PROJECT NAME: Lawel Mountain

DATE: 1-94

TYPE: Santay/Sever rever

LOCATION: 415-220

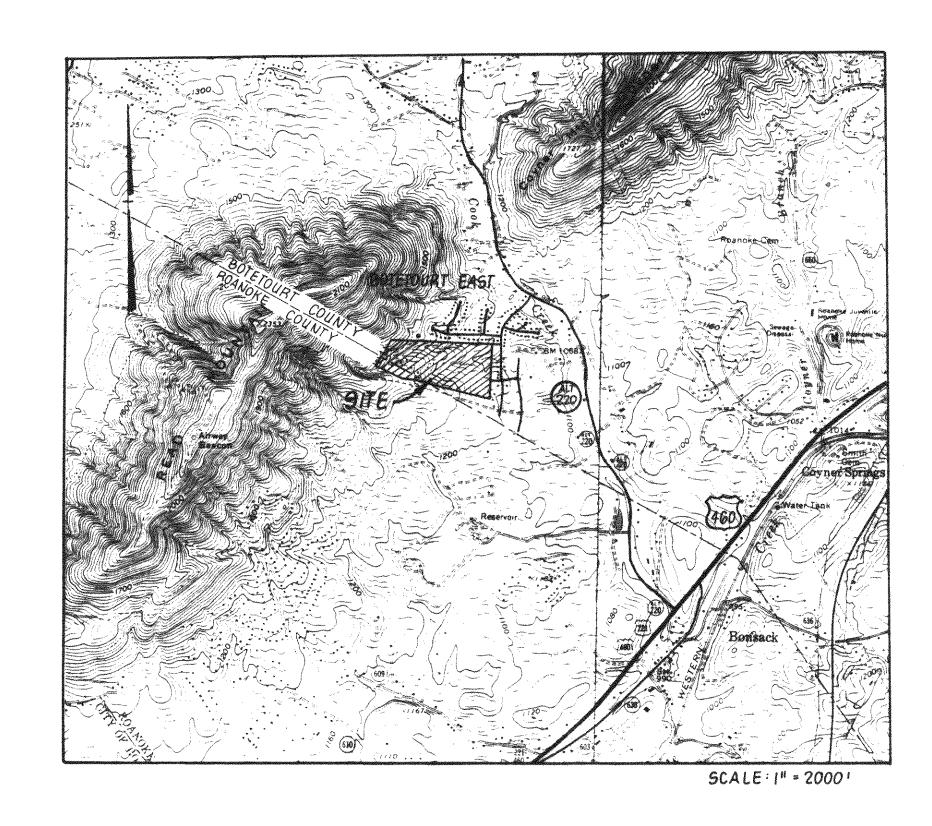
TOTAL # SHEETS: 14

A/E FIRM: LAW

OF SETS: 46

A/E Rever Set

Laurel Mountain

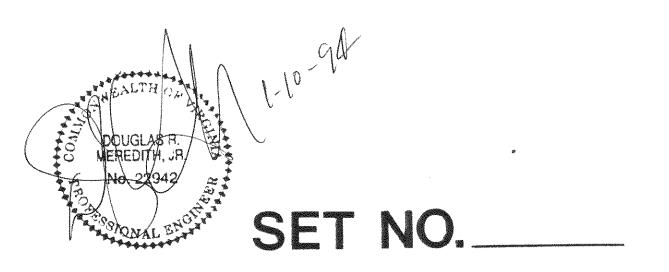


BOTETOURT COUNTY, VIRGINIA

owner
R. & R. JOINT VENTURE, INC.

RON GEIGER P.O. BOX 386 BLUE RIDGE, VIRGINIA 24064

LIMW, P.C. Engineering · Surveying



ABBREVIATIONS LEGEND EROSION AND SEDIMENT CONTROL SYMBOLS JOINT DESCRIPTION EXISTING NEW JUNCTION BOX ARANDON OR ARANDONED ABUTMEN" LENGTH. LONG EROSION CONTROL STONE ABOVE LINEAL FOOT BUILDING WITH PORCH OR STOOP ACOUSTICAL LIGHT POLE ADDITIONA. STRAW BALE BARRIER FOUNDATION ONLY LONG RADIUS 1 system white we seem assum to the seem some seems consider ABOVE FINISH FLOOR 35 - - - (6 CONTOUR, CONTOUR WITH ELEVATION STORM DRAIN AGGREGATE LIGHTING AGGR 20.0 OR X 1025 SPOT ELEVATION ANCHOR MACHINER INLET PROTECTION 20.0 E OR X.05 MASONRY ALUM AL UMINUM ALTERNATE CONCRETE CURB TEMPORARY DIVERSION DIKE MAXIMUM CONCRETE CURB & GUTTER APPROXIMATE MAI: BOX ARCHITECTURAL MECHANICAL American - 100 American 111 (100 American 100 American 10 man to make the same that the CONCRETE WALK OR SLAB AMERICAN WATER WORKS ASSOCIATION MANUFACTURER TEMPORARY SEDIMENT TRAP MANHOLE, MOUNTING HEIGHT was desirable and the distributed distributed and the distributed distributed by the distributed as a constraint of the distributed distributed as a constraint of the distributed distrPAVEMENT BITUMINOUS MINIPALIMIM TEMPORARY GRAVEL BELL JOINT MISCELL ANEOUS анничности UNPAVED OR GRAVEL ROAD BASE LINE MECHANICAL JOIN mimbou- elentrolegimus santon-revans- esanyascentes differentricines CONSTRUCTION ENTRANCE BEGIN OF BEGINNING MONUMENT CONSTRUCTION EASEMENT BUILDING Antonomierano amendratura attendratura esperante esperante procesa BLDG MOUNTED BLOCKING RIPRAP PERMANENT EASEMENT stayone divine signer some course spaces spaces control couples copyrigh BENCH MARK BEAT METAL MANNY TO THE TOTAL TO THE TOTAL TOTA TREE LINE - TYN TYN N & C MAIL AND CAP BYPASS NOT IN CONTRACT X -----X SILT FENCE TREE OR SHRUB BEARING BASEMENT NON POTABLE WATER BLACK STEEL PIPE NOT TO SCALE FENCE (PS) BUILT UP ROOF ON CENTERS PERMANENT SEEDING CENTERLINE CREEK . SWALE OR DITCH BUTTERFLY VALVE OUTSIDE DIAMETER CHANNEL . COLD OPERATOR PROPERTY LINE C/C. C CENTER TO CENTER waterway with the second OPENING CABINET OPPOSITE CENTERLINE OR BASELINE (13) POINT OF CURVE (TS)CUBIC FEET TEMPORARY SEEDING POINT OF COMPOUND CURVE LIMIT OF WORK LINE CHANGE OF GRADE CURB AND GUTTER C & C PERFORATED FIELD SURVEY TRAVERSE POINT CAST IRON PERPENDICULAR CIRC CIRCULAR P.C. OR P.T. 0 (CD) ROCK CHECK DAMS CIRCUIT POST INDICATOR VALVE CENTER LINE 0 GEOLOGIC BORE HOLE PLATE. PROPERTY LINE CLEAR PLYWOOD STORM DRAIN AND ENDWALL CONST CONSTRUCTION POINT ON LINE CORRUGATED METAL PIPE **OUTLET PROTECTION** POINT OF TANGENCY (OP)SANITARY SEWER CONCRETE MASONRY UNIT POINT ON TANGENT POINT OF REVERSE CURVE FORCE MAIN CLEANOUT POUNDS PER SQUARE INCH COMBINATION POINT OF TANGENT GAS MAIN OR SERVICE LINE CONCRETE (PORTLAND CEMENT) POLYVINYL CHLORIDE WATER MAIN OR SERVICE LINE POINT OF VERTICAL INTERSECTION CONT CONTINUOUS, CONTRO. PAVENEN" CONTRACTOR CONTR PRIVATE ELECTRICAL LINE CONV RADIUS. RISER CR STONE CRUSHED STONE UNDERGROUND TELEPHONE LINE RAILROAD CENTER REINFORCED CONCRETE PIPE CUL V UNDERGROUND ELECTRICAL LINE ROOF DRAIN, ROAD CUBIC YARD REDUCER Principle of the second PIPE FITTINGS DEPTH OR DEGREE OF CURVE RECEPTACLE DEPARTMENT RECTANGULAR - FIRE HYDRANT DRINKING FOUNTAIN . REINFORCE. REINFORCEMENT DROP INLET, DUCTILE IRON GATE VALVE DR DR RELOCATED DIMENSION REQUIRED CLEANOUT DISCOMMECT REVISION ROUTE MANHOLE RIGHT DRIVE DROP INLET (CURB AND GRATING TYPES) Approximation DOWN SPOUT SANITARY SEWER, SOUTH, STORY, SWITCH DETAIL G.M. - GAS METER, W.M. - WATER METER SANITARY DWELLING SCHEDULE TELEPHONE LINE SECTION TELEPHONE POLE, GUY AND ANCHOR E.B.L. EASTBOUND LANE POWER POLE, GUY AND ANCHOR EACH FACE EXPANSION JOINT SIMILAR LIGHT POLE SPECIFICATION ELECTRICAL ELEC SPECIFICATIONS TELEPHONE PEDESTAL ENGINEER ENGP ENTR ENTRANCE STAINLESS STEE. BURIED TELEPHONE VAULT END OF LINE STREET EDGE OF PAYEMENT STATION ABANDON OR REMOVE STANDARD EQUIPMEN" PAVED DITCH STEE EACH WAY, ENDWA Acres to the state of the state EXISTING DRIVEWAY CULVERT menter recent results are recent access regarder comment amongs of EXPARSION SURVEY The same and the s CULVERT WITH FLARED END SECTION EXTERIOR when me were some some SYMMETRICAL ØF: FLOOR DRAIN FOUNDATION 100 AND BOTTOM FLARED END SECTION MATERIALS SYMBOLS & WV WATER VALVE FINISH FLOOP TELEPHONE FINISHED FLOOR ELEVATION O H/I HUB AND TACK METAL PROFILE PLAN FIXTURE TELEVISION FLEXIBLE TYPICA. AIR RELEASE VALVE UNDERGROUND FLANCE UNLESS OTHERWISE MOTED FOOTING J.S.C.&G.S UNITED STATES COAST AN CONCRETE MASONRY GEODETIC SURVE GALVANIZE VAPOR BARRIER CONCRETE GARAGE VERTICAL CURVE **VCLUME** GROUT OR FINISHED GO/ERMMENT FIRGINIA DEPARTMENT OF TRANSPORTATION CONCRETE GALLONS PER HINUTE VERTICAL SIGHT DISTANCE WESTBOUND LANE GRATINE INSULATION GATE VALVE WIDE FLANGE. WIDE . WASTE. WATER (RIGID) WOOD BLOCKING HOSE 8188 WATER LINE HOLLOW METAL FINISHED WOOD OR PLYWOOD WATER SURFACE HOR. WATERTIGHT, WEIGHT HORSE POWER WEST FIRGINIA DEPARTMEN HIGH POINT GRAVEL OR STONE DE HIGHWATS WELDED WIRE FABRIC INSIDE DIAMETER EARTH POWER POLE IRON PIN TELEPHONE POLE Phone (703)345-0675 | 1401 2nd Street, S.W.

SHEET INDEX

SHEET NUMBER

& INDEX
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DESCRIPTION

GENERAL NOTES

THE LOCATION OF EXISTING UTILITES, INCLUDING UNDERGROUND UTILITIES IS INDICATED ON THE DRAWINGS IN SQ FAR AS THEIR EXISTENCE AND LOCATION WERE KNOWN AT THE TIME OF PREPARATION OF THESE DRAWINGS. HOWEVER, NOTHING IN THESE CONTRACT DOCUMENTS SHALL BE CONSTRUED AS A GUARANTEE THAT SUCH UTILITIES ARE IN THE LOCATION INDICATED OR THAT THEY ACTUALLY EXIST OR THAT OTHER UTILITIES ARE NOT WITHIN THE AREA OF OPERATIONS. THE CONTRACTOR SHALL MAKE ALL NECESSARY INVESTIGATIONS TO DETERMINE THE EXISTENCE AND LOCATIONS OF SUCH UTILITIES THE CONTRACTOR SHALL PAY FOR ANY DAMAGE TO AND FOR MAINTENANCE AND PROTECTION OF EXISTING UTILITIES AND STRUCTURES.

EXISTING WATER LINE LOCATIONS BOTH HORIZONTAL AND VERTICAL ARE APPROXIMATE. THE LOCATION IS NOT THE RESULT OF A FIELD SURVEY.

THE CONTRACTOR IS DIRECTED TO DIG AND LOCATE ALL UTILITIES IN ADVANCE OF PIPELAYING TO ALLOW FOR ADJUSTMENTS DUE TO CONFLICTS WITH EXISTING UTILITIES. SHOULD A CONFLICT ARISE THE ENGINEER IS TO BE NOTIFIED IMMEDIATELY.

THE CONTRACTOR IS REQUIRED TO NOTIFY "MISS UTILITY" AT 1 800-552-7001 AT LEAST TWO, BUT NOT MORE THAN TEN WORKING DAYS IN ADVANCE OF CONSTRUCTION.

SURVEY INFORMATION OBTAINED FROM TOPOGRAPHIC SURVEY COMPLETED IN JUNE, 1993 BY LMW, P.C. ENGINEERS - SURVEYORS.

