVALENT
- Inner Wall Detail: 5.00' diameter

Flexible Boot Detail:

- STAINLESS STEEL BAND
- FLEXIBLE BOOT

42" 2.00' 42"

1.00' MAX.

VARIES

VARIES

VARIES

STEP

1.33' MAX.

VARIES

CONNECTION AT 0.25" / 1' MINIMUM SLOPE

SLOPE 1"

VARIES

4.00'

.50'

VALVE BOX
RECESS 1/4"

12"

12" X 12" X 6" CONC. COLLAR
3000 PSI CONCRETE

6"

PAINT BLUE

FOR VALVES ON LINES WITH 5 FEET
OR MORE COVER, USE TRENCH ADAPTER
VALVE BOXES AS MANUFACTURED BY
AMERICAN FLOW CONTROL

MAIN LINE

MINIMUM AVERAGE WEIGHT

FRAME	300
COVER	170
TOTAL	470

PLAN

3 - 1" DIA. HOLES @ 120°

MARKING OPTIONAL

SECTION - A

29 1/2"

27 1/2"

26"

24"

28"

34"

1 3/8"

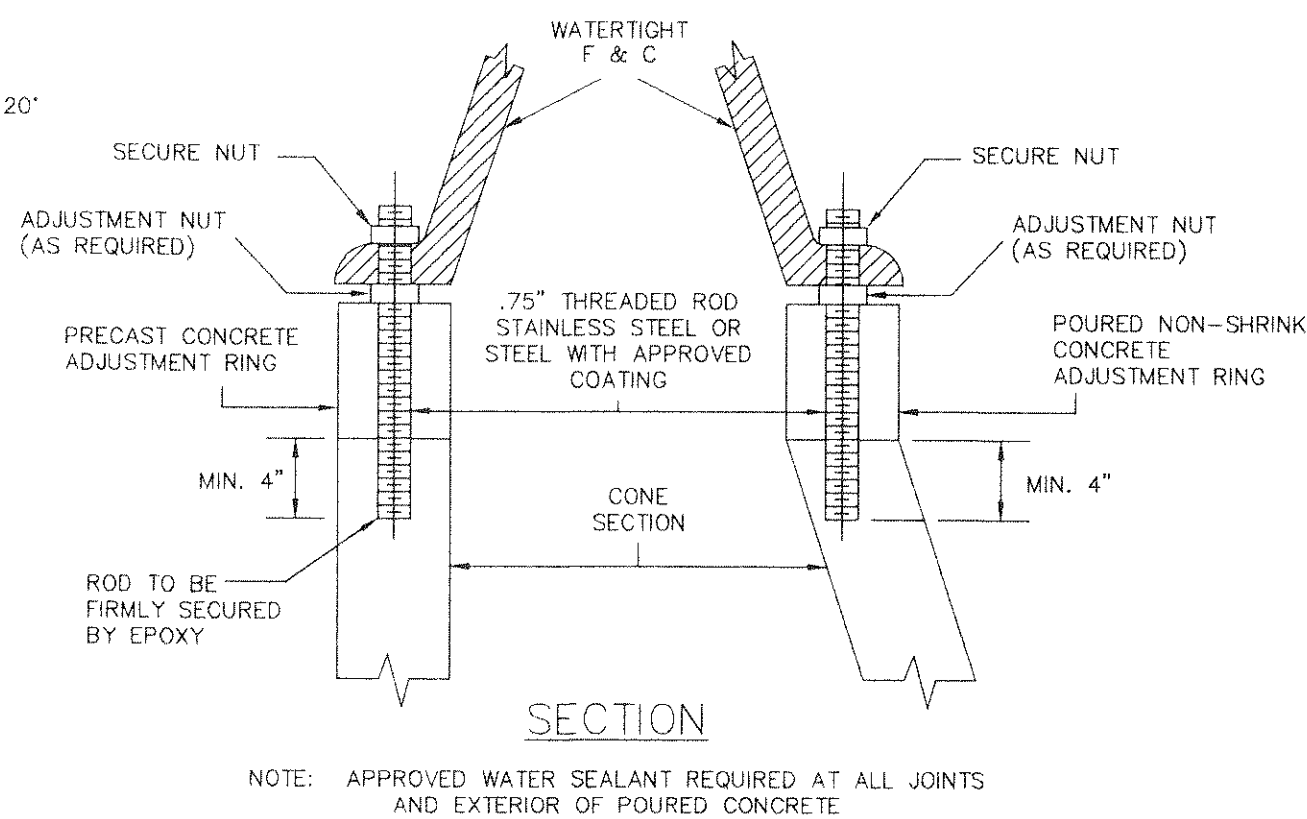
1"

7 1/2"

