

UTILITY NOTES:

- 1. WHILE CERTAIN UTILITIES ARE SHOWN, OTHER UNDERGROUND UTILITIES MAY BE ENCOUNTERED DURING CONSTRUCTION. PARKER DESIGN GROUP DOES NOT WARRANT LOCATION OR DEPTH OF ANY UTILITIES SHOWN. THE CONTRACTOR SHALL COORDINATE WITH "MISS UTILITY" PRIOR TO CONSTRUCTION AND ALL UTILITY DEPARTMENTS TO DETERMINE IF THE ENCOUNTERED UTILITY IS PUBLIC OR PRIVATE AND TAKE PROPER AND ADEQUATE METHODS TO PROTECT, IF NECESSARY.
- 2. WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE GOVERNING AUTHORITIES FOR UTILITY INSTALLATIONS. ALL NEW UTILITIES SHALL BE INSTALLED UNDERGROUND.
- 3. REFERENCE IS DIRECTED TO THE COVER FOR GENERAL WATER AND SANITARY SEWER NOTES.
- 4. THE CONTRACTOR SHALL INVESTIGATE EXISTING SEWER MAINS TO VERIFY THE SIZE, LOCATION AND DEPTH OF EACH UTILITY. THE LOCATIONS SHOWN FOR ALL UTILITIES ARE APPROXIMATE AND BASED UPON AVAILABLE WVWA GIS RECORDS, VDOT RECORDS, OR THE BOUNDARY SURVEY OF 2002.

SITE SPECIFIC FILTERRA NOTES:

- THE FILTERRA UNIT PROPOSED IS BEING CONSTRUCTED ADJACENT TO EXISTING PAVEMENT. THE CONTRACTOR SHALL CONSTRUCT VDOT CG-2 CURB FROM EXISTING CURB TO THE FILTERRA STRUCTURE. THE THROAT OF THE FILTERRA STRUCTURE SHALL BE PLACED IN THE FIELD, MATCHING EXISTING PAVEMENT, AND DEPRESSING THE INLET BY 2 INCHES. RUNOFF THAT BYPASSES THE FILTERRA STRUCTURE WILL CONTINUE TO FLOW ON PAVEMENT AND LEAVE THE PAVEMENT AS IT CURRENTLY DOES. THE FILTERRA STRUCTURE SHALL NOT BE PLACED AT THE SUMP AREA OF THE PAVEMENT, BUT SHALL BE PLACED AS SHOWN ON THE PLAN. ELEVATIONS SHOWN ON THE PLANS ARE APPROXIMATE AND ARE BASED UPON VISUAL FIELD INSPECTION, AND ROANOKE CITY AERIAL TOPOGRAPHY.
- 3. SEE OTHER SHEETS FOR GENERAL AND SPECIFIC DETAILS PERTAINING TO THE FILTERRA STRUCTURE.

GENERAL DRAINAGE AND OPERATION NOTES:

- THE TANK CONTAINMENT AREA IS DESIGNED TO FUNCTION IN THE FOLLOWING MANNER:
- A CONTAINMENT WALL IS CONSTRUCTED TO WITHSTAND THE LARGEST TANK (8,000 GALLONS) PLUS THE 25-YEAR, 24-HR STORM EVENT. NO MANIFOLDS BETWEEN TANKS
- WITHIN THE CONCRETE SLAB, TWO STRUCTURES SHALL BE INSTALLED TO COLLECT RUNOFF FROM THE CONCRETE SLAB. BOTH STRUCTURES WILL BE CONTROLLED BY GATE THE GATE VALVES SHALL BE CLOSED DURING NORMAL OPERATIONS.
- WHEN A STORM EVENT OCCURS, THE CONCRETE SLAB WILL COLLECT AND STORE RUNOFF THAT FALLS WITHIN THE CONTAINMENT AREA. A SITE REPRESENTATIVE SHALL INSPECT AND TEST THE WATER COLLECTED WITHIN THE CONTAINMENT AREA. SEE SPILL PREVENTION PLAN FOR SPECIFICS.
- IF THE WATER IS CLEAR AND FREE OF OILS, THE GATE VALVE FOR THE STORM SEWER SYSTEM SHALL BE OPENED, AND ALLOW THE WATER TO DRAIN TO THE DEPRESSED AREA ADJACENT TO AERIAL WAY DRIVE. ONCE ALL WATER IS REMOVED FROM THE CONTAINMENT AREA, THE GATE VALVE SHALL BE CLOSED AND LOCKED.
- IF OILS ARE PRESENT WITHIN THE WATER, THE AREA SHALL BE CLEANED BY USE OF VACUUM TRUCK OR OTHER MEASURES AS SPECIFIED IN THE SPILL PREVENTION PLAN FOR THE FACILITY. THE REMAINING WATER SHALL BE SENT TO THE OIL/WATER SEPARATOR BY OPENING THE GATE VALVE TOWARD THE OIL/WATER SEPARATOR. ONCE ALL WATER IS REMOVED FROM THE CONTAINMENT AREA. THE GATE VALVE SHALL BE CLOSED AND LOCKED
- 2. A STRUCTURE SHALL BE INSTALLED WITHIN THE CONCRETE SLAB OF THE TRUCK FILLING AREA. ALL RUNOFF FROM THIS STRUCTURE IS DIRECTED TO THE OIL/WATER SEPARATOR.
- 3. THE PROPOSED PIPE FOR THE TANK CONTAINMENT SYSTEM IS C900 PVC PIPE (WATERLINE PIPE) TO ALLOW FOR THE INSTALLATION OF THE GATE VALVES AND PRESSURES ASSOCIATED WITH THE APPROXIMATE 4-FOOT OF HEAD UPSTREAM OF THE GATE VALVES.

Tank Cont	ainment Are	a Calculation								EVALUATIO	N OF 25-Y	r, 24-Hr Sto	rm			٠
			L. (ft)						1.	Determine the rainfall ar	nount (inche	s) of the 1-y	ear storm (A	Appendix 4B)	_
	erall Concrete		100		Perim. Concre		980.38			25-year rainfall =	6.0	inches				
Initial C	Containment V		6000		Dı	rain Top Elev.	979.75			average c=	0.90					_
Number of 8K Tanks: Number of 3K Tanks:		13							Corresponding RCN =	98		·····				
	Number	or ar tanks:		<u> </u>		ļ			 	-year depth of runoff=		(From Appe	ndiv 4C)			-
				Deduction	Reduction	Total	Net	Net	-			 ``				
				Reduction	Reduction					Area to BMP =	0.1377	acres	<u>~</u> ⇒	6,000	sq. ft.	_
	Depth (ft)	<u>.</u>	Initial	Per	Per	Reduction	Containment	Containment	2.	Determine Storm Volur	ne			1		
Datum	From	Depth (ft)	Containment	8K Tank	3K Tank	from Tanks	Volume	Volume		Vst 25	2,860	cf				_
Elevation	Perimeter	From Drain	Volume (cf)	(cf)	(cf)	(cf)	(cf)	(gallons)	3	List Largest Tank		gallons	==>	1 070	cu. ft.	-
															cu. ft.	-
979.75		0.05	000	<u> </u>				0	\						-	
980.00		0.25	298	0,0	0.0	0.0	298	2230	5.	5. Establish detention elevation using lower & upper stage-storage elevation			s & available	3		
980.38	0,00	0,63	985	19.2	13,5	343.6	641	4797	1							
980.48	0,10	0.73	1585	27.1	19.1	485.8	1099	8222		Lower Elevation =	981.08	L	wer Storag	e Volume =	3,586	,
980,58	0.20	0.83	2185	35,9	25.2	642.7 812.5	1542 1973	11537 14755		Upper Elevation =			·····		4,080	-
980.68	0.30	0.93	2785	45.4	31.8	993.9	2391	17885	<u> </u>				4,000	-		
980,78	0.40		3385	55.5°	38.9	1186.0	2799	20936	0	Interpolate the storage elevation to obtain required storage volume.				_		
980,88		1.13	3985	66,3	46.3		3197	23915		Detention Volume Elevation = 981.16 Detention W		n WQVol=	3,929			
980.98	0.60	1.23		77.6 89.4	54.2 62,3	1387.8 1598.7	3586	26826	7	Establish top of detention	n storage	981.20	10	-inch wall		
981.08	0.70 0.83	1.33 1.46		105.5	73.3	1885.1	4080		·	Calculate Freeboard ab	 	!		Feet		7
981.21	0.83	1.46	5905	100.0	13.3	1000.1	4000	30310	L	Culcumo i recoonta at	o to require	a storage	0.04	1 300	L	-