LMW, PC. Engineering - Surveying

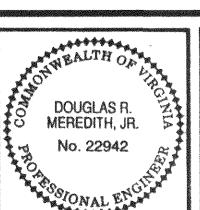
Fax (703)342-4456

Roanoke, Virginia 24016

Laurel Mountain

ABBREVIATIONS, LEGEND, GENERAL NOTES, & INDEX

BOTETOURT COUNTY, VA.



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VIRGINIA DEPARTMENT OF TRANSPORTATION NOTES:

QUALITY CONTROL

STREETS TO BE GRADED, PAVED AND ALL STRUCTURAL COMPONENTS ERECTED IN ACCORDANCE WITH CURRENT VIRGINIA DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE SPECIFICATIONS AND ROAD DESIGN STANDARDS, BOTETOURT COUNTY & ROANOKE COUNTY DESIGN STANDARDS AND SPECIFICATIONS. ALL MATERIALS USED SHALL BE TESTED IN ACCORDANCE WITH STANDARD POLICIES. THE DEVELOPER MUST CONTACT THE OFFICE OF THE RESIDENT ENGINEER, PRIOR TO BEGINNING ANY CONSTRUCTION AT WHICH TIME AN INSPECTION AND TESTING PROCEDURE POLICY WILL BE DRAWN. THE DETELOPER WILL PRODUCE TEST REPORTS FROM APPROVED INDEPENDENT LABORATORIES AT THE DEVELOPER'S

THE PAVEMENT DESIGNS SHOWN ARE BASED ON A SUBGRADE CBR /ALUE OF 10 OR GREATER. THE SUBGRADE SOIL IS TO BE TESTED BY AN INDEPENDENT LABORATORY AND THE RESULTS SUBMITTED TO THE VIRGINA DEPARTMENT OF TRANSPORATION PRIOR TO BASE CONSTRUCTION. SHOULD THE SUBGRADE OBR VALUED BE LESS THAN 10, THEM ADDITIONAL BASE MATERIAL WILL BE REQUIRED IN ACCORDANCE WITH DEPARTMENTAL SPECIFICATIONS.

THE SUBGRADE MUST BE APPROVED BY VIRGINIA DEPARTMENT OF TRANSPORTATION PPIOR TO PLACEMENT OF THE BASE. BASE MUST BE APPROVED BY VIRGINIA DEPARTMENT OF TRANSPORTATION FOR DEPTH, TEMPLATE AND COMPACTION BEFORE CURFACE IS APPLIED.

ALL NECESSARY UTILITY LATERALS ALONG WITH PROVISIONS FOR DONDUITO (NE. WATER, DEWER, STORM, BAS AND TELEPHONE) WILL BE CONSTRUCTED PRIOR TO PLACEMENT OF BASE MATERIAL.

BAC OR PETROLEUM TRANSMISSION LINED WILL NOT BE PERMITTED WITHIN THE PAVEMENT OR CHOULDER ELEMENT (BACK OF CURB TO BACK) OF CURB) OF THIS DEVELOPMENT. SERVICE LATERALS CROSSING AND PIPE LINES LOCATED OUTSIDE THE PAVEMENT BUT INSIDE THE RIGHT OF WAY WILL BE CONSTRUCTED IN CONFORMITY WITH ASA B 31.8 SPECIFICATIONS AND CAFETY REGULATIONS. DISTRIBUTION LINES WITH PRECSURES LESS THAN 120 LBS. ARE UNAFFECTED BY THE ABOVE.

PERMITS WILL BE REQUIRED FOR ALL UTILITIES WITHIN STREET RIGHT OF WAY PRIOR TO ACCEPTANCE INTO THE SECONDARY HIGHWAY SYSTEM.

ANY EASEMENTS GRANTED TO A UTILITY COMPANY FOR PLACEMENT OF POWER, TELEPHONE, ETC. MUST BE RELEASED PRIOR TO ACCEPTANCE.

PRIVATE ENTRANCES

MODIFIED OG-9D GUTTER MILL BE PROVIDED AT ALL ENTRANCES TO PRIVATE LOTS WHERE STANDARD OG-8 SURB AND GUTTER IS APPROVED FOR

DRIVEWAYS CONNECTING TO ROADS WITHOUT OURB & GUTTER CHALL CONFORM TO THE PAVEMENT, CHOULDER & SLOPE.

PERMITS WILL BE REQUIRED FOR ALL PRIVATE ENTRANCES CONSTRUCTED IN STREET RIGHTS OF WAY AFTER ACCEPTANCE INTO THE SECONDARY HIGHWAY SYSTEM.

EROSION CONTROL AND LANDSCAPING

CARE MUST BE TAKEN DURING CONSTRUCTION TO PREVENT EROSION, DUST AND MUD FROM DAMAGING ADJACENT PROPERTY, CLOGGING DITCHES, TRACKING PUBLIC STREETS AND OTHERWISE CREATING A PUBLIC OR PRIVATE NUISANCE TO SURROUNDING AREAS.

THE ENTIRE CONSTRUCTION AREA INCLUDING DITCHES, CHANNELS, BACK OF CURBS AND OR PAYEMENT ARE TO BE BACKFILLED AND SEEDED. AT THE EARLIEST POSSIBLE TIME AFTER FINAL GRADING.

DRAINAGE EASEMENTS MUST BE DEFINED BY EKCAVATED DITCHES OR CHANNELS FOR THEIR FULL LENGTH TO WELL DEFINED EXISTING NATURAL WA TERCOURSES.

THE ROAD WILL BE REVIEWED DURING DONSTRUCTION FOR THE MEED OF PAVED DITCHED. OF EROSION IS ENCOUNTERED IN ANY DRAINAGE. EASEMENT, IT WILL BE THE RESPONSIBILITY OF THE DEVELOPER TO SCD, RIP RAP, GROUT, PAVE OR TO DO WHATEVER IS NECESSARY TO CORRECT THE PROBLEM.

ALL VEGETATION AND OVERBURDEN. TO BE REMOVED FROM SHOULDER TO SHOULDER PRIOR TO THE CONDITIONING (CUTTING AND/OR PREPARATION; OF THE SUBGRADE.

5. INTERSECTION PAVEMENT RADIUS

MINIMUM PAVEMENT RADIUS OF 25 FEET IS REQUIRED AT ALL STREET INTERSECTION.

CONNECTIONS TO STATE-MAINTAINED ROADS

WHILE THESE PLANS HAVE BEEN APPROVED, SUCH APPROVAL DOES NOT EXEMPT CONNECTIONS WITH EXISTING STATE-MAINTAINED ROADS FROM CRITICAL REVIEW AT THE TIME PERMIT APPLICATIONS ARE MADE. THIS IS NECESSARY IN ORDER THAT THE PREVAILING CONDITIONS BE TAKEN INTO CONSIDERATION REGARDING SAFETY ACCOMPANIMENTS SUCH AS TURNING LANES.

GUARDRAILS STANDARD GUARDRAIL WITH SAFETY END SECTIONS MAY BE REQUIRED ON FILLS AS DEEMED NECESSARY BY THE RESIDENT ENGINEER. AFTER COMPLETION OF ROUGH GRADING OPERATIONS, THE OFFICE OF THE RECIDENT ENGINEER, SHALL BE NOTIFIED SO THAT A FIELD REVIEW MAY BE MADE OF THE PROPOSED LOCATIONS.

WHERE GUARDRAILS ARE TO BE INSTALLED THE SHOULDER WIDTH SHALL BE "ICREASED IN ACCORDANCE WITH VOOT ROAD AND BRIDGE STANDARDS.

STORM DRAINAGE

FIELD REVIEW WILL BE MADE DURING CONSTRUCTION TO DETERMINE THE NEED AND LIMITS OF PAVED DITCHES AND FOR DITCH STABILIZATION TREATMENTS, AND TO DETERMINE THE NEED AND LIMITS OF ADDITIONAL DRAINAGE EASEMENTS. ALL DRAINAGE EASEMENTS MUST BE CUT AND MADE TO FUNCTION TO A NATURAL WATERCOURSE. ANY EROSION PROBLEMS ENCOUNTERED IN AN EASEMENT MUST BE CORRECTED BY WHATEVER MEANS NECESSARY PRIOR TO SUBDIVISION ACCEPTANCE.

DITCH SLOPES ARE TO BE FOUR TO ONE (4:1) FOR SHOULDER WIDTHS OF SIX FEET (6') OR GREATER AND THREE TO ONE (3:1) FOR SHOULDER WIDTHS OF FOUR FEET (4') OR FIVE FEET (5'), UNLESS THERWISE SPECIFIED IN THE PLANS.

ENTRANCE PERMIT

CONTRACTOR SHALL OBTAIN ENTRANCE PERMIT TO THE EXISTING VIRGINIA DEPARTMENT OF TRANSPORTATION RIGHT OF WAY FROM RESIDENT ENGINEER PRIOR TO ROAD CONSTRUCTION.

10. INSPEDTION

AN INSPECTOR WILL NOT BE FURNISHED EXCEPT FOR PERIODIC PROGRESS INSPECTION, THE ABOVE MENTIONED FIELD REVIEWS AND CHECKING FOR REQUIRED STONE DEPTHS. THE DEVELOPER WILL BE REQUIRED TO POST A SURETY TO GUARANTEE THE ROAD FREE OF DEFECTS FOR ONE YEAR AFTER ACCEPTANCE BY THE DEPARTMENT OF TRANSPORTATION.

STREET MAINTENANCE

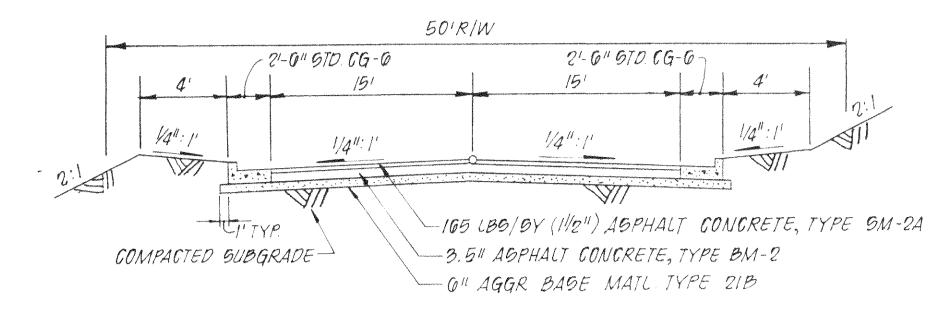
THE STREETS MUST BE PROPERLY MAINTAINED UNTIL ACCEPTANCE. AT SUCH TIME AS ALL REQUIREMENTS HAVE BEEN MET FOR ACCEPTANCE, ANOTHER INSPECTION WILL BE MADE TO DETERMINE THAT THE STREET HAS BEE'T PROPERLY MAINTAINES.

UNDERGROUND UTILITIES

CONTRACTOR SHALL VERIFTY LOCATION AND ELEVATION OF ALL UNDERGROUND STILITIES SHOWN ON THE PLANS IN AREAS OF CONSTRUCTION PRIOR TO STARTING WORK BY JONTACTING WISS UTILITY. JONTACT SITE ENGINEER MMEDIATELY F LOCATION OR ELEVATION IS DIFFERENT FROM THAT SHOWN ON THE PLANS. IF THERE APPEARS TO BE A CONFLICT, AND UPON DISCOVERY OF ANY UTILITY NOT SHOWN ON THIS PLAN, CALL "MISS UTILITY" OF CENTRAL MRGINIA AT 1-800-552-7001.

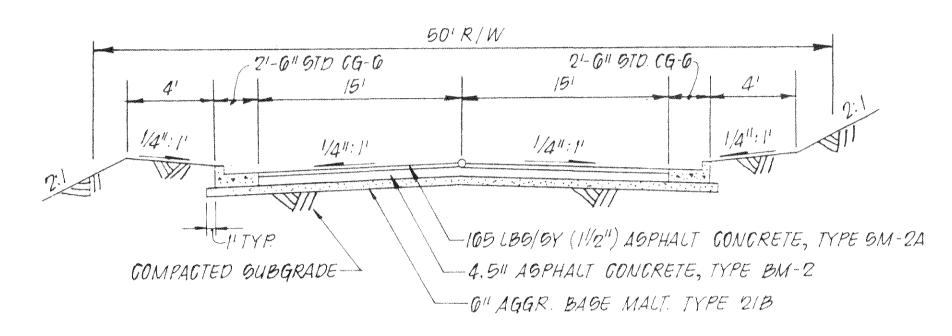
13. REVISIONS OF SPECIFICATIONS AND STANDARDS

APPROVAL OF THESE PLANS WILL BE BASED ON SPECIFICATIONS AND STANDARDS IN EFFECT AT THE TIME OF APPROVAL AND WILL BE SUBJECT, UNTIL COMPLETION OF THE ROADWAY AND ACCEPTANCE BY THE DEPARTMENT, TO FUTURE REVISIONS OF THE SPECIFICATIONS AND STANDARDS.



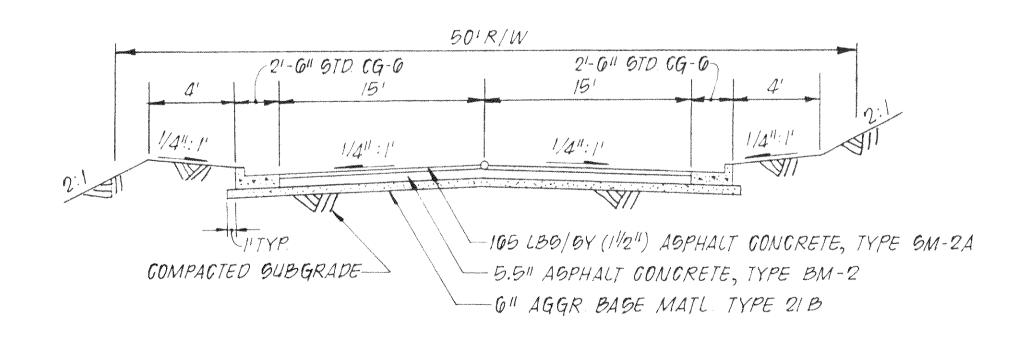
CATEGORY I

6TA 10+15.00 TO STA 13+10.00 ROAD 'B'



CATEGORY II

6TA 12+50.00 TO 6TA. 40+90.00 RAY 6TREET



CATEGORY III

6TA. 7+98.47 TO GTA. 12+50.00 RAY GTREET 9TA, 10+15,00 TO 9TA, 15+50.00 ROAD 'A'

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Phone (703)345-0675 | 1401 2nd Street, S.W.

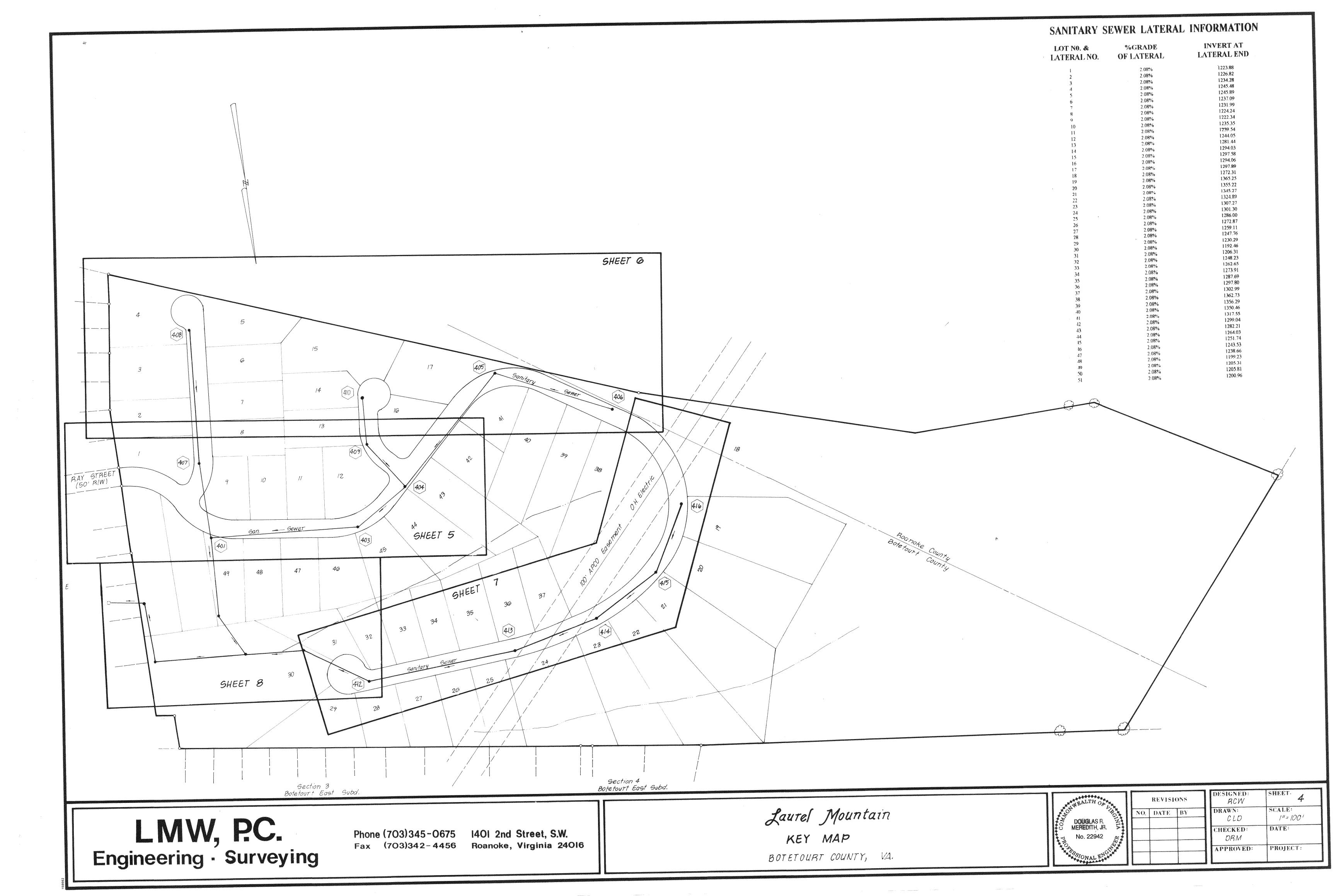
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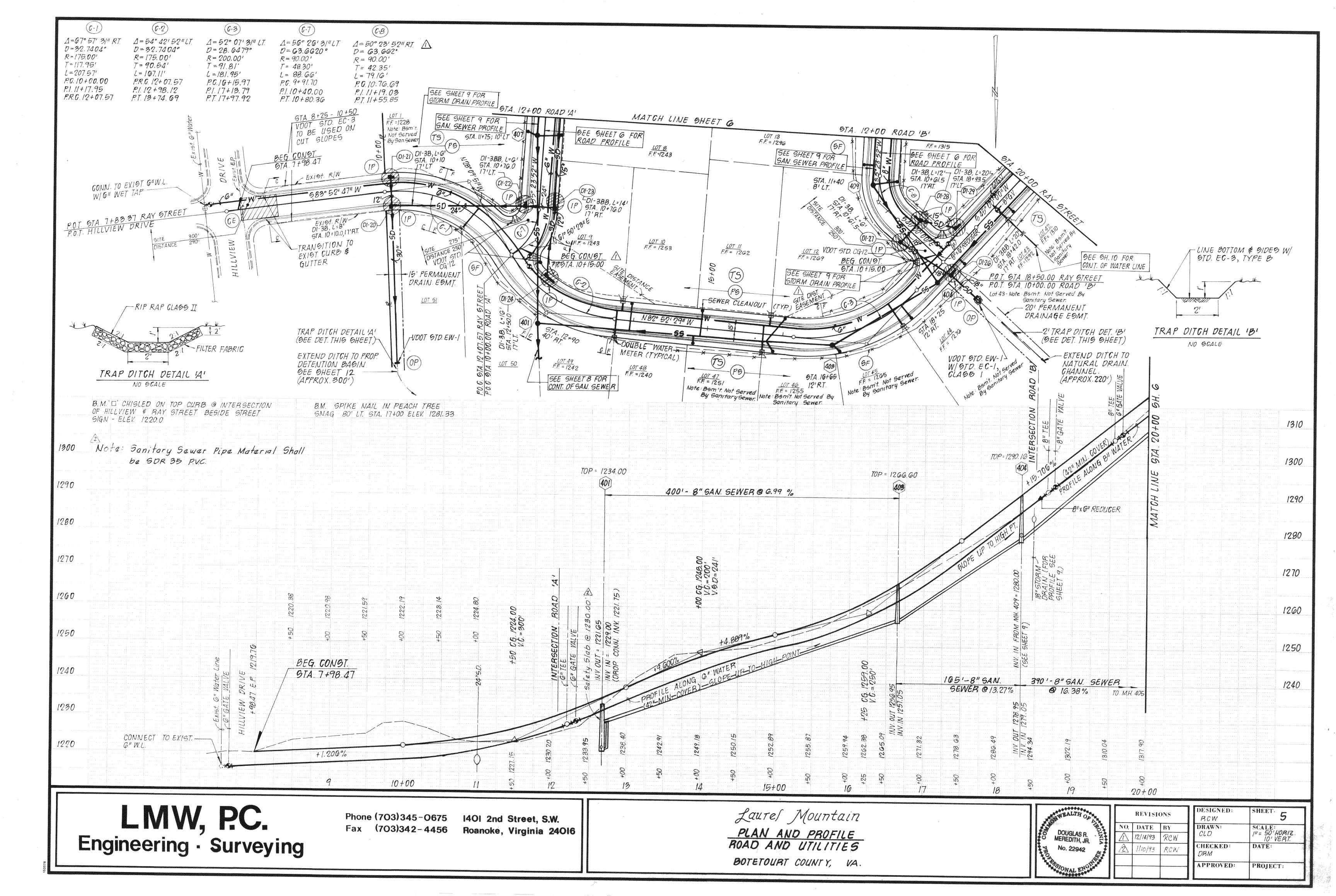
ROADWAY

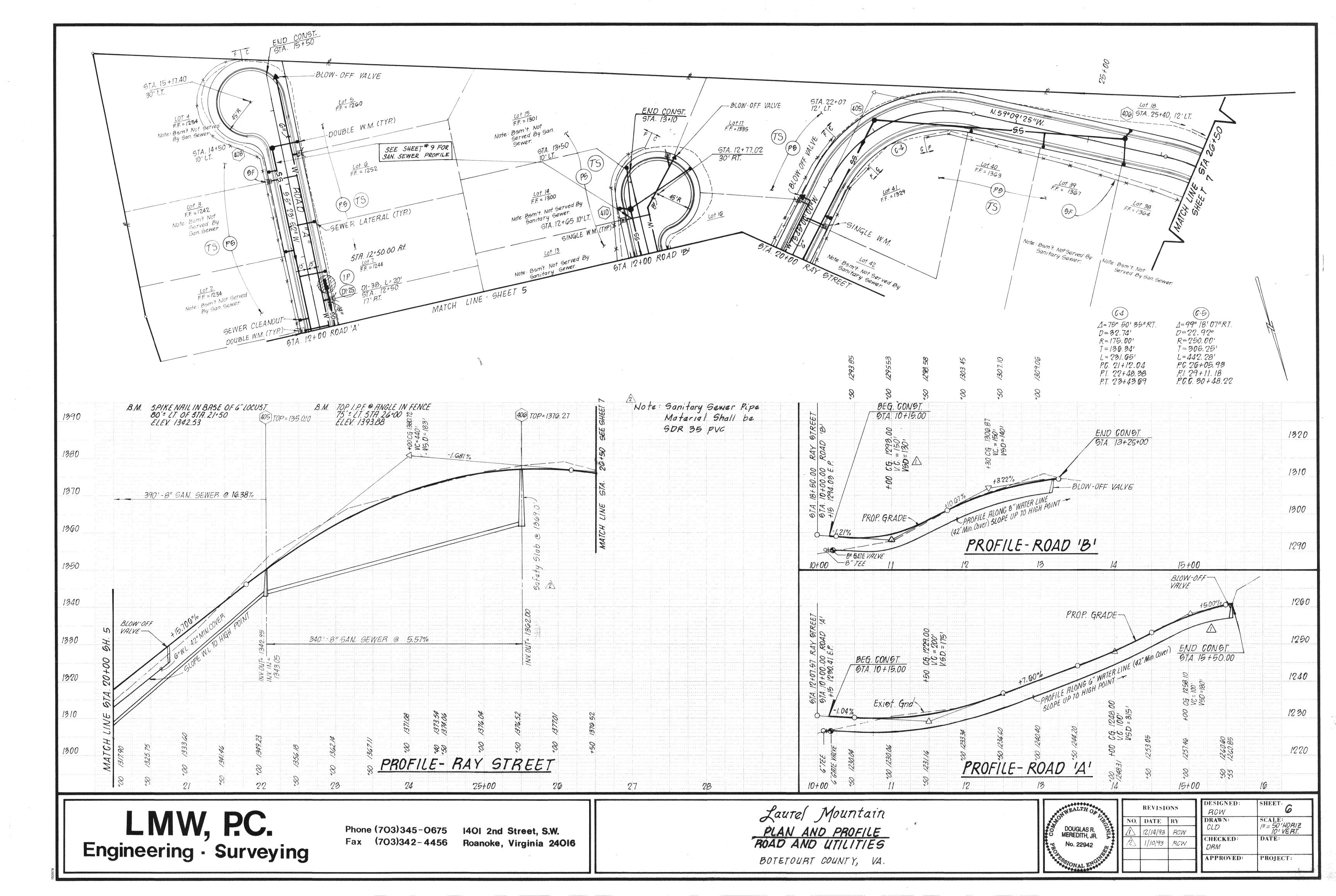
Laures Mountain CONSTRUCTION AND GENERAL NOTES

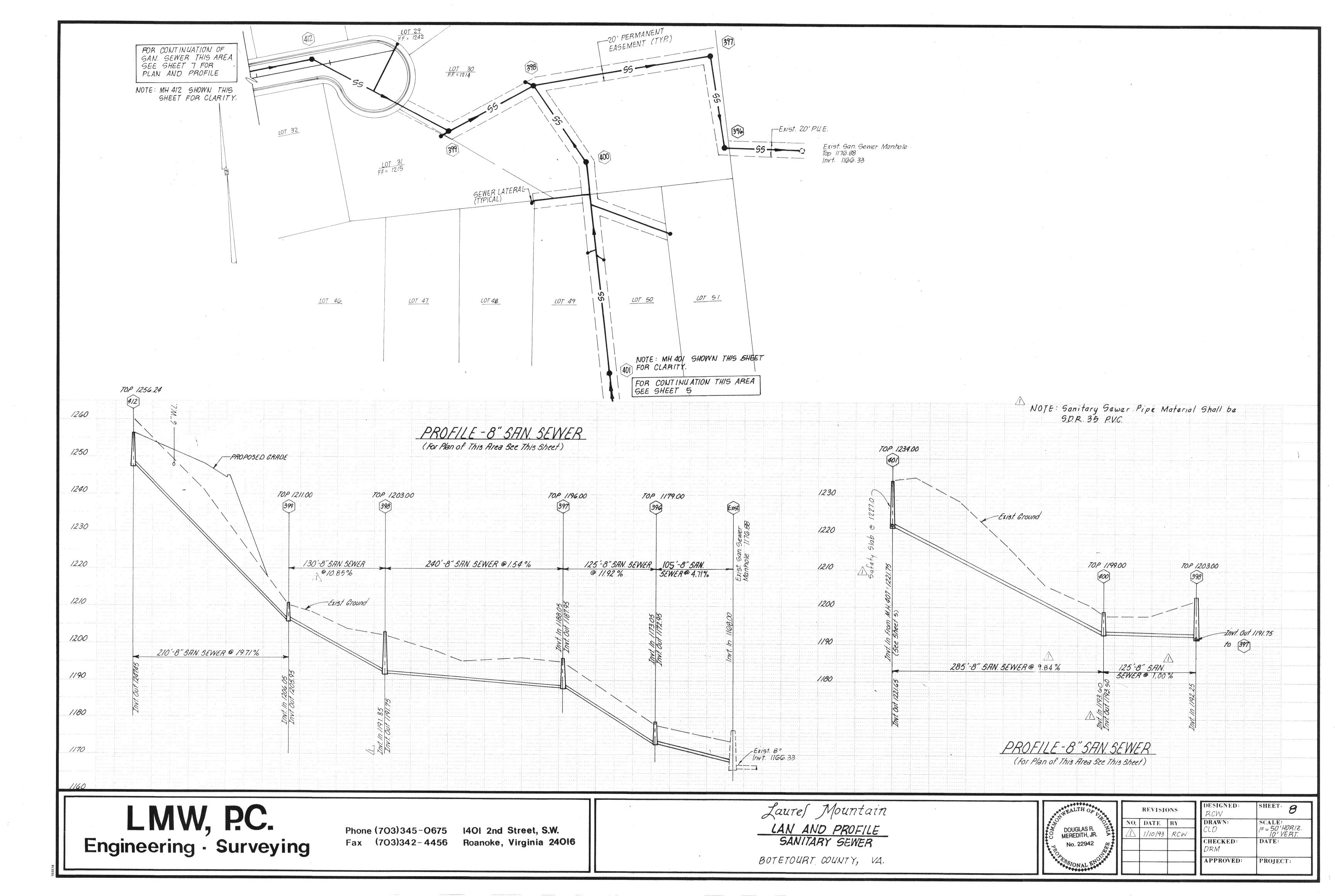
BOTETOURT COUNTY, VA.

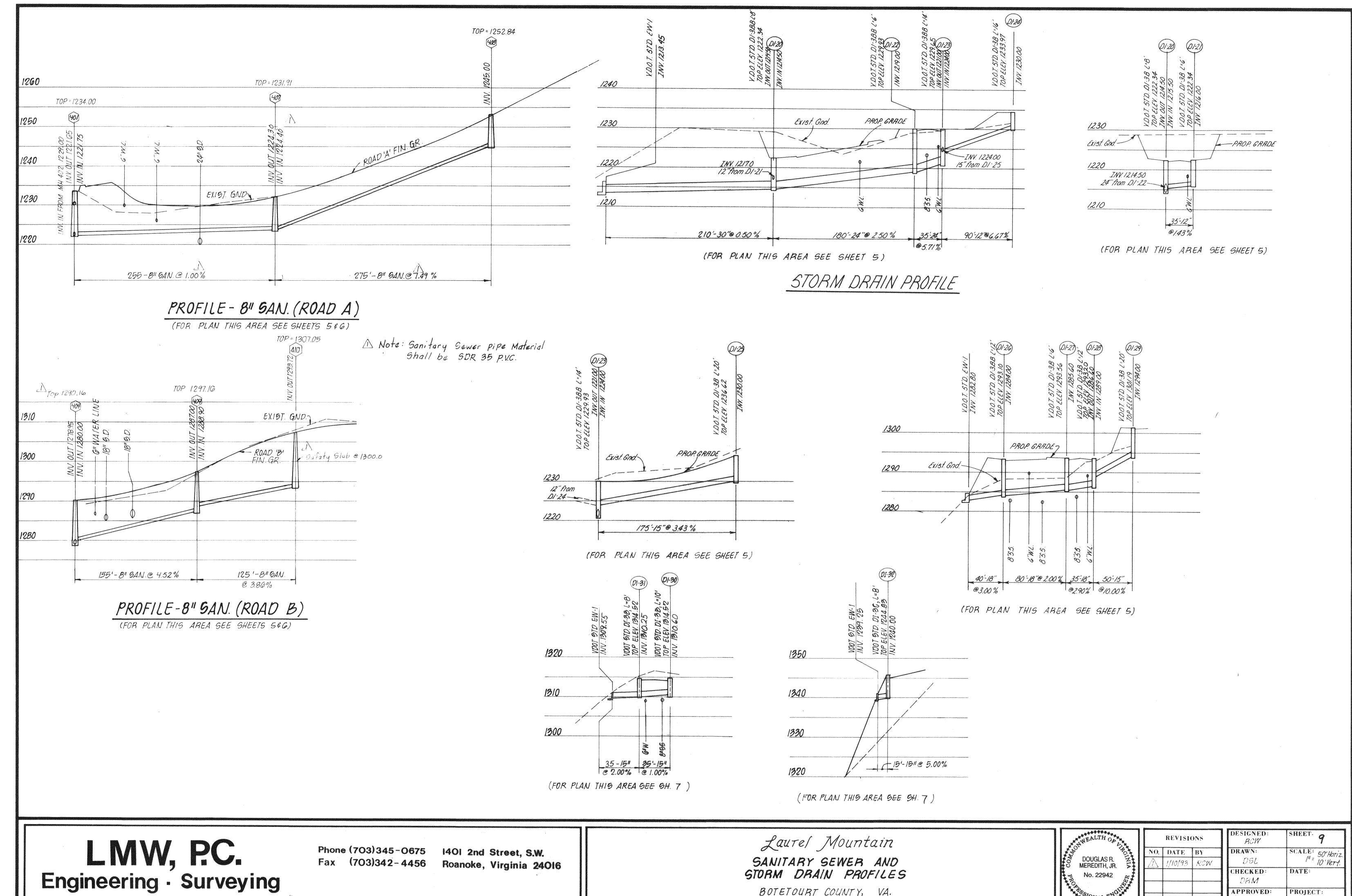
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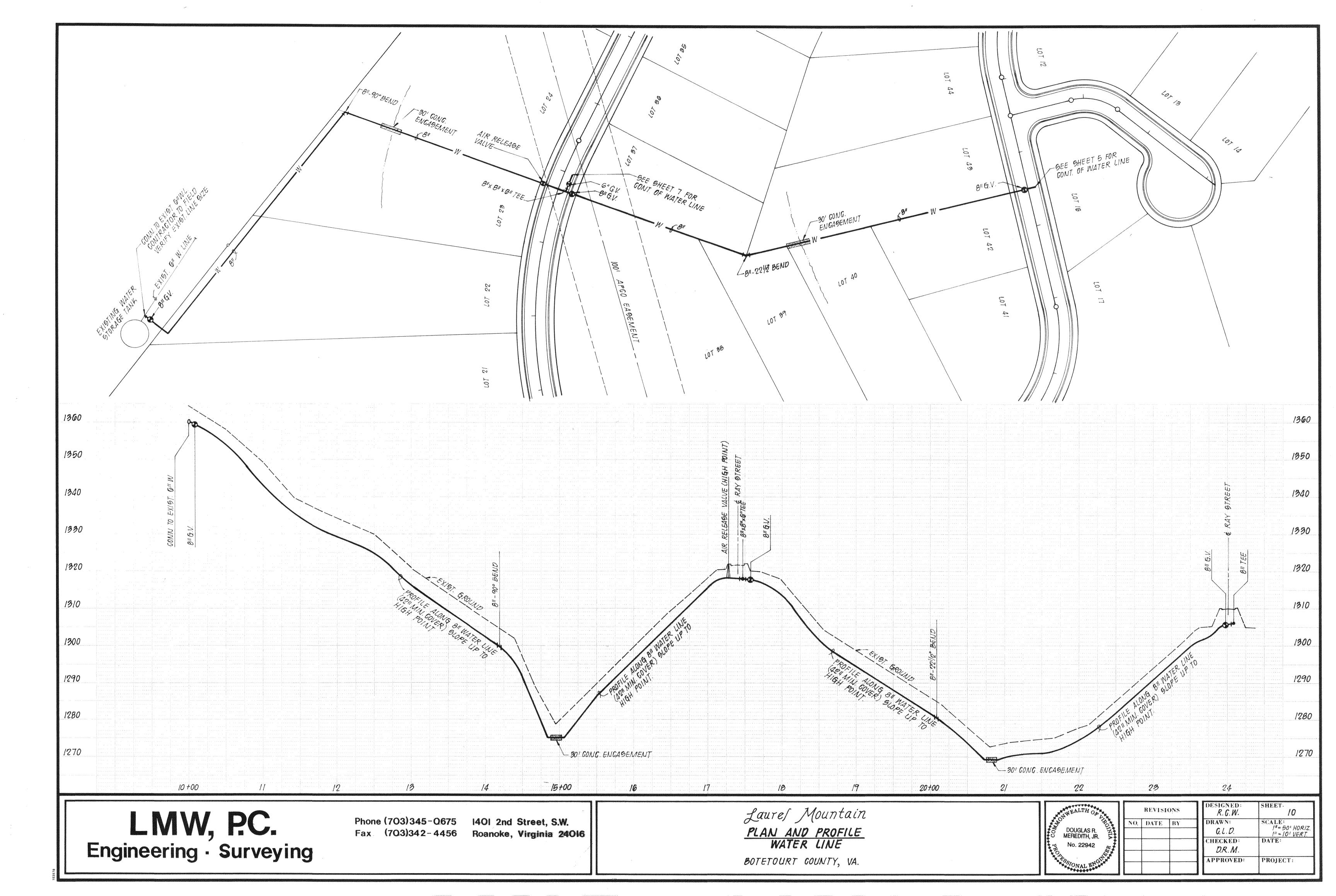


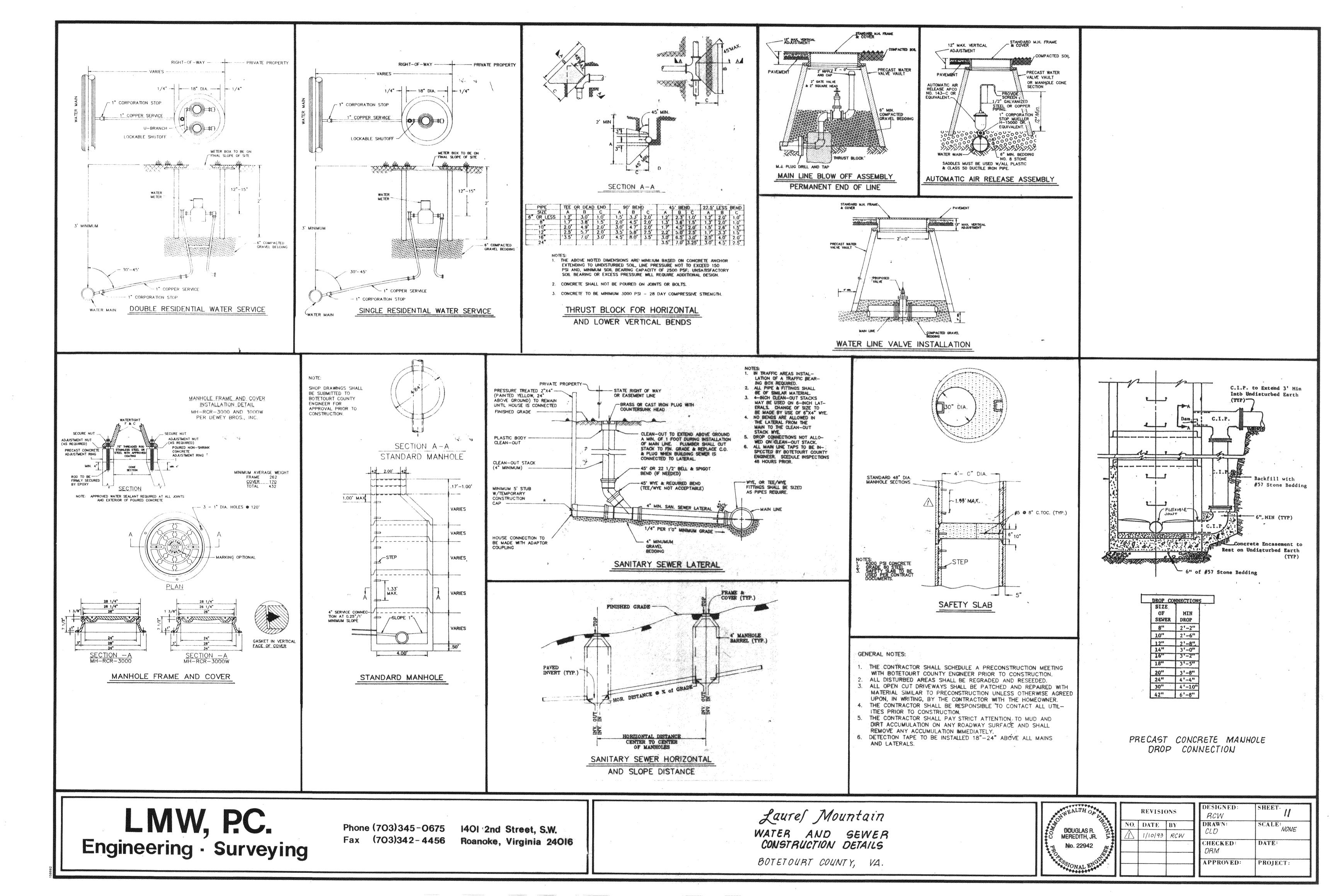


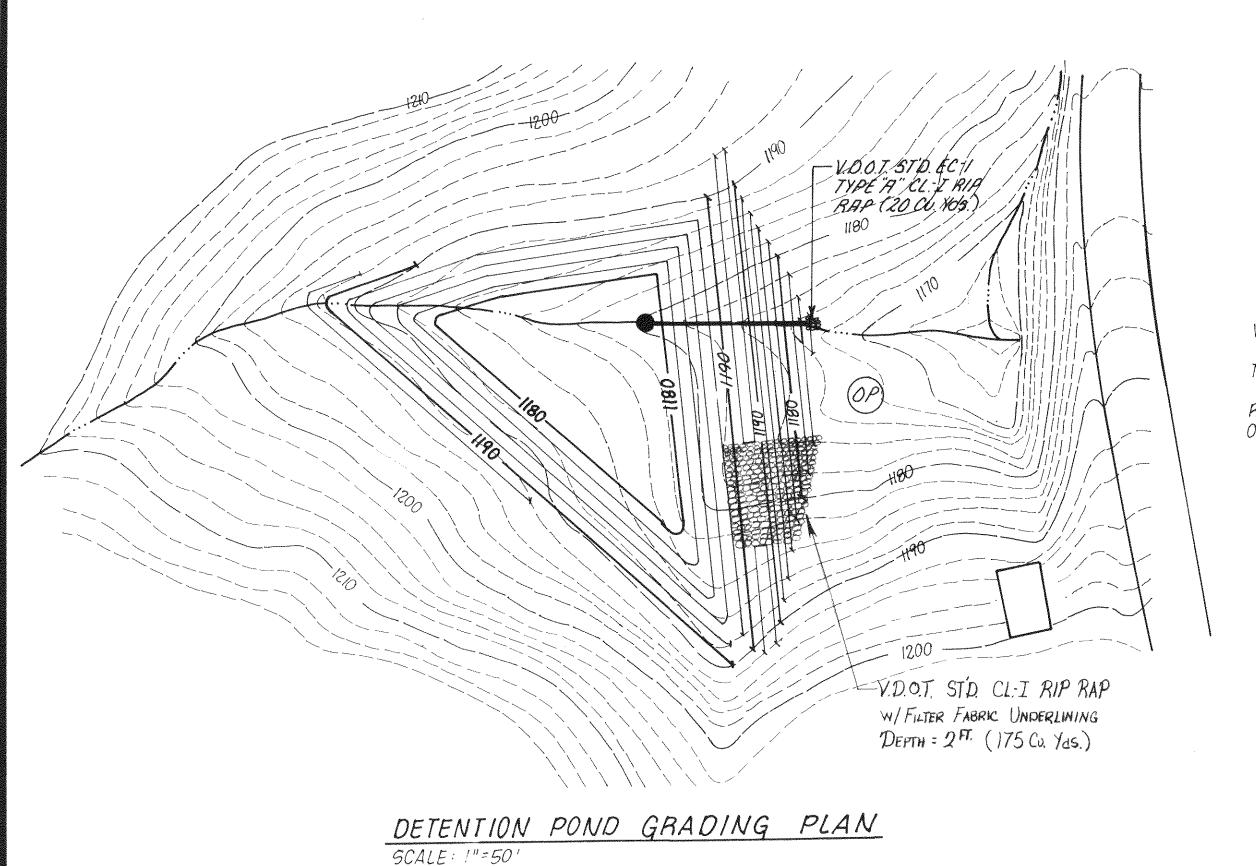
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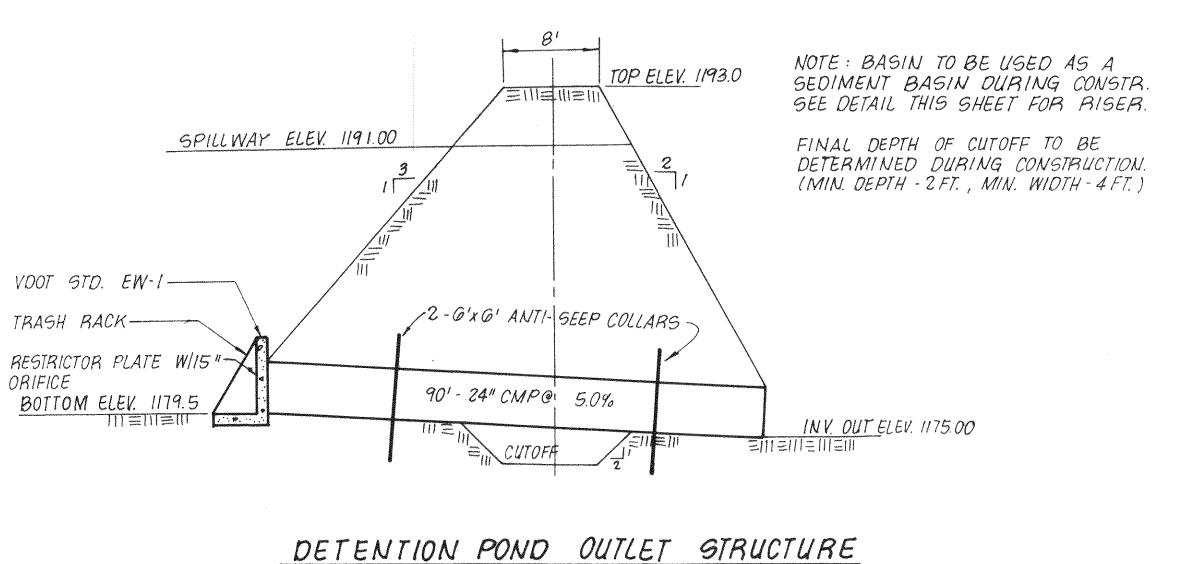
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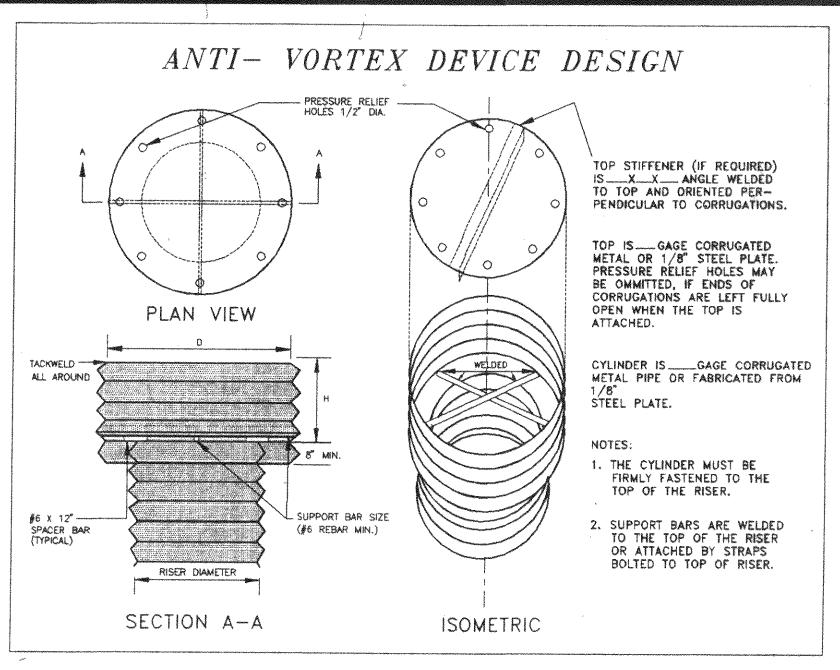
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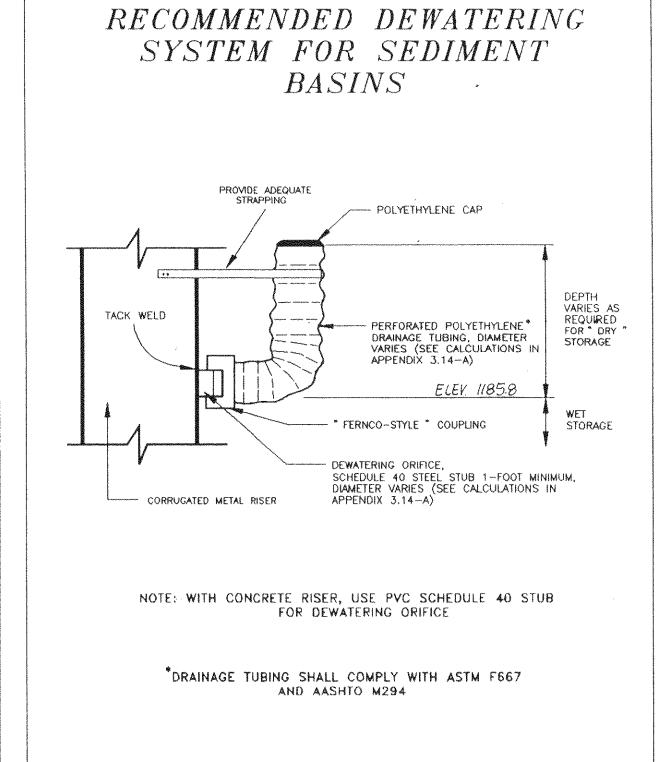


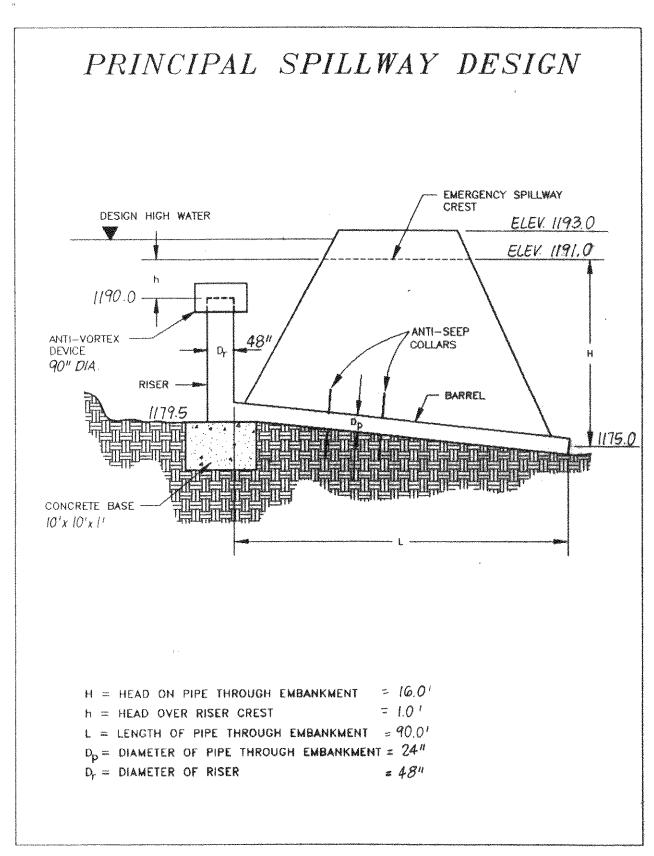


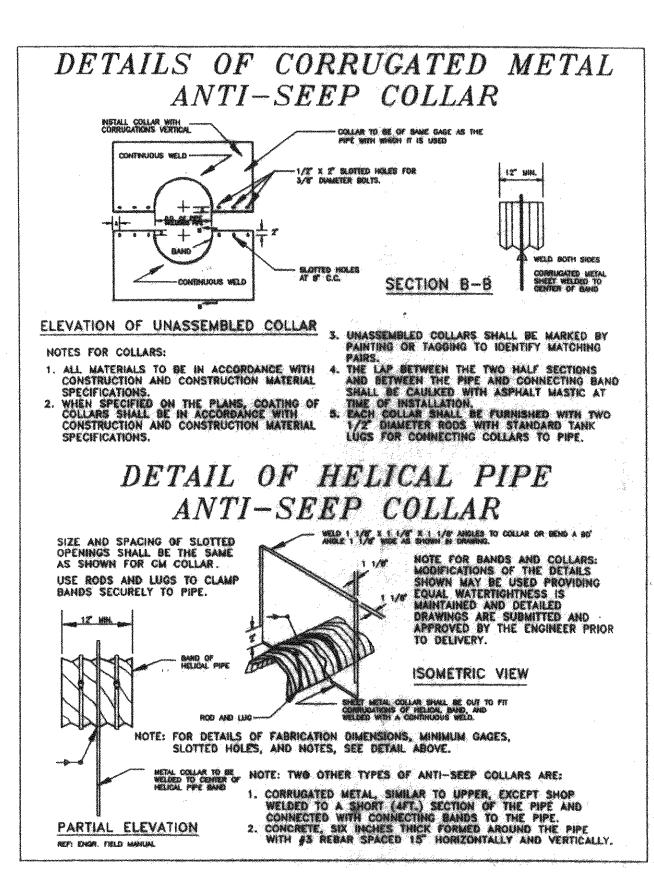




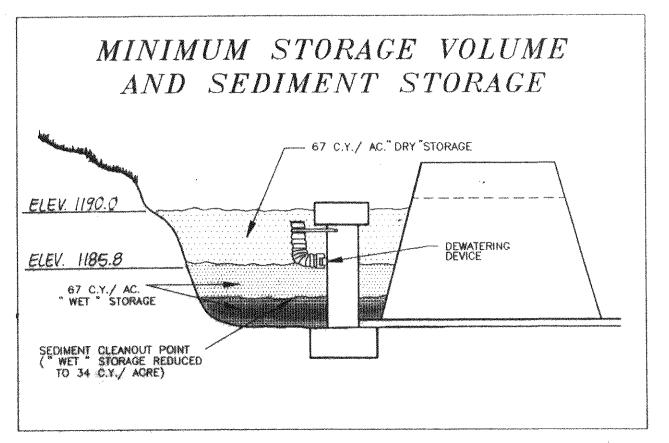








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LIMW, P.C.
Engineering - Surveying

Phone (703)345-0675 Fax (703)342-4456

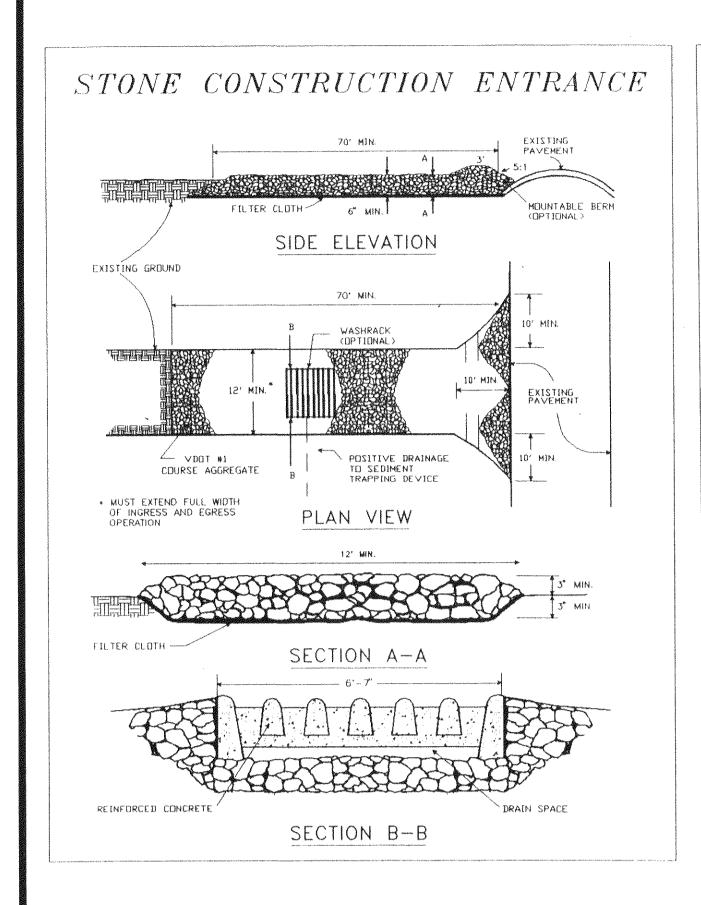
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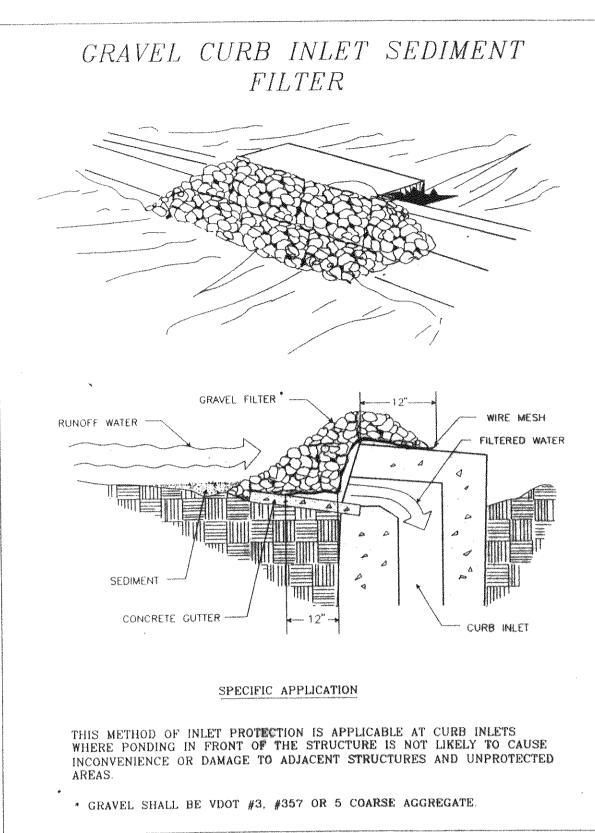
Laures Mountain EROSION CONTROL PLAN AND DETAILS

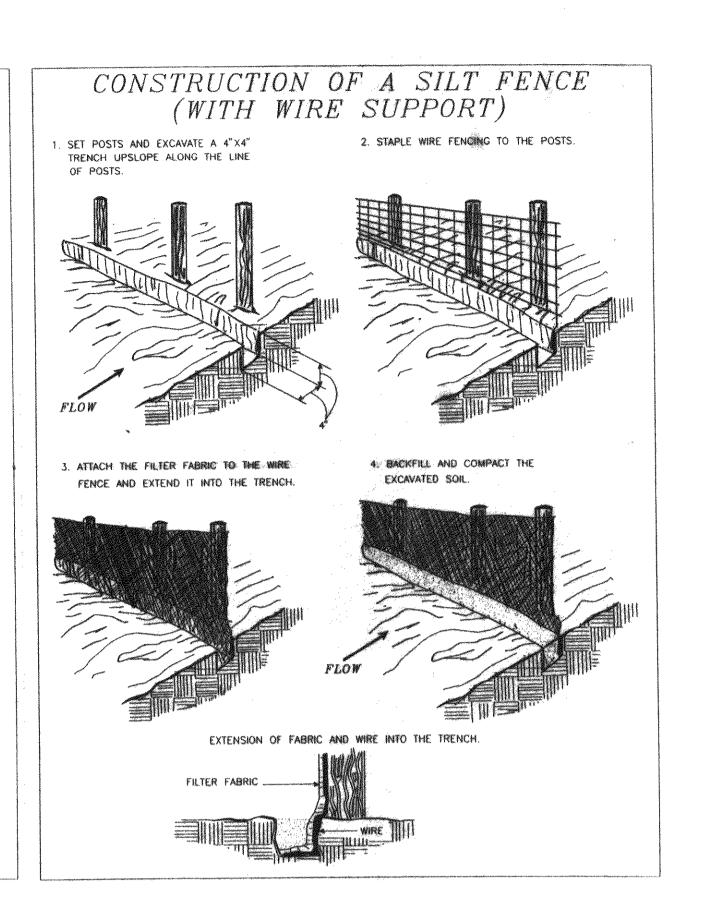
BOTETOURT COUNTY, VA.

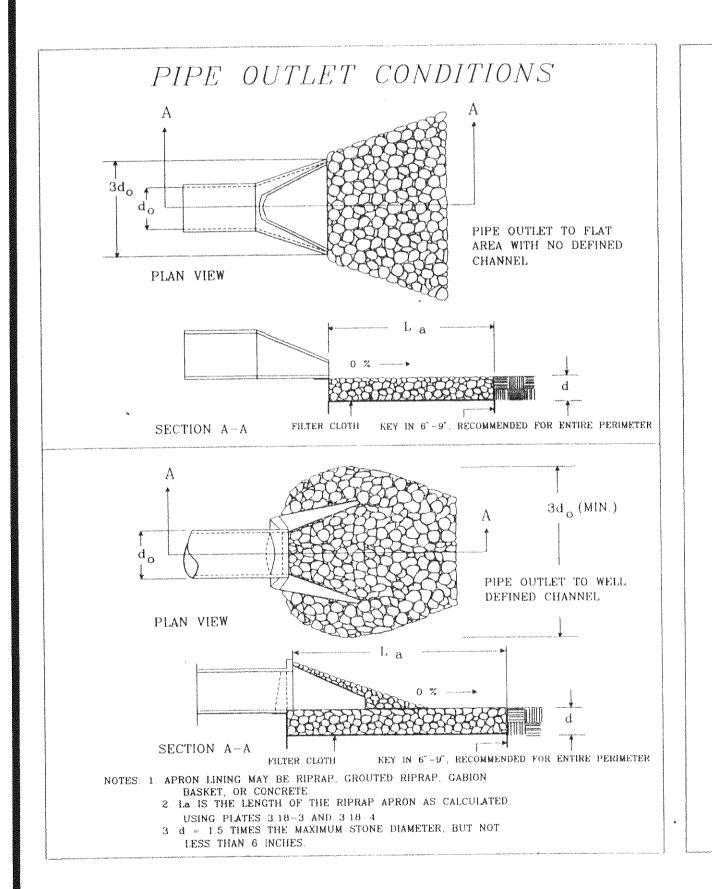
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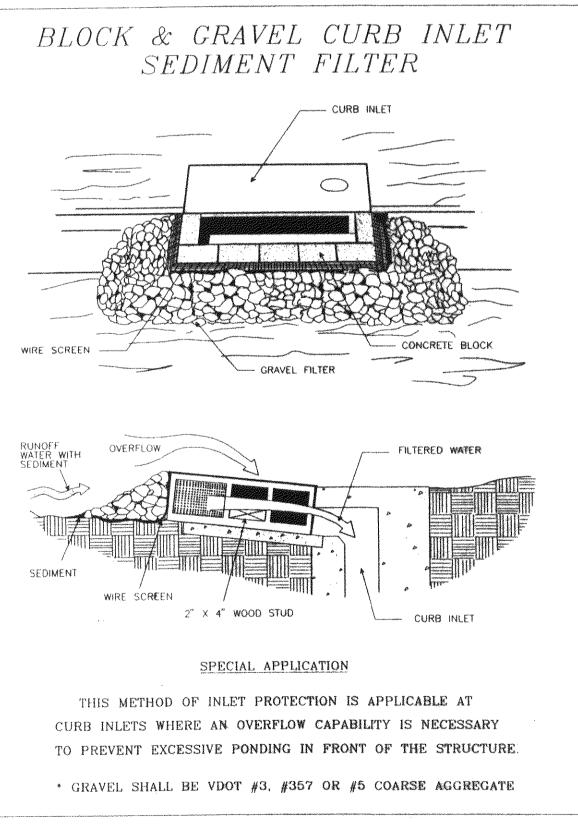
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LIMW, P.C.
Engineering - Surveying

Phone (703)345-0675 Fax (703)342-4456 1401 2nd Street, S.W. Roanoke, Virginia 24016

Laurel Mountain EROSION CONTROL DETAILS

BOTETOURT COUNTY, VA.

DOUGLAS R. MEREDITH, JR

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STATE SHEET CONTRACTOR STATE S

SCALE: DATE: PROJECT:

SPECIAL CONDITIONS

- 1. A minimum cover of three (3) feet over the proposed lines
- A. A preconstruction conference shall be scheduled with Batetourt Co.
- 2. No work shall begin without notifying Botetourt County 24 hours in advance. The contractor is responsible for obtaining any and all necessary permits.
- 3. No work shall begin without written approval of construction plans.
- 4. Work shall be subject to inspection by the County inspectors and design engineer. Sanitary Sewer cut sheets 5. Contractor shall be responsible for locating and uncovering all valve boxes after surface treatment of

roads and adjusting boxes to final road grades, if

- 6. All existing utilities may be shown or may not be shown in the exact location. The contractor shall comply with the State Water Works regulations, Section 12.05.03 where
- 7. The contractor shall notify the County and obtain County approval of any field correction to the approved plans prior
- 8. All trenches within the existing or future Virginia State Department of Highways and Transportation right-of-way must be compacted in six inch layers.
- 9. All lines to be staked prior to construction.

to such construction.

- 10. Contractor to coordinate with the Engineer to provide as-built plans. Contractor shall maintain a set of red—line plans showing as—built location of all structures. As-built information to be submitted to design engineer for preparation of record as-built plans. Such as-built plans shall be submitted to Botetourt County prior to County acceptance.
- 11. All construction shall be in accordance to approved construction practices of the applicable trades.
- 12. Unless noted otherwise herein all construction shall be in accordance to the latest edition of AWWA standards. EXCAVATION, STABILIZATION AND BEDDING

A. TRENCHING

- 1. Excavation for trenches shall include the removal of all material encountered regardless of classification in accordance with the elevations and arades at the locations and stations indicated on the plans or specified herein.
- 2. Excavation, unless otherwise specified, shall be open cut. The Contractor shall open no more than two hundred (200) feet of trench at one time during the laying of pipe, unless approved by the Engineer.
- 3. Trenches shall be excavated in straight lines and shall be accurately araded in order to establish a true elevation for the invert of the pipe.
- 4. The width of trenches, from existing grade to one (1) foot above the top of the pipe shall be of sufficient width to permit the proper installation of bracing. shoring or sheeting.
- 5. The sides of the trenches shall be as vertical as practical.
- 6. Excavation for structures shall allow a minimum of twelve (12) inches clear between the structure and the sides of the trench or any required bracing, shoring or sheeting.
- 7. Excavated materials suitable for backfill shall be stockpiled in an orderly manner at a sufficient distance from the sides of the trench in order to avoid overloading the banks of the trench and to prevent slides
- 8. Excavated materials which are not required or approved for backfill shall be removed from the site and disposed of by the Contractor, at his expense.
- 9. Contractor to adhere to all local, state and federal construction laws, including OSHA Trench Safety Regulations.

B. TRENCH STABILIZATION

- 1. Trench stabilization material shall be coarse aggregate size number 2 and shall conform with VDOT Section 203 and/or ASTM C 33.
- Whenever excessively wet or unstable material is encountered in the bottom of the trench, which in the opinion of the Engineer is incapable of properly supporting the pipe or structures, such material shall be removed and backfilled with trench stabilization material and shall be graded to allow for the compacted bedding material. 3. All unauthorized overdepths of excavation shall be backfilled, at the Contractor's expense, with trench stabilization material and shall be graded to allow for

C. COMPACTED BEDDING MATERIAL

the compacted bedding material.

- Bedding material shall be coarse aggregate size Number 57. and shall conform with VOOT Section 203 and/or ASTM C 33.
- 2. The bottom of the pipe trench shall be excavated to a minimum overdepth of six (6) inches below the bottom of the pipe, to provide for the compacted bedding material. Bedding material shall be placed, shaped and compacted.
- 3. Bell holes and depressions required for the jointing of the pipe shall be dua after the compacted bedding material has been graded and shaped and shall be only of the length, depth and width required to make the joint properly.

PIPE, JOINTS AND FITTINGS

A. SCOPE OF WORK

- All materials and appurtenances required for the work shall be new, or first class quality and shall be furnished, delivered, erected, connected and finished in every detail as specified or indicated. All materials found defective, reagraless of the circumstances, snall be replaced with new material at the expense of the
- The materials specified for the construction shall comply with the latest revisions of the applicable American Society for Testing Materials (ASTM), American National Standards Institute (ANSI) and/or the Virginia Department of Transportation (VDOT) standards.

B. OPTIONAL PIPE SELECTIONS

- 1. The Contractor shall install only one (1) type of pipe between structures except where ductile iron pipe is specified or indicated. Where existing pipe is to be replaced or extended the same type of pipe shall be installed, unless specified or indicated otherwise.
- 2. Water line shall be either PVC or ductile iron
- 3. Sanitary sewers with an inside diameter less than or equal to twelve (12) inches shall be either polyvinyl chloride or ductile iron pipe, at the Contractor's option, unless specified or indicated otherwise. Contractor shall obtain approval of pipe material by Batetourt County Engineer prior to beginning
- 4. Service laterals shall be Schedule 40 polyvinyl chloride.

C. TYPES OF PIPE

- 1. Polyvinyl chloride (PVC) water pipe shall be AWWA C900 DR 18 minimum, unless specified or indicated otherwise.
- 2. Ducine iron pipe shall conform with AWWA C 151/ANSI 21.51 and fittings shall conform with AWWA C 110/ANSI 21.10. The pipe and fittings shall be bituminous coated and cement lined in accordance with AWWA C 104/ANSI 21.40. The pipe thickness shall conform with AWWA C 150/ANSI 21.50 and shall be Class 50, as a minimum. unless specified or indicated otherwise.
- 3. PVC sewer pipe and fittings shall be SDR 35 (ASTM D

D. JOINTS COUPLINGS AND APPURTENANCES

- 1. PVC pipe and fittings shall be bell and spigot type joints. The bell and spigot joint shall be sealed with elastomeric aaskets conforming to ASTM D 3212. The joints shall be made in strict accordance with the recommendation of the pipe manufacturer.
- 2. Ductile iron pipe and fittings shall be either mechanical or bell and spigot type joints as specified or indicated. Joints shall be made with a single watertight rubber gasket manufactured in accordance with AWWA C 111/ANSI 21.11. The joints shall be made in strict accordance with the recommendations of the pipe manufacturer
- 3. Gate Valves shall be iron-body, bronze-mounted, doubledisc, parallel-seal, O-ring sealed, inside-screw, nonrising stem, fitting with 2 inch square operating nut for valve vault service, all in accordance with AWWA Standard C500 (latest revision). Connections shall be sultable for the pipe with which it is used. The valves shall be suitable for 200 p.s.i. water working pressure and shall be tested at twice the rated working pressure. All gate valves shall be installed in valve vaults and equipped with a 2-inch square operating nut. The nut shall be marked with an arrow and the word "OPEN" and shall open by turning to the right (clockwise).
- 4. All other materials and appurtences to be in accordance with details shown on plans.

PIPE INSTALLATION

A GENERAL

- 1. The Contractor shall not lay pipe or place manholes until all water has been removed from the trench, or when in the opinion of the Engineer, the trench or the weather conditions are unsuitable for work.
- 2. Pipe that may require field cutting shall be done so in a neat and workmanlike manner, so as to leave a smooth end at right angles to the axis of the nine. Care shall be taken to avoid damaging the pipe and any coatings or linings. Ductile iron pipe shall not be cut with an oxyacetylene torch.
- 3. The materials shall be visually inspected for defects before lowering the pipe or placing the manholes into the trench. During the laving operation no tools, clothing or other material shall be placed in the pipe or manhole. The interior of the pipe shall be clear of all soil, debris and superfluous materials prior to and during the
- 4. The Contractor shall exercise every precaution to prevent foreign material from entering the pipe while it is being placed in the trench. Failure by the contractor to take such precautions may result in the Engineer requiring a heavy, tightly woven canvas bag of suitable size be placed over each end of the pipe and removed only when the joint can be made properly.
- 5. The pipe and manholes shall be lowered carefully into the trench by suitable means and handled with care at all times to avoid damage. Under no circumstances shall the materials be dropped or dumped into the trenches.
- 6. When work is not in progress, the Contractor shall plug the open ends of the pipe to prevent trench water or other substances from entering the pipe. The plug shall be watertight and shall remain in place until any required dewatering has been completed.
- Parallel Installation Water lines shall be laid at least ten feet horizontally from a sewer or sewer manhole whenever possible. When local conditions prevent a horizontal separation of ten feet, the water line may be laid closer to a sewer or sewer manhole provided that:
 - 1. The invert of the water main shall be at least 18 inches above the crown of the sewer.
 - 11. Where this vertical separation cannot be obtained, the sewer shall be constructed of AWWA approved water pipe, pressure tested in place without leakage prior to backfilling.
 - III. The sewer mannole shall be of water-tight construction and tested in place.

Crossing - Water lines crossing sewers shall be laid to provide a separation of at least 18 inches between the bottom of the water line and the top of the sewer whenever possible. When local conditions prevent this vertical separation, the following construction shall be

> 1. Sewers passing over or under water lines shall be constructed of AWWA approved water pipe, pressure tested in place without leakage prior to backfilling.

II. Water lines passing under sewers shall, in addition, be protected by providing:

- (a) A vertical reparation of at least 18 inches between the bottom of the sewer and the top of the water line.
- (b) Adequate structural support for the sewers to prevent excessive deflection of the joints and the settling on and breaking of the waterline,
- (c) That the length of the water line be centered at the point of the crossing so that joints shall be equal distance and as far as possible from the sewer
- 8. Before joints are made the pipe shall be well bedded on a firm foundation and no pipe shall be brought into position until the preceding length has been thoroughly embedded and secured in place. Any defects due to settlement shall be made good by the Contractor at his expense. Bell holes shall be dua sufficiently large to insure the making of proper joints.
- 9. Pipe shall be jointed in full accordance with manufacturer's recommendations. Push-on joints shall be thoroughly cleaned, the rubber gasket inserted in the bell socket, a thin film of approved gasket lubricant applied, the spigot end of the pipe centered into the socket and the join completed by forcing the spigot end to the bottom of the socket by a jack-type tool or other device approved by the Engineer. Mechanical joints shall be thoroughly cleaned, the gland slipped over the spigot end of the pipe, the rubber gasket painted with soap solution and placed on the spigot end, the spigot end of the pipe seated in the bell, the gasket pressed into place within the bell, the gland moved into position, and bolts and nuts assembled by hand and tightened with an approved torque-limiting wrench

B. INSTALLING WATER MAINS

- 1. The water main shall be laid and maintained at the required lines and grades with fittings and valves at the required locations.
- 2. Deflection of the line of pipe, in either, the vertical or horizontal plane to avoid obstructions, or in locations where long-radius curves are required, the amount of deflection shall not exceed approved AWWA standards. Alignment that may require deflections in excess of the recommended limitations, special bends, or a sufficient number of shorter lengths of pipe to provide the angular deflections within the limits as set forth. shall be approved by the Engineer.
- All plugs, except mechanical joint plugs at connections for future lines, all tees, and all bends in water mains under pressure shall be provided with reaction backing consisting of concrete thrust blocks. Valves for connections to future lines and fire hydronts shall be anchored to the water main with the rods Detection tape to be installed 12"-18" above all new
- C. DISINFECTION OF WATER MAINS PVC water lines. 1. All pipe shall be disinfected, tested and flushed in
- accordance with AWWA Standard C601 (latest revision).

 All Valves & Appurtenances shall be Operated during the disinfected procedure. 2. Contractor shall provide all materials, equipment, necessary tops and perform all work required for the

sterilization, testing and flushing of the water main.

- 3. No tested section of water line shall be approved to deliver water service until a favorable laboratory report has been achieved. Any tested section of water line failing to meet the requirements specified shall be repaired by the Contractor and retested until the results are within the limits specified.
- 4. The water main or valved off section that has been completed shall be filled, tested and flushed. Test locations shall be subject to the discretion of the
- After testing and before final inspection of the completed systems, water mains and service laterals shall be flushed and disinfected in accordance with AWWA Specifications C601 (latest revision). Flushing shall be accomplished at a flow velocity of not less than 2.5 feet per second.

Disinfection as described in AWWA C651 - "Placing of calcium hypochlorite tablets" shall be used. 5 aram calcium hypochlorite tablets with 3.25 gram available chlorine per tablet shall be attached at the inside top of the pipe by an adhesive such as Permatex No. 1 or equal. The following number of tablets for the given pipe size shall be used for an initial dose of 25 mg/1 (ppm) chlorine:

Number Tablets Per

18-20 Ft. Pipe Section

Pipe Diameter

or the number of tablets equal to 0.0012d2L rounded to the next higher integer, where d is the inside diameter. in inches and L is the length of the pipe section, in feet. Use of the continuous feed or slug method of disinfecting may only be used to re-chlorinate a water pipe after the initial disinfection or in other specific cases approved by the Design Engineer. When filling the pipeline for disinfection, the rate of filling must

The disinfection solution shall remain in the pipe line for not less than twenty-four (24) hours, after which time a chlorine residual of 10 ppm at all parts of the line shall be required.

result in a velocity of less than 1 ft./sec

Following chlorination, the piping shall be thoroughly flushed. The Virginia Waterworks Regulations require at least two consecutive satisfactory bacteriological samples at 24 hour intervals from the distribution system at maximum spacing of 2000 feet before the system an be placed in service. If the initial testing is not satisfactory the new lines will be retested until satisfactory results are achieved. The Contractor shall pay all costs associated with disinfection and testing of installed facilities including any bacteriological samples and retesting if required. Samples will be collected in accordance with the Virginia Waterworks Regulations.

D. INSTALLING SEWER PIPE & MANHOLES

The installation of the sanitary sewer system shall begin at the downstream manhole and proceed upstream. The downstream sections shall be completed, tested and approved prior to allowing sanitary sewage to enter the

- The pipe shall be installed in accordance with the pipe manufacturer's recommendations and as directed by the Engineer. The pipe shall be laid in true straight lines with the bell ends upstream and with the invert of the pipe being the true elevation and grade of the system.
- 3. The Contractor shall be responsible for establishing and maintaining the horizontal alignment and vertical elevation and grade of the system in accordance with the survey information indicated on the plans.
- 4. The horizontal alignment of the pipe shall be maintained by a transit or theodolite plumbed over the center of the downstream manhole. The vertical elevation and arade shall be maintained by not less than three (3) batter boards placed between manholes or by an adjustable laser level mounted at the invert of the downstream manhole with target(s) placed in the bell and of the pipe being
- Sewer pipe shall be installed in 4 inch gravel bedding extending to the springline of pipe and in accordance with manufacturers recommendations.
- 6. The sanitary sewer system shall be laid and joined complete-in-place so that each length and section of pipe between the manholes shall have a smooth and uniform
- The pipe shall be connected to manholes through precast openings and joined with either a flexible boot adapter or a pipe seal gasket.
- 8. Detection tape to be installed 12" to 18" above all new sewer pipe mains and sewer service laterals.

E. CONNECTION TO EXISTING SYSTEMS

- The new pipe connection to be made to an existing manhole where no stub or opening exists, shall be made through an opening of maximum diameter cut into the manhole wall at the required location and elevation.
- 2. The existing invert channels and benches shall be reworked as required to form a new flow channel from the new connection to the existing flow channel.
- The new pipe connected to an existing manhole shall be secured in position and the remaining opening shall be filled and sealed with brick and mortar. The outer surface of the connection shall be given a coat of heavy bitumastic waterproofing compound.

E. SERVICE CONNECTIONS

- The Contractor shall make all service connections to the sewer pipe and from manhales where shown on the plans and/or where located in the field. The service connections to the sewer pipe shall be made with a wye or tee wye branch fitting.
- The wve and tee wve branch fittings for service connections shall be commercially manufactured and installed in strict accordance with the recommendations of the pipe manufacturer.
- 3. The sewer pipe shall not be cut or tapped for service connections except when and where permitted by the
- 4. All service connections shall be made with four (4) inch pipe as a minimum, unless the size of an existing service connection dictates otherwise, and shall be installed on a minimum grade of one-quarter (1/4) inch per one (1) foot from the sewer pipe or manhole to the property or easement line.
- 5. Future service connections shall extend to the property or easement line with cleanout and be properly capped with a watertight fitting to prevent infiltration into strict accordance with the recommendations of the pipe
- 6. Future service connections shall be field marked by a treated, solid wooden (2 x 4) marker three (3) feet long set vertically plumb with the end of the capped extension. The tops of the markers shall be painted yellow and set 24" above the finished grade. The location and invert depth of the service connection shall be shown on the as-built plans.

BACKFILLING

A. JOB CONDITIONS

- Prior to placing backfill, all organic, rubbish debris or other unsultable or objectionable material within the trench shall be removed. All concrete forms shall be removed. All shoring or sheeting shall be removed or cut off at the depth stipulated by the Engineer.
- 2. Prior to placing backfill, the trench box shall be removed. All concrete forms shall be removed. All shoring or sheeting shall be removed or cut off at the depth stipulated by the Engineer.
- Bockfill material shall be placed in uniform horizontal layers and thoroughly compacted with proper mechanical or hand operated tampers or other equipment as approved by the Engineer to perform such work.
- Backfill material shall be placed and compacted so as to not unevenly support, damage or displace the alignment of the pipe or structures.
- 5. Backfill shall not be placed or compacted against cast in place concrete until it has obtained sufficient strength to withstand the backfilled pressure placed upon it.
- Upon the completion of backfilling, all excess soil. stones and debris shall be removed from the site and disposed of by the Contractor.

B. BACKFILL MATERIAL

- Materials for backfill shall be approved excavated material or approved suitable material obtained from other sources. All material shall be approved by a Soils
- 2. Material shall consist of durable natural granular material or granular aggregates free of organic material, loam, debris, or other objectionable material which cannot be thoroughly compacted.
- Material shall not contain stones larger in diameter than those specified herein, granite, broken concrete, masonry rubble or other material which in the opinion of the Engineer is unsuitable for backfill.

4. Excessively wet excavated material shall not be used as backfill. Frazen material shall not be placed in the trench, nor shall approved backfill be placed upon frozen material. However, backfilling may be allowed in freezing weather with prior approval of the Engineer

C. BACKFILL BELOW UNPAVED AREAS

- 1. Backfill from the top of the pipe bedding or bottom of the pipe trench to one (1) foot above the top of the pipe shall be free of stones larger than two (2) inches in diameter and shall be placed in layers not to exceed six (6) inches and compacted with hand operated tampers.
- Backfill from one (1) foot above the top of the pipe to the topsoil subgrade shall be free of stones larger than six (6) inches in diameter and shall be placed in layers not to exceed twelve (12) inches and compacted with mechanical tampers.
- 3. Orginage channels to be constructed of fill material shall be graded and shaped to the topsoil subgrade with material free of stones larger than four (4) inches in diameter and shall be placed in layers not to exceed eight (8) inches and compacted with mechanical tampers.

D. BACKFILL BELOW EXISTING OR NEW PAVED AREAS AND SIDEWALKS

- Backfill from the top of the pipe bedding or battom of the pipe trench to one (1) foot above the top of the pipe shall be free of stones larger than two (2) inches in diameter and shall be placed in layers not to exceed six (6) inches and compacted with hand tampers.
- 2. Backfill from one (1) foot above the top of the pipe to the payement subgrade shall be free of stones larger than four (4) inches in diameter and shall be placed in layers not to exceed eight (8) Inches and compacted with mechanical tampers.

INSPECTION AND TESTS

A TESTING OF SANITARY SEWER

1. The Contractor shall prove the watertightness of the sewer system or portions thereof by one of the following tests, at such times as the Engineer may direct. Tests shall be made only in the presence of the Engineer. The Contractor shall furnish all labor and equipment required for the test and shall make repairs necessary until test results are satisfactory. Botetourt County Engineer shall be notified of all tests 48 hours prior to conducting such tests. All tests shall be coordinated with the Design Engineer for his attendance and abservation.

The testing equipment, procedure, and results will all be subject to the strict approval of the Engineer. Results of the air test will be reviewed for compliance with ASTM designation C-828, current revision. The air test is to be conducted between two (2) consecutive manholes. The test equipment shall consist of two (2) plugs (one tapped and equipped for air inlet connection), a shut-off valve, a pressure regulating valve, a pressure reduction valve, and a monitoring pressure gauge having a pressure range from 0 to 5 psi, graduated in 0.10 psi with an accuracy of plus/minus .04 psi. The test equipment shall be set up outside the manhole for easy access and reading. Air shall be supplied to the test slowly and shall be regulated to prevent the pressure inside the pipe from exceeding 5.0 psig. The pipeline shall be filled until a constant internal pressure of 3.5 psig is maintained. The internal pressure shall be maintained at 3.5 psig or slightly above for a five (5) minute stabilization period, after which time the internal pressure will be adjusted to 3.5 psig, the air supply shut off and the test begun. No person shall remain in the manhole while pipe is being pressurized or throughout the test for safety purposes. A pressure drop of 1.0 psi from 3.5 to 2.5 psig shall be allowed for the test times specified in the fol based upon the designated pipe size and test segment length.

AIR TEST TABLE

BASED ON EQUATIONS FROM ASTM C-828-80 SPECIFICATIONS TIME (MIN: SEC) REQUIRED FOR PRESSURE DROP FROM 3.5 TO 2.5 PSI WHEN

PIPE DIAMETER, INCHES

TESTING ONE PIPE DIAMETER ONLY.

EST EGMENT		4	6	8	10	12	15	18
	25	0:04	0:10	0:18	0: 28	0: 40	1:02	1:29
	50	0:09	0: 20	0: 35	0: 55	1:19	2:04	2: 58
	75	0:13	0: 30	0:53	1:23	1:59	3: 06	4: 27
	100	O: 181	0: 40	1:10	1:50	2: 28	4:08	5: 56
	125	0:22	0: 50	1:28	2:18	3:18	5:09	7: 26
	150	0: 26	0:59	1:46	2: 45	3:58	6:11	8:30
	175	0:31	1:09	2:03	3:13	4: 37	7:05	
	200	0: 35	1:19	2:21	3: 40	5:17		
	225	0:40	1:29	2: 38	4:08	5: 40		
	250	0:44	1: 39	2: 56	4: 35			8: 31
	275	0:48	1:49	3:14	4:43			9: 21
	300	Ö: 5J	1:59	3: 31				10:12
	350	1:02	2:19	3: 47			8:16	11:54
	400	1:10	2:38			6: 03	9: 27	13.36
	450	1.19	2:50			6: 481	0:38	15.19

5:14 7:341 1:49 17:01 Should the 1.0 psi drop occur in less time than that specified in the table, the sewer segment shall have failed. If the time required for the pressure to drop 1.0 psi is greater than

that shown in the table, the sewer segment shall have passed.

For a more detailed description of the air test method refer to ASTM designation C-828, current revision. An air pressure correction shall be required when the prevailing ground water is above the sewer line being tested and shall be calculated

Ground Water Depth (ft) + 3.5 = Starting Test Pressure

Ending Test Pressure = Starting Pressure - 1.0 psi There is no change from time requirements established for the

Manholes shall be tested by exfiltration by plugging lines with inflatable stoppers and filling the manhole with water for 12 hour sock period. Leakage shall not exceed one-half (1/2) gallan per hour in the two hour test period following the sock period. An approved air test for manholes will also be considered. Ex-filtration tests performed by approved vacuum tests procedures shall

C. TESTING OF WATER LINES

- 1. After placing all harnessing and all valve support concrete, sufficient backfill shall be placed prior to filling the pipe with water and field testing to prevent lifting of the pipe. When local conditions require that the trenches be backfilled immediately after the pipe has been laid, the testing shall be carried out after backfilling has been completed but prior to placement of the permanent surface. At least fourteen (14) days shall elabse after the last valve support or hydrant block has been cast (Type I Portland Cement) prior to testing, unless high early strength concrete (Type III) is used, In which case three (3) days shall elapse.
- 2. All testing will be performed in accordance with the AWWA C600-82 or current revision.
- 3. Pressure Test: After the pipe has been laid, all newly laid pipe or any valved section thereof shall be subjected to a hydrostatic pressure of at least 1.5 times the working pressure at the point of testing.
 - Test pressure restrictions. Test pressures shall:
 - a. not be less than 1.50 times the working pressure at the highest point along the test section;
 - b. not exceed pipe or thrust restraint design
 - c. be of at least 2—hour duration;
 - d. not vary by more than + 5 psi;
 - e. not exceed twice the rated pressure of the valves or hydrants when the pressure boundary of the test section includes closed gate valves or hydrants;
 - f. not exceed the rated pressure of the valve.

Each valved section of pipe shall be filled with properly disinfected water slowly and the specified test pressure shall be applied by means of a pump connected to the pipe in a manner satisfactory to the Engineer.

Before applying the specified test pressure, air shall be expelled completely from the pipe, valves, and hydrants.

All exposed pipe, fittings, valves, hydrants and joints shall be examined carefully during the test. Any damaged or defective pipe, fittings, valves, or hydrants that are discovered following the pressure test shall be repeated until it is satisfactory to the Engineer.

4. A leakage test shall be conducted concurrently with the pressure test. Leakage shall be defined as the quantity of water that must be supplied into the newly laid pipe, or any valved section thereof, to maintain pressure within 5 psi of the specified test pressure after the air in the pipeline has been expelled and the pipe has been filled with water. No pipe installation will be accepted if the leakage is greater than that determined by the following formula:

L = SDV(P)133,200

in which L is the allowable leakage, in gallons per hour; S is the length of pipeline tested in feet; D is the nominal diameter of the pipe, in inches: and P is the average test pressure during the leakage test, in pounds per square inch gauge. When testing against closed metal-seated valves, an additional leakage per enclosed valve of 0.0078 gal/hr/in, of nominal valve size shall be allowed. When hydrants are in the test section, the test shall be made on the basis of allowable leakage. If any test of pipe laid discloses leakage greater than the allowable amount, the Contractor shail, at his own expense, locate and repair the defective material until the leckage is within the specified allowance. All visible leaks are to be repaired regardless of the amount

D. MANDREL TEST

of leakage.

Should PVC or ABS gravity pipe be utilized, the Contractor shall deflection test the entire length of pipe by means of a go-no-go mandrel to assure that a 5.0% deflection has not been exceeded. The testing shall be performed completely at the expense of the Contractor and shall be performed in the presence of the Engineer. Hendrel and proving ring details shall be approved by the Engineer and shall be sized at 5% less than ASTM dimensions for the sever pipe. The mandrel test shall be performed no sooner than three (3) months afte backfill of the pipe is completed. All pipe that fails the deflection test shall be removed and replaced at the Contractor's expense. The "rerounder" technique shall not be allowed. The Contractor shall use approved nine (9) are mendrels and proving rings for each size of mainline pipe. The contract length "L" of the mandrel agms and the actual mandrel diameter "D" (ID of the proving ring) shall equal the dimensions in Table I below. Critical mandrel dimensions shall carry a tolerance of ±.01."

9 Arm Mandrel

ASTM 03034 ASTM 02751 (6" only)* ASIM DIGRO Hom. Dia. ___SDR_12_ 5.65 M 5.49" (SDR 23.5 8 M 7.56" 9.45" 11.26" 11.22* 13.78"

Mandrel and proving ring may be obtained from Wortco, Inc., 220 High Street, Franklin, Ohio 45065 (1-513-746-6439), or Hurco ENCUENTION

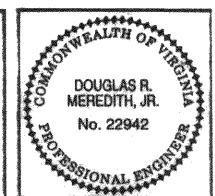
LMW, P.C.

Phone (703)345-0675 1401 2nd Street, S.W.

Fax (703)342-4456 Roanoke, Virginia 24016

Laures Mountain UTILITY SPECIFICATIONS

BOTETOURT COUNTY, VA.



DATE BY

REVISION BCW DRAWN: SCALE: NONE CLD DATE: CHECKED DRM APPROVED: PROJECT:

DESIGNED:

SHEET.