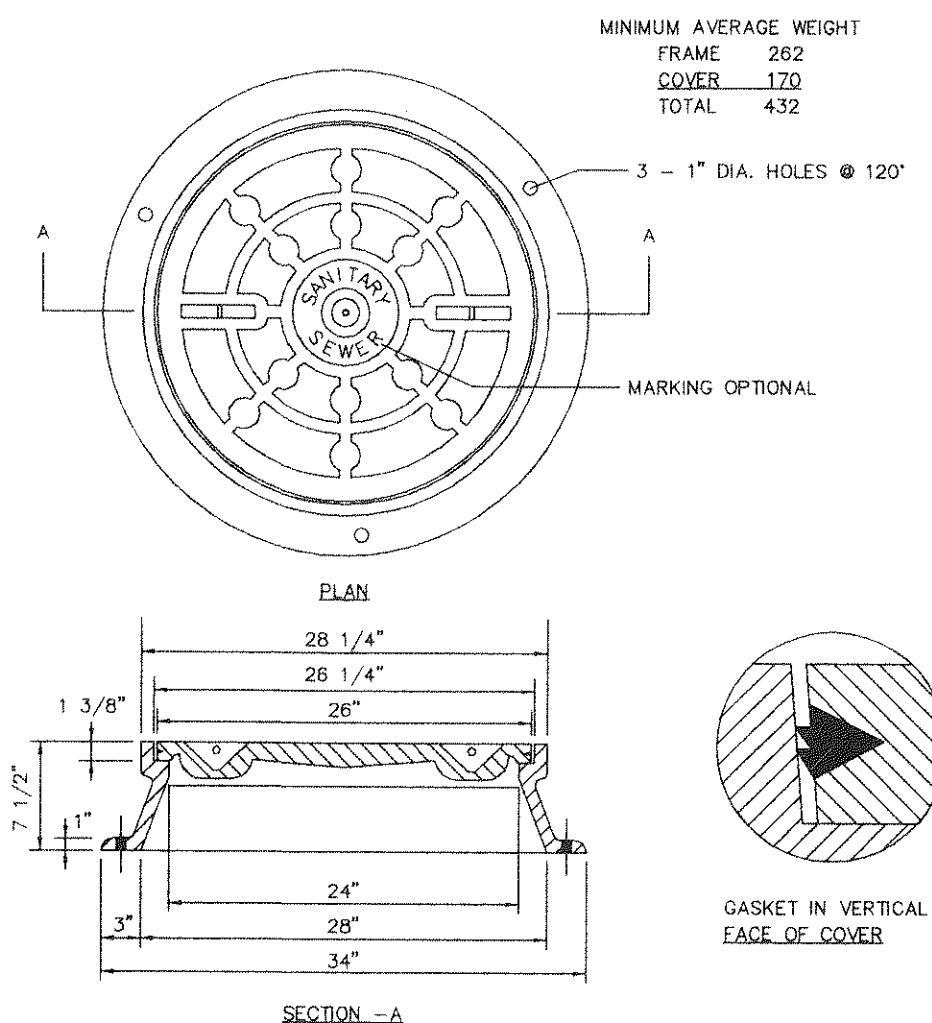
3"

GASKET IN VERTICAL FACE OF COVER & PVC INSERT

MH-RCR-3000EC
PER DEWEY BROS., INC.



