

Water Quantity. The project consists of constructing an approximate 8,320 SF retail building with associated site improvements that will drain to and be treated by an on-site underground infiltration basin.

The existing project site is comprised of two (2) parcels with parcel one (1) as the primary location of work totaling approximately 0.94 acres. Working being done on the second parcel is only for a shared access driveway parcel two (2) approximately 0.14 acres. Parcel one (1) contains existing vacant buildings with paved and gravel parking areas, with the remainder of the site as grass and soil. No stormwater management facilities exist onsite. The site is bordered to the east by an existing building; to the north by Orange Avenue NW (City Street); to the west by 17th Street NW; and to the south by parcel two (2) an existing vacant commercial building and a two story residential house.

The existing grade divides the site into five (5) drainage areas as shown on the Pre Development Drainage Area Map in Appendix A. Drainage Area 1 is 0.09 acres and drains uncontrolled to the north into Orange Avenue NW. Drainage Area 2 is the majority of the site with an area of 0.49 acres and drains uncontrolled to a dense group of trees to the southeast. Drainage Area 3 is 0.04 acres and drains uncontrolled to the north into Orange Avenue NW. Drainage Area 4 is 0.17 acres and drains uncontrolled to the south. Drainage Area 5 is 0.17 acres and drains uncontrolled into 17th Street NW.

The post-development site condition will include an approximate 8,320 SF retail building, 33 parking spaces, landscaping, utilities and an underground infiltration basin for quality and quantity control. The area of proposed earth disturbance will be approximately 1.20 acres.

User: DDEANNUNTIS Plot Date/Time: Sep. 30, 15 - 08:50:48 Drawing: C:\Vault_Projects\FAM-16430-Roanoke\Presentation\FAM-16430-P-SITE.dwg; DA MAPS 09

The project is considered a Redevelopment according to the City of Roanoke.

Storm CAD, Hydroflow and Hydroflow Express to calculate the existing flows leaving the site, the storage needed for the basin and the HGL's within the pipe system.

III. <u>referenc</u>es

All hydrological data used for the proposed storms was taken from the city of Roanoke Stormwater Management Design Manual. Revised July 1, 2014 Appendix 5C

All Topographic and pipe invert information was taken from the survey performed by US

IV. <u>ASSUMPTIONS</u>

Surveyors Dated 06-20-14

There are no assumptions taken with the creation of this report, all calculations and information provided meets standard engineering practices.

<u>ANALYSIS</u>

Vi. WATER QUALITY REQUIREMENTS

The stormwater quality management design requirements for the management of stormwater quality design are based on the Virginia Runoff Reduction Method in a

The water quality requirements are:

redevelopment condition.

 20% Reduction of phosphorus below the Pre-Redevleopment Load The existing phosphorus load according to the Runoff Reduction Spreadsheet updated

June 2014, is 1.45 lb. The required phosphorus reduction is 0.31 lb.

drain into an underground infiltration basin along the southern drive isle of the parking area. The stormwater facility is designed to infiltrate the 10-Year 24-hour storm by using Stormtech underground chambers and a 40% void stone bed.

The infiltration test results prepared by Terracon. Dated March 24, 2015, are provided in Appendix G, including results from the tests conducted from four (4) infiltration samples along the southern parking area drive aisle. Three (3) of the four (4) results were averaged together to calculate an infiltration rate of 0.65 in/hr. For design purposes a safety factor of two (2) was used for the design calculations; therefore, the design infiltration rate used is 0.33 in/hr. The fourth infiltration test was removed from the calculations due to its low infiltration rate and the Infiltration basin is not proposed in the location of this test.

The basin design is considered an offline design with the two forms of pre treatment being handled by the Flexstorm Pure insert for each inlet and the isolation rows within the basin itself that are wrapped in filter fabric and hold the first flush from the site (2 year storm). The isolation rows are separated from the rest of the basin by a weir inside the inflow manholes on both sides of the basin.

Required quality volume calculation:

Please see the Runoff Reduction Spreadsheet in Appendix D for detailed calculations. The Total Load Reduction Required is 0.31 lbs which is approximately a treatment volume of 3,115 cf.

Using the Runoff Reduction Spreadsheet and the Virginia BMP Clearinghouse an underground infiltration basin was selected to meet the required phosphorus removal.

According to the Virginia BMP Clearinghouse an infiltration basin #1 spec #8 has a removal rate of 25%.

The proposed method of infiltration is through an underground basin designed in accordance with the Virginia BMP Clearinghouse. Stormwater will drain into the basin through a pipe system collecting water in the parking area through grate inlets and from the building via roof drains. This pipe system will drain into the underground infiltration basin under the drive aisle of the southern parking spaces. For basin sizing and detail please see the Stormtech Chamber details in Appendix E.

The Stormtech chamber system provides 3,415 cf of storage.

Total storage provided for quality control in the underground infiltration basin exceeds the required amount of storage = 3,415 cf > 3,115 cf.

1.11 ft * 12 in / 0.33 in/hr = 40 hours < 48 hours

The proposed condition infiltrates the 10 year storm over all the proposed impervious surface and infiltrates that volume within the 48 hour requirement; therefore, the basin is compliant with the Virginia BMP Clearinghouse regulations. Due to there not being any flow leaving the basin for the 10 year storm an adequate outfall analysis of the existing storm system is not required.

The 1.11 ft height of the outfall control structure weir was generated through the use of the stormwater modeling software Hydroflow. Based on the drainage areas (shown on the drainage area maps) and the C factors along with the IDF curve data for the 10 year 24 hour storm event the amount of water that is collected inside the underground basin reaches a height of 1.11 ft. The 1.11 ft storage is also able to be infiltrated within the 48 hour dewatering requirement. The infiltration of the entire 10 year 24 hour storm was chosen to reduce any potential negative impact of the proposed site on the existing

Viv. STORMWATER QUANTITY CONTROL REQUIREMENT

The City of Roanoke refers to Section 9VAC25-870-66 of the Virginia State Code for the stormwater control requirements. The following stormwater quantity controls are

- Channel Protection:
- Manmade stormwater conveyance systems. When stormwater from a development is discharged to a manmade stormwater conveyance system following the land-disturbing activity.
- The manmade stormwater conveyance system shall convey the post development peak flow rate from the two-year 24-hour storm event without causing erosion of the system. Detention of stormwater or downstream improvements may be incorporated into the approved land-disturbing activity to meet this criterion, at the discretion of the VSMP authority.
- Flood Protection:

 Concentrated stormwater flow to stormwater conveyance systems that currently do not experience localized flooding during the 10-year 24-hour storm event: The point of discharge releases stormwater into a stormwater The five (5) individual existing drainage areas with the existing surface C-factors was placed into Hydroflow to determine the existing site release rate using the Rational Method. (Calculations sheets in Appendix A):

Rainfall amounts taken from the City of Roanoke stormwater design manual.

2 YR Pre - 3.11 CFS 5 YR Pre 3.78 CFS 4.28 CFS 10 YR Pre 25 YR Pre - 4.80 CFS 50 YR Pre - 5.22 CFS

100 YR Pre – 5.72 CFS

VivB. POST-DEVELOPMENT CONDITIONS

The post development area draining to the infiltration basin then through a storm pipe into the 17th Street NW storm system is 0.84 acres the proposed stormwater runoff to 17th Street NW was calculated by routing the proposed storm through the infiltration basin using the Rational Method and Hydroflow (calculations sheets in Appendix B):

 0.00 CFS 2 YR Post 0.00 CFS 5 YR Post 10 YR Post

 0.00 CFS 25 YR Post - 0.05 CFS 50 YR Post - 0.14 CFS 100 YR Post - 0.28 CFS

Infiltration Basin Release

Total Site Release 2 YR Post – 0.55 CFS 5 YR Post – 10 YR Post -0.62 CFS 25 YR Post - 0.70 CFS 50 YR Post - 0.76 CFS

100 YR Post - 0.83 CFS

Through the use of an underground infiltration basin, the site is able to infiltrate 3,415 cf of water within 40 hours which is larger and faster than the 3,115 cf and 48 hours required, respectively by the Virginia BMP Clearinghouse. The resultant flows leaving the project site are also less than the State of Virginia Code 9VAC25-870-66

3.11 CFS > 2 YR Post - 0.45 CFS 5 YR Pre 3.78 CFS 5 YR Post -0.55 CFS 10 YR Pre 4.28 CFS 10 YR Post _-0.62 CFS 25 YR Pre 4.80 CFS 25 YR Post -0.70 CFS 50 YR Pre 5.22 CFS 50 YR Post 0.76 CFS 100 YR Pre 5.72 CFS 100 YR Post -0.83 CFS

Based on the above, the site meets all City of Roanoke criteria for Stormwater

SITE LOCATION

1626 ORANGE

AVENUE NW

ROANOKE, VA 24017

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REVISIONS

06-01-15 PER CITY COMMENTS

P 07-15-15 PER CITY COMMENTS DP/M

DOCUMENT COMPREHENSIVE

DEVELOPMENT PLAN CP150014 PROJECT FAMILY DOLLAR IN ROANOKE, VA

COMMENT

DRAINÂGE AREA MAP & STORMWATER NIADD ADITITIC

NARRATIVES	
JOB #:	FAM-1643
DATE:	04-10-1
SCALE:	1" = 40'
DRAWN BY:	K
CHECKED BY:	A

Nov 1 0 2013

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