,	EVALUATION OF 25	-Yr, 24-Hr	Storm (Los	iding Area)	, .		
	Determine the rainfall an)	
	25-year rainfall =	6.0	inches				
	average c =	0.90					
	Corresponding RCN =	98					
1-	year depth of runoff=	5.72	(From App	endix 4C)			
	Area to BMP =	0.0349	acres	 >	1,520	sq. ft.	
2.	Determine Storm Volun	ne					
	Vst 25	724	cf				
3.	Failure Exposure	2,200	gallons	==>	294	cu. ft.	
4.	Combine and Calculate	Required S	torage Volu	ne;	1,019	cu. ft.	
5.	Establish detention eleva	age elevation	ıs & availabl	e volumes.			
	Lower Elevation =	980.88	L	ower Storag	ge Volume = 91		cf
	Upper Elevation =	980.98	U	pper Storag	1,042	cf	
6.	Interpolate the storage of						
·	Detention Volume I	Elevation =	980.96	980.96 Detention WQVol=			cf
7.	Establish top of detention	n storage	980.98		-inch wall		
	Calculate Freeboard ab		d storage		Feet	,	

Tank Load	mg Arou		·····			····	
			L (ft)	W (f)		<u> </u>	
Overall Concrete Dimensions:			66	20	Perim. Concre	ete Floor Elev.	980.38
Initial Containment Volume (cf/ft):			1320		D	ain Top Elev.	980.00
					Net		
	Depth (ft)		······································		Containment	<u> </u>	
Datum	From	Depth (ft)	Containment		Volume		
Elevation	Perimeter	From Drain	Volume (ci)		(gallons)		
980.00		0.00	0		0		
980.38	0.00	0.38	250		1870		
980.48	0.10	0.48	382		2857		
980.58	0.20	0.58	514		3845		
980.68	0.30	0.68	646		4832		
980.78	0.40	0.78	778		5820		
980.88	0.50	0.88	910		6807		
980.98	0.60	0.98	1042		7795	•	

DRAINAGE SUMMARY

- FILTERRA STRUCTURE SEE OTHER SHEETS FOR ALL INFORMATION PERTAINING TO THE STRUCTURE
- 6" OUTLET INV OUT = 976.00
- 3 90.1 LF 6" C900 PVC PIPE @ 1.3% SLOPE INV AT GATE VALVE = 977.17 INV OUT = 976.00
 - 6" GATE VALVE WITH FRAME & COVER CONCRETE SLAB ELEVATION = 981.0 6" INV = 977.17 PROVIDE STEM TO ALLOW ACCESS TO GATE VALVE ABOVE WATER ELEVATION OF 981.7
- 44.4 LF 6" C900 PVC PIPE @ 1.3% SLOPE INV IN = 977.75 INV AT GATE VALVE = 977.17
 - HANSON 22"X22" SQUARE INLET $\langle 4 \rangle$ TOP = 979.75 6" INV OUT = 977.75

DRAINAGE SUMMARY

- 6" OUTLET INV OUT = 975.92
- 6 49.1 LF 6" C900 PVC PIPE @ 1.0% SLOPE INV AT CLEANOUT = 976.41 INV OUT = 975.92
- 6 TRAFFIC BEARING CLEANOUT (45° HORIZ. BEND) 6" INV = 976.41
- 7 6 23.3 LF 6" C900 PVC PIPE @ 1.0% SLOPE INV LEAVING OIL/WATER SEP. = 976.64 INV AT CLEANOUT = 976.41
 - HIGHLAND TANK OIL/WATER SEPERATOR OUTLET WITH ACCESS PORT. TOP = 980.146" INV OUT = 976.64
- HIGHLAND TANK OIL/WATER SEPERATOR HIGHLAND TANK TO REVIEW PLANS AND APPROVE OILWATER SEPERATOR PRIOR TO ORDERING AND INSTALLATION.

HIGHLAND TANK OILWATER SEPERATOR INLET ⟨ 8 ⟩ WITH ACCESS PORT TOP = 980.306" INV = 976.89

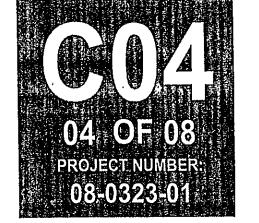
DRAINAGE SUMMARY

- 5.2 LF 6" C900 PVC PIPE @ 1.0% SLOPE \rangle INV AT GATE VALVE = 976.94 INV OUT = 976.89
- 6" GATE VALVE WITH FRAME & COVER **9** CONCRETE SLAB ELEVATION = 980.38 6" INV = 976.94 PROVIDE STEM TO ALLOW ACCESS TO GATE VALVE ABOVE WATER ELEVATION OF 981.00
- \langle 10 \rangle \sim \langle 9 \rangle 15.7 LF 6" C900 PVC PIPE @ 1.0% SLOPE ' INV IN = 977.10 INV AT GATE VALVE = 976.94
 - HANSON 22"X22" SQUARE INLET $\langle 10 \rangle$ HANGE: TOP = 980.00 6" INV IN = 977.20
 - 54.8 LF 6" C900 PVC PIPE @ 1.0% SLOPE INV IN = 977.75 / INV OUT = 977.20
 - HANSON 22"X22" SQUARE INLET TOP = 979.75 $\left\langle 11 \right\rangle \begin{array}{l} 10F = 970... \\ 6'' \text{ INV OUT} = 977.75 \end{array}$

6" INV OUT = 977.10

APPROVED

APR 27 2009



DATE: December 17, 2008

Utility

Plan

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Adress City, SPCCC, & WVW/

DESIGNED BY:

CHECKED BY:

SHEET TITLE:

DRAWN BY:

SCALE:

PJB.

MFW.

1" = 40'.

volumes.

ENGREE PER LESS FOR DES PROPERTIES

816 Boulevard

SANITARY SEWER LATERAL NOTES AND DESIGN:

THIS PROJECT NO LONGER CONNECTS TO THE EXISTING SANITARY SEWER MAIN. THE OILWATER SEPERATOR NOW DISCHARGES TO THE SURFACE.

FIRE HYDRANT NOTES AND DESIGN:

- 1. UNDER A SEPARATE REVIEW, THE CITY OF ROANOKE APPROVED A CHEMICAL FIRE SUPPRESSION SYSTEM UNDER PERMIT NO. F080025 FOR BUILDING IMPROVEMENTS. THE SITE IMPROVEMENTS TO GET THE FIRE LINE TO THE BUILDING IS BEING REVIEWED UNDER THIS PLAN.
- 2. THIS PROJECT SHALL COORDINATE WITH THE INSTALLATION OF THE PUBLIC FIRE VAULT, PUBLIC FIRE HYDRANT, AND PRIVATE FIRE LINE.
- 3. THE PROPOSED FIRE HYDRANT SHALL BE INSTALLED
- 4. THE MOST REMOTE POINT OF THE TANK CONTAINMENT AREA SHALL BE NO MORE THAN 250' FROM THE FIRE HYDRANT. AS SHOWN ON THE PLANS, THE HYDRANT IS 245' FROM THE MOST REMOTE POINT WHEN MEASURED AROUND THE WALL PERIMETER.