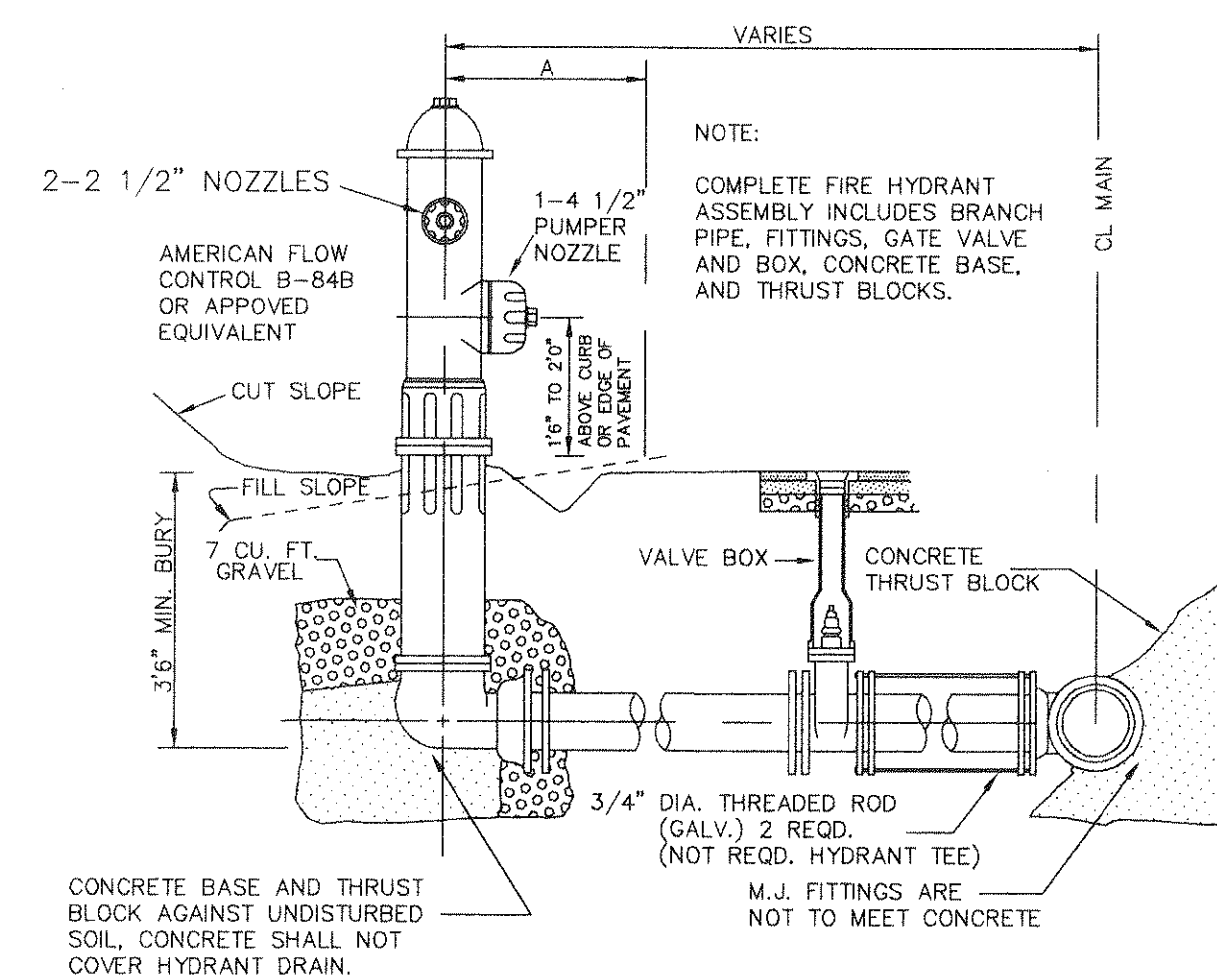
NOTE: APPROVED WATER SEALANT REQUIRED AT ALL JOINTS
AND EXTERIOR OF POURED CONCRETE



MH-RCR-3000W
PER DEWEY BROS., INC.

NOTES:

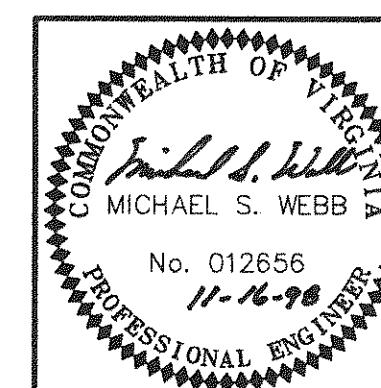
- (1) MAIN BODY OF HYDRANT MUST BE SILVER
AND HAVE RED CAPS AND BONNET, OR AS OTHERWISE
DETERMINED BY BOTEBOURNE COUNTY.
- (2) DISTANCE "A" WITH CURB AND GUTTER,
2' MIN., 4' MAX. FROM BACK OF CURB.
- (3) DISTANCE "A" WITHOUT CURB AND CUTTER,
6' MIN., 12' MAX. FROM EDGE OF PAVEMENT.
- (4) AREA AROUND HYDRANT AT A RADIUS OF 4'
TO BE APPROXIMATELY LEVEL AND UNOB-
STRUCTURED FOR ROAD CONSTRUCTION.

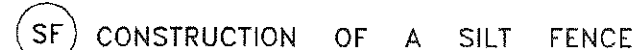


PERMANENT END OF LINE

NOTE:
ALL PIPE AND FITTINGS
TO BE BRASS.

REVISION	DATE	DESCRIPTION
DESIGNED	MSW	<p align="center">WATER & SEWER DETAILS FOR PHASE I "BOTETOURT COMMONS" PREPARED FOR TIMBERBROOK ASSOCIATES, L.L.C. AMSTERDAM MAGISTERIAL DISTRICT BOTETOURT COUNTY, VIRGINIA</p>
DRAWN	RAC	
CHECKED	MSW	
LUMSDEN ASSOCIATES, P.C. ENGINEERS - SURVEYORS - PLANNERS ROANOKE, VIRGINIA		SCALE: NONE DATE: 10 JAN, 1999
		COMM: #97-452 SHEET 16 of 17





IP GRAVEL CURB INLET SEDIMENT FILTER



IP GRAVEL AND WIRE MESH DROP INLET SEDIMENT FILTER