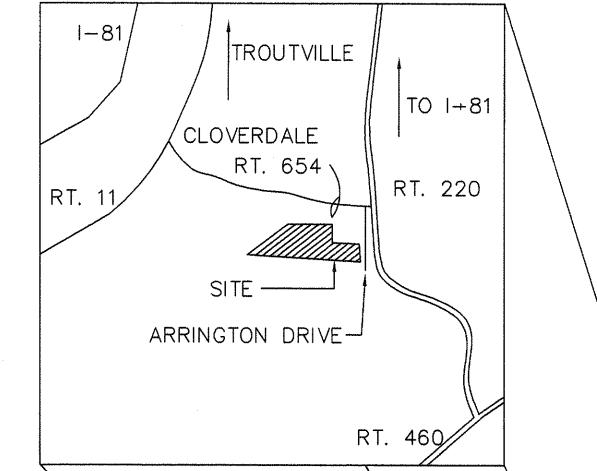
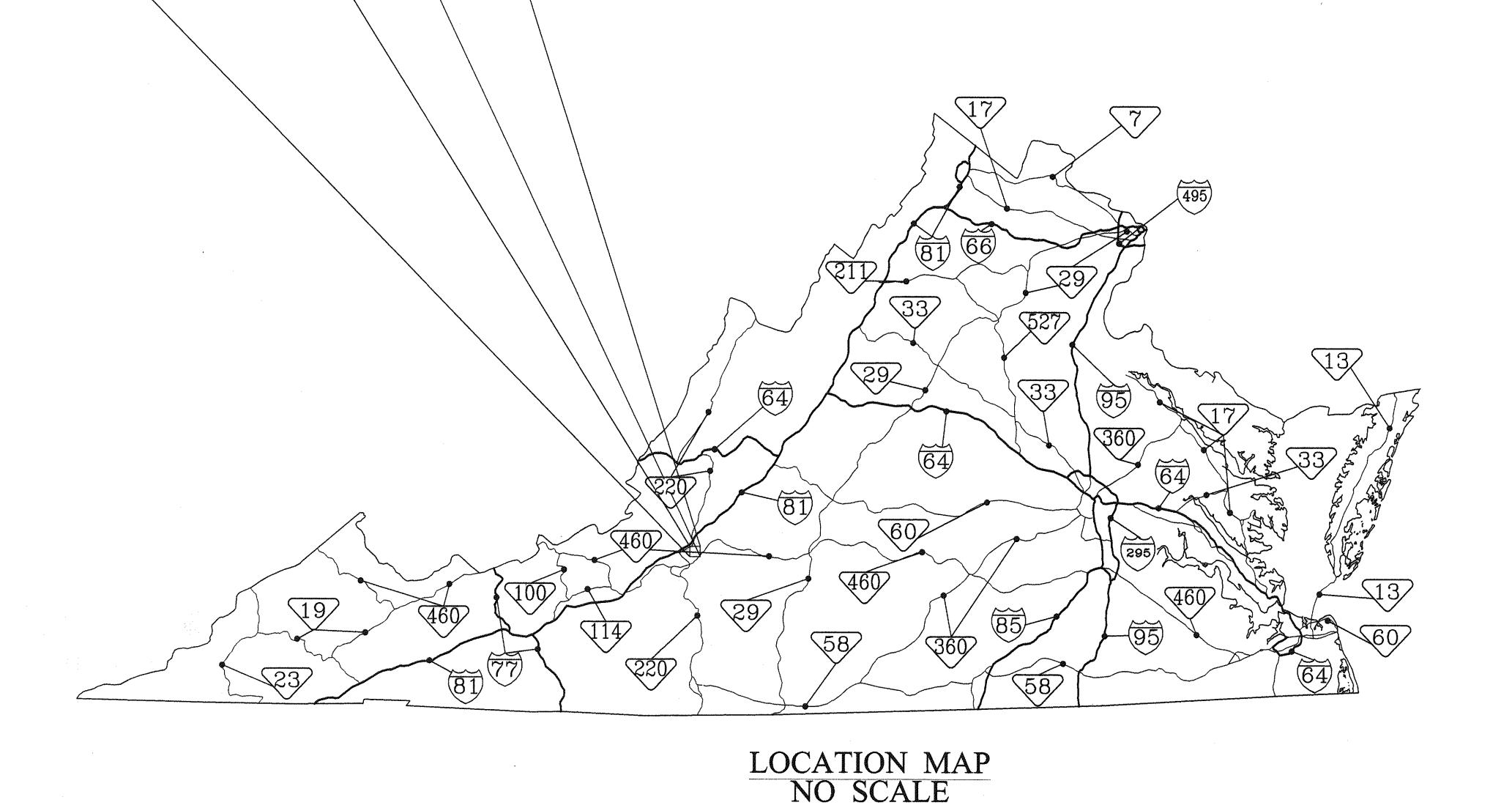
MOUNTAIN CREST

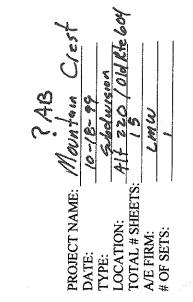
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BOTETOURT COUNTY, VIRGINIA

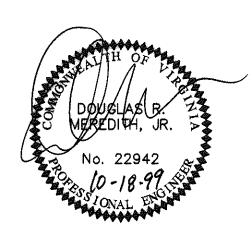
FRED HANCOCK
P.O. BOX 6
DALEVILLE, VIRGINIA 24083
PHONE #: 992-2384





INDEX OF DRAWINGS

- 1. COVER
- 2. ABBREVIATIONS, LEGEND, GENERAL NOTES & PLAN & PROFILE KEY PLAN
- 3. TRANSPORTATION NOTES & PAVEMENT SECTION
- 4. UTILITY SPECIFICATIONS
- 5. PLAN & PROFILE AUDREY LANE
- 6. PLAN & PROFILE TERESA LANE STA. 0+00 TO 8+00
- 7. PLAN & PROFILE TERESA LANE STA. 8+00 TO 15+00
- 8. PLAN & PROFILE TERESA LANE STA. 15+00 TO 23+51.4
- 9. PLAN & PROFILE HOLLY LANE
- 10. SEDIMENT POND GRADING PLAN & OUTLET STRUCTURE DETAILS
- 10A. STORM SEWER PLAN & PROFILE
- 11. WATER DETAILS
- 12. SEWER DETAILS
- 13. EROSION & SEDIMENT CONTROL NARRATIVE
- 14. EROSION & SEDIMENT CONTROL ADDITIONAL DETAILS



P.C.

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(540) 345-0675

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ROANOKE, VIRGINIA 24013

MOUNTAIN CREST SITE PLAN

COMM. NO. 1809

DATE: 05/26/99 REVISED: 10/18/99

SET NO.

MECH

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MH

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VESCR

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U.S.C.&G.S

TRTD

T & B

STRUCT

SD

SE

PVMT

MECHANICAL

MANHOLE

MINIMUM

METAL

NUMBER

MONUMENT

NAIL AND CAP

NOT TO SCALE

ON CENTERS

PAVEMENT

PERIMETER

PERFORATED

PERPENDICULAR

POINT ON LINE

POWER POLE

RADIUS, RISER

RAILROAD

REDUCER

REFERENCE

RELOCATED

RIGHT OF WAY

STORM DRAIN

SPECIFICATION

SPECIFICATIONS

STAINLESS STEEL

SLOPE EASEMENT

SANITARY SEWER

REQUIRED

REVISION

SANITARY

SIDEWALK

SECTION

SERVICE

SQUARE

STREET

STEEL

THICK

TREATED

TYPICAL

TELEVISION

TOP OF WALL

UNDERGROUND

VALVE, VENT

VARIABLE

VERTICAL

VOLUME

GEODETIC SURVEY

VERTICAL CURVE

WESTBOUND LANE

WATER SURFACE

OF HIGHWAYS

WATERTIGHT, WEIGHT

WATER LINE

WITHOUT

CONTROL REGULATIONS

VERTICAL SIGHT DISTANCE

WEST VIRGINIA DEPARTMENT

UNLESS OTHERWISE NOTED

UNITED STATES COAST AND

VIRGINIA EROSION AND SEDIMENT

WIDE FLANGE, WIDE, WASTE, WATER

HIGH DENSITY POLYETHYLENE

VIRGINIA DEPARTMENT OF TRANSPORTATION

STATION

STANDARD

SURVEY

STRUCTURAL

TELEPHONE

TEMPORARY

TOP AND BOTTOM

TELEPHONE POLE

SHEET

ROUTE

RIGHT

ROAD

MANUFACTURER

MECHANICAL JOINT

NOT IN CONTRACT

OUTSIDE DIAMETER

POINT OF CURVE

POINT OF COMPOUND CURVE

POINT OF INTERSECTION

PLATE, PROPERTY LINE

POINT OF REVERSE CURVE

PUBLIC UTILITY EASEMENT

REINFORCED CONCRETE PIPE

REINFORCE, REINFORCEMENT

POUNDS PER SQUARE INCH

POINT OF VERTICAL INTERSECTION

POINT OF TANGENCY

POINT ON TANGENT

POINT OF TANGENT

POLYVINYL CHLORIDE

NON POTABLE WATER

ABANDON, ABANDONED

ABUTMENT

ADJACENT

ANCHOR

AGGREGATE

APPROXIMATE

BITUMINOUS

BELL JOINT

BASE LINE

BUILDING

BENCH MARK

CAST IRON

CONDUIT

CLEANOUT

COMBINATION

CONTRACTOR

CRUSHED STONE

DRAINAGE EASEMENT

CONVEYOR

CORNER

CENTER

CULVERT

DIAMETER

DOWN

DETAIL

DRIVEWAY

DWELLING

DRAWING

ELEVATION

ELECTRICAL

ENGINEER

EQUAL

ENTRANCE

EQUIPMENT

EXISTING

FLOOR

FLEXIBLE

FLANGE

FOOTING

FUTURE

GALLON

GARAGE

GROUND

GRAVEL

GRATING

GALVANIZED

GOVERNMENT

GATE VALVE

HORIZONTAL

HIGH POINT

INSULATION

LIGHT POLE

MASONRY

MATERIAL

MAXIMUM

MAIL BOX

MINIMUM BUILDING LINE

LONG RADIUS

LENGTH, LONG LINEAL FOOT

HYDRANT

INCH

INVERT

LONG

LEFT

HUB AND TAC

INSIDE DIAMETER

IRON PIN (FOUND OR SET NOTED)

GALLONS PER MINUTE

FOOT

FINISH FLOOR

END OF LINE

EACH

DIMENSION

DISCONNECT

DROP MANHOLE

EASTBOUND LANE

EDGE OF PAVEMENT

EACH WAY, ENDWALL

FLARED END SECTION

FINISHED FLOOR ELEVATION

END VERTICAL CURVE ELEVATION

END VERTICAL CURVE STATION

CENTER LINE

CONSTRUCTION

BEGIN. BEGINNING

BLACK STEEL PIPE

BUTTERFLY VALVE

CURB AND GUTTER

CORRUGATED METAL PIPE

CONNECT, CONNECTION

CONCRETE MASONRY UNITS

CONCRETE (PORTLAND CEMENT)

DEPTH OR DEGREE OF CURVE

DROP INLET, DUCTILE IRON

BEGIN VERTICAL CURVE ELEVATION

BEGIN VERTICAL CURVE STATION

ABUT

AGGR

APPROX

ADJ

ANC

BIT

BEG

BLDG

BSP

ΒV

BVCE

BVCS

C & G

CONST

CMP

CMU

CND

CO

COMB

CONC

CONN

CONV

COR

CTR

CULV

DIA

DIM

DISC

DMH

DN

DTL

DWL

DWG

E.B.L.

ELEC

ENGR

ENTR

EOL

EQ

EQPT

EVCE

EVCS

EW

FES

FFE

FIG

FLEX

FLG

FTG

FUT

GAL

GALV

GAR

GND

GOVT

GRTG

GPM

GΥ

H&T

HPT

HYD

INSUL

INV

MAS

MATL

MAX

MB

MBL

HORIZ

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EXIST

EL. ELEV

DW, D/W

CONTR

CR STONE

ВМ

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DESCRIPTION

FOUNDATION ONLY

SPOT ELEVATION

CONCRETE CURB

PAVEMENT

TREE LINE

TREE OR SHRUB

PROPERTY LINE

P.C. OR P.T.

CENTERLINE OR BASELINE

GEOLOGIC BORE HOLE

SANITARY SEWER

FORCE MAIN

PIPE FITTINGS

FIRE HYDRANT

GATE VALVE

CLEANOUT

MANHOLE

LIGHT POLE

PAVED DITCH

WM - WATER METER

TELEPHONE PEDESTAL

STEEL ENCASEMENT

CONCRETE ENCASEMENT

ABANDON OR REMOVE

LIMITS OF CONSTRUCTION

BURIED TELEPHONE VAULT

STORM DRAIN AND ENDWALL

GAS MAIN OR SERVICE LINE

OVERHEAD ELECTRICAL LINE

OVERHEAD TELEPHONE LINE

FIELD SURVEY TRAVERSE POINT

CONCRETE CURB & GUTTER

UNPAVED OR GRAVEL ROAD

FENCE (EXISTING OR PROPOSED NOTED)

BENCH MARK (EXISTING OR SET NOTED)

CENTERLINE CREEK, SWALE, DITCH

CONSTRUCTION EASEMENT

PERMANENT EASEMENT

CONCRETE WALK OR SLAB

BUILDING WITH PORCH OR STOOP

CONTOUR, CONTOUR WITH ELEVATION

EXISTING

«Метреней» выбольный найоней не «Метреней» на применей на предела на предела на предела на предела на предела Дей на предела на пре

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PROFILE

PROFILE

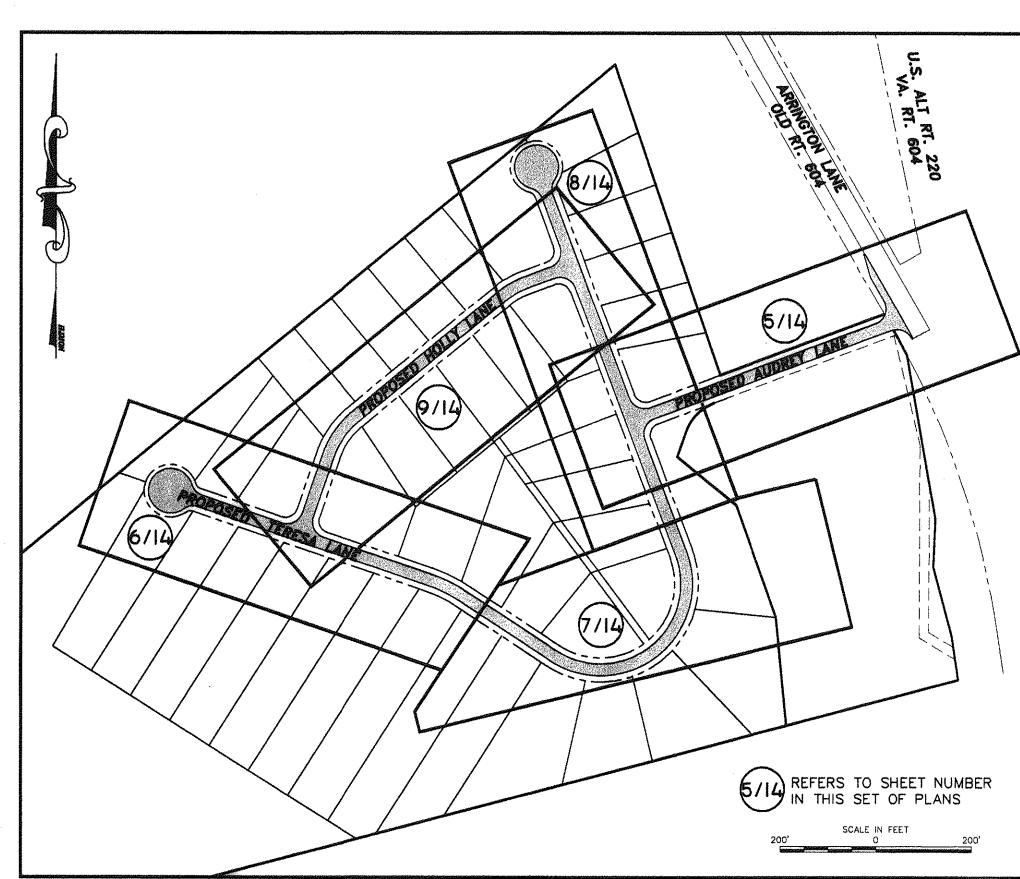
PLAN

EXISTING WATER LINE LOCATIONS BOTH HORIZONTAL AND VERTICAL ARE APPROXIMATE. THE LOCATION IS NOT THE RESULT OF A FIELD SURVEY.

THE CONTRACTOR IS DIRECTED TO DIG AND LOCATE ALL UTILITIES IN ADVANCE OF PIPELAYING TO ALLOW FOR ADJUSTMENTS DUE TO CONFLICTS WITH EXISTING UTILITIES. SHOULD A CONFLICT ARISE THE ENGINEER IS TO BE NOTIFIED IMMEDIATELY.

THE CONTRACTOR IS REQUIRED TO NOTIFY "MISS UTILITY" AT 1-800-552-7001 AT LEAST TWO, BUT NOT MORE THAN TEN, WORKING DAYS IN ADVANCE OF CONSTRUCTION.

WATER MAIN OR SERVICE LINE UNDERGROUND ELECTRICAL LINE UNDERGROUND TELEPHONE LINE DROP INLET (CURB AND GRATING TYPES) DWM - DOUBLE WATER METER TELEPHONE POLE, GUY AND ANCHOR POWER POLE, GUY AND ANCHOR STORM PIPE (SIZE / TYPE NOTED) CULVERT WITH FLARED END SECTION AIR RELEASE VALVE / VAULT ASSEMBLY BLOW OFF VALVE / VAULT ASSEMBLY



PLAN & PROFILE KEY PLAN SCALE: 1"= 200'

RCW Designed By DJB Drawn By RCW Checked By Approved By Submitted By 1809 ABBF Drawing 05/26/99 AS NOTE Commission No.

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1. QUALITY CONTROL

STREETS TO BE GRADED, PAVED AND ALL STRUCTURAL COMPONENTS ERECTED IN ACCORDANCE WITH CURRENT VIRGINIA DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE SPECIFICATIONS AND ROAD DESIGN STANDARDS, VIRGINIA DESIGN STANDARDS AND SPECIFICATIONS. ALL MATERIALS USED SHALL BE TESTED IN ACCORDANCE WITH STANDARD POLICIES. THE DEVELOPER MUST CONTACT THE OFFICE OF THE RESIDENT ENGINEER PRIOR TO BEGINNING ANY CONSTRUCTION AT WHICH TIME AN INSPECTION AND TESTING PROCEDURE POLICY WILL BE DRAWN. THE DEVELOPER WILL PRODUCE TEXT REPORTS FROM APPROVED INDEPENDENT LABORATORIES AT THE DEVELOPER'S EXPENSE.

THE PAVEMENT DESIGNS SHOWN ARE BASED ON A SUBGRADE CBR VALUE OF 10 OR GREATER. THE SUBGRADE SOIL IS TO BE TESTED BY AN INDEPENDENT LABORATORY AND THE RESULTS SUBMITTED TO THE VIRGINIA DEPARTMENT OF TRANSPORTATION PRIOR TO BASE CONSTRUCTION. SHOULD THE SUBGRADE CBR VALUES BE LESS THAN 10, THEN ADDITIONAL BASE MATERIAL WILL BE REQUIRED IN ACCORDANCE WITH DEPARTMENTAL SPECIFICATIONS.

THE SUBGRADE MUST BE APPROVED BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION PRIOR TO PLACEMENT OF THE BASE. BASE MUST BE APPROVED BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION FOR DEPTH, TEMPLATE AND COMPACTION BEFORE SURFACE IS APPLIED.

2. UTILITIES

ALL NECESSARY UTILITY LATERALS ALONG WITH PROVISIONS FOR CONDUITS (I.E. WATER, SEWER, STORM, GAS, AND TELEPHONE) WILL BE CONSTRUCTED PRIOR TO PLACEMENT OF BASE MATERIAL.

GAS OR PETROLEUM TRANSMISSION LINES WILL NOT BE PERMITTED WITHIN THE PAVEMENT OR SHOULDER ELEMENT (BACK OF CURB TO BACK OF CURB) OF THIS DEVELOPMENT. SERVICE LATERALS CROSSING AND PIPE LINES LOCATED OUTSIDE THE PAVEMENT, BUT INSIDE THE RIGHT OF WAY, WILL BE CONSTRUCTED IN CONFORMITY WITH ASA B 31.8 SPECIFICATIONS AND SAFETY REGULATIONS. DISTRIBUTION LINES WITH PRESSURES LESS THAN 120 LBS. ARE UNAFFECTED BY THE ABOVE.

PERMITS WILL BE REQUIRED FOR ALL UTILITIES WITHIN STREET RIGHT OF WAY PRIOR TO ACCEPTANCE INTO THE SECONDARY HIGHWAY SYSTEM.

ANY EASEMENTS GRANTED TO A UTILITY COMPANY FOR PLACEMENT OF POWER, TELEPHONE, ETC. MUST BE RELEASED PRIOR TO ACCEPTANCE.

3. PRIVATE ENTRANCES

DRIVEWAYS CONNECTING TO ROADS WITHOUT CURB & GUTTER SHALL CONFORM TO THE PAVEMENT, SHOULDER & SLOPE.

PERMITS WILL BE REQUIRED FOR ALL PRIVATE ENTRANCES CONSTRUCTED ON STREET RIGHTS-OF-WAY AFTER ACCEPTANCE INTO THE SECONDARY HIGHWAY SYSTEM.

4. EROSION CONTROL AND LANDSCAPING

CARE MUST BE TAKEN DURING CONSTRUCTION TO PREVENT EROSION, DUST AND MUD FROM DAMAGING ADJACENT PROPERTY, CLOGGING DITCHES, STREAKING PUBLIC STREETS AND OTHERWISE CREATING A PUBLIC OR PRIVATE NUISANCE TO SURROUNDING AREAS.

THE ENTIRE CONSTRUCTION AREA INCLUDING DITCHES, CHANNELS, BACK OF CURBS AND OR PAVEMENT ARE TO BE BACKFILLED AND SEEDED WITHIN 7 DAYS AFTER COMPLETING FINAL GRADING.

DRAINAGE EASEMENTS MUST BE DEFINED BY EXCAVATED DITCHES OR CHANNELS FOR THEIR FULL LENGTH TO WELL DEFINED EXISTING NATURAL WATERCOLLESS

THE ROAD WILL BE REVIEWED DURING CONSTRUCTION FOR THE NEED OF PAVED DITCHES. IF EROSION IS ENCOUNTERED IN ANY DRAINAGE EASEMENT, IT WILL BE THE RESPONSIBILITY OF THE DEVELOPER TO SOD, RIP RAP, GROUT, PAVE, OR TO DO WHATEVER IS NECESSARY TO CORRECT THE PROBLEM.

ALL VEGETATION AND OVERBURDEN TO BE REMOVED FROM SHOULDER TO SHOULDER PRIOR TO THE CONDITIONING (CUTTING AND/OR PREPARATION) OF THE SUBGRADE.

5. INTERSECTION PAVEMENT RADIUS

MINIMUM PAVEMENT RADIUS OF 35 FEET IS REQUIRED AT ALL STREET INTERSECTIONS.

6. CONNECTIONS TO STATE-MAINTAINED ROADS

WHILE THESE PLANS HAVE BEEN APPROVED, SUCH APPROVAL DOES NOT EXEMPT CONNECTIONS WITH EXISTING STATE-MAINTAINED ROADS FROM CRITICAL REVIEW AT THE TIME PERMIT APPLICATIONS ARE MADE. THIS IS NECESSARY IN ORDER THAT THE PREVAILING CONDITIONS BE TAKEN INTO CONSIDERATION REGARDING SAFETY ACCOMPANIMENTS SUCH AS TURNING LANES.

7. GUARDRAILS

STANDARD GUARDRAIL WITH SAFETY END SECTIONS MAY BE REQUIRED ON FILLS AS DEEMED NECESSARY BY THE RESIDENT ENGINEER. AFTER COMPLETION OF ROUGH GRADING OPERATIONS, THE OFFICE OF THE RESIDENT ENGINEER SHALL BE NOTIFIED SO THAT A FIELD REVIEW MAY BE MADE OF THE PROPOSED LOCATIONS.

WHERE GUARDRAILS ARE TO BE INSTALLED, THE SHOULDER WIDTH SHALL BE INCREASED IN ACCORDANCE WITH VDOT ROAD AND BRIDGE STANDARDS.

8. ENTRANCE PERMIT

CONTRACTOR SHALL OBTAIN ENTRANCE PERMIT TO THE EXISTING VIRGINIA DEPARTMENT OF TRANSPORTATION RIGHT-OF-WAY FROM RESIDENT ENGINEER PRIOR TO ROAD CONSTRUCTION.

9. INSPECTION

AN INSPECTOR WILL NOT BE FURNISHED EXCEPT FOR PERIODIC PROGRESS INSPECTION, THE ABOVE MENTIONED FIELD REVIEWS AND CHECKING FOR REQUIRED STONE DEPTHS. THE DEVELOPER WILL BE REQUIRED TO POST A SURETY TO GUARANTEE THE ROAD FREE OF DEFECTS FOR ONE YEAR AFTER ACCEPTANCE BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION.

10. STREET MAINTENANCE

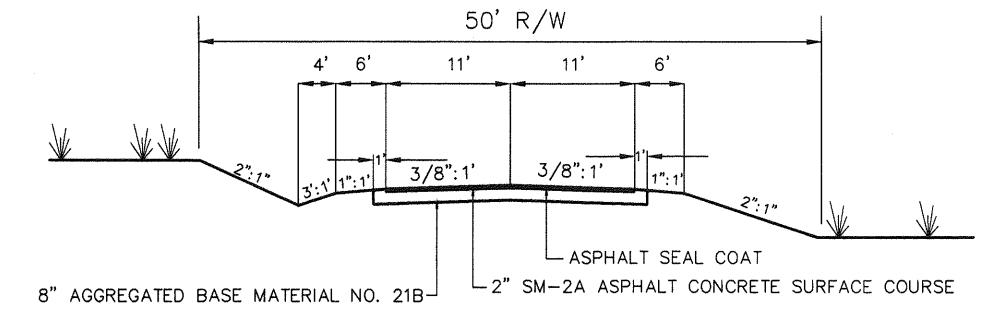
THE STREETS MUST BE PROPERLY MAINTAINED UNTIL ACCEPTANCE. AT SUCH TIME AS ALL REQUIREMENTS HAVE BEEN MET FOR ACCEPTANCE, ANOTHER INSPECTION WILL BE MADE TO DETERMINE THAT THE STREET HAS BEEN PROPERLY MAINTAINED.

11. UNDERGROUND UTILITIES

CONTRACTOR SHALL VERIFY LOCATION AND ELEVATION OF ALL UNDERGROUND UTILITIES SHOWN ON THE PLANS IN AREAS OF CONSTRUCTION PRIOR TO STARTING WORK BY CONTACTING MISS UTILITY. CONTACT CONSULTING ENGINEER IMMEDIATELY IF LOCATION OR ELEVATION IS DIFFERENT FROM THAT SHOWN ON THE PLANS. IF THERE APPEARS TO BE A CONFLICT AND UPON DISCOVERY OF ANY UTILITY NOT SHOWN ON THIS PLAN, CALL "MISS UTILITY" OF CENTRAL VIRGINIA AT 1-800-552-7001.

12. REVISIONS OF SPECIFICATIONS AND STANDARDS

APPROVAL OF THESE PLANS WILL BE BASED ON SPECIFICATIONS AND STANDARDS IN EFFECT AT THE TIME OF APPROVAL AND WILL BE SUBJECT, UNTIL COMPLETION OF THE ROADWAY AND ACCEPTANCE BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION, TO FUTURE REVISIONS OF THE SPECIFICATIONS AND STANDARDS.



AUDREY LANE, TERESA LANE & HOLLY LANE

TYPICAL PAVEMENT SECTION

NO SCALE

C ENGINEER

MOUNTAIN CREST SUBDIVISION CONTED IN

NO. DATE DESCRIPTION BY TRANSPORTATION NOTES PAVEMENT SECTIONS

NO WORK SHALL BEGIN WITHOUT WRITTEN APPROVAL OF CONSTRUCTION PLANS.

WORK SHALL BE SUBJECT TO INSPECTION BY THE COUNTY INSPECTORS AND DESIGN ENGINEER. SANITARY SEWER CUT SHEETS SHALL BE SUBMITTED TO THE BOTETOURT COUNTY ENGINEER.

5. CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND UNCOVERING ALL VALVE BOXES AFTER SURFACE TREATMENT OF ROADS AND ADJUSTING BOXES TO FINAL ROAD GRADES, IF

6. ALL EXISTING UTILITIES MAY BE SHOWN OR MAY NOT BE SHOWN IN THE EXACT LOCATION. THE CONTRACTOR SHALL COMPLY WITH THE STATE WATER WORKS REGULATIONS, SECTION 12.05.03 WHERE

7. THE CONTRACTOR SHALL NOTIFY THE COUNTY AND OBTAIN COUNTY APPROVAL OF ANY FIELD CORRECTION TO THE APPROVED PLANS PRIOR TO SUCH CONSTRUCTION.

8. ALL TRENCHES WITHIN THE EXISTING OR FUTURE VIRGINIA STATE DEPARTMENT OF HIGHWAYS AND TRANSPORTATION RIGHT-OF-WAY MUST BE COMPACTED IN SIX INCH LAYERS.

9. ALL LINES TO BE STAKED PRIOR TO CONSTRUCTION.

10. CONTRACTOR TO COORDINATE WITH THE ENGINEER TO PROVIDE AS-BUILT PLANS CONTRACTOR SHALL MAINTAIN A SET OF RED-LINE PLANS SHOWING AS-BUILT LOCATION OF ALL STRUCTURES. AS-BUILT INFORMATION TO BE SUBMITTED TO DESIGN ENGINEER FOR PREPARATION OF RECORD AS-BUILT PLANS. SUCH AS-BUILT PLANS SHALL BE SUBMITTED TO BOTETOURT COUNTY PRIOR TO COUNTY ACCEPTANCE.

11. ALL CONSTRUCTION SHALL BE IN ACCORDANCE TO APPROVED CONSTRUCTION PRACTICES OF THE APPLICABLE TRADES.

UNLESS NOTED OTHERWISE HEREIN ALL CONSTRUCTION SHALL BE IN ACCORDANCE TO THE LATEST EDITION OF AWWA STANDARDS.

EXCAVATION, STABILIZATION AND BEDDING

1. EXCAVATION FOR TRENCHES SHALL INCLUDE THE REMOVAL OF ALL MATERIAL ENCOUNTERED REGARDLESS OF CLASSIFICATION IN ACCORDANCE WITH THE ELEVATIONS AND GRADES AT THE LOCATIONS AND STATIONS INDICATED ON THE PLANS OR

2. EXCAVATION, UNLESS OTHERWISE SPECIFIED, SHALL BE OPEN CUT. THE CONTRACTOR SHALL OPEN NO MORE THAN TWO HUNDRED (200) FEET OF TRENCH AT ONE TIME DURING THE LAYING OF

TRENCHES SHALL BE EXCAVATED IN STRAIGHT LINES AND SHALL BE ACCURATELY GRADED IN ORDER TO ESTABLISH A TRUE ELEVATION FOR THE INVERT OF THE PIPE.

4. THE WDTH OF TRENCHES, FROM EXISTING GRADE TO ONE (1) FOOT ABOVE THE TOP OF THE PIPE SHALL BE OF SUFFICIENT WDTH TO PERMIT THE PROPER INSTALLATION OF BRACING. SHORING OR SHEETING.

5. THE SIDES OF THE TRENCHES SHALL BE AS VERTICAL AS

6. EXCAVATION FOR STRUCTURES SHALL ALLOW A MINIMUM OF TWELVE (12) INCHES CLEAR BETWEEN THE STRUCTURE AND THE SIDES OF HÉ TRENCH OR ANY REQUIRED BRACING, SH**ORING OR SHEET**ING.

7. EXCAVATED MATERIALS SUITABLE FOR BACKFILL SHALL BE STOCKPILED IN AN ORDERLY MANNER AT A SUFFICIENT DISTANCE FROM THE SIDES OF THE TRENCH IN ORDER TO AVOID OVERLOADING THE BANKS OF THE TRENCH AND TO PREVENT SLIDES

8. EXCAVATED MATERIALS WHICH ARE NOT REQUIRED OR APPROVED FOR BACKFILL SHALL BE REMOVED FROM THE SITE OF BY THE CONTRACTOR, AT HIS EXPENSE.

9. CONTRACTOR TO ADHERE TO ALL LOCAL, STATE AND FEDERAL CONSTRUCTION LAWS, INCLUDING OSHA TRENCH SAFETY REGULATIONS.

B. TRENCH STABILIZATION

TRENCH STABILIZATION MATERIAL SHALL BE COARSE AGGREGATE SIZE NUMBER 2 AND SHALL CONFORM WITH VDOT SECTION 203

MHENEVER EXCESSIVELY WET OR UNSTABLE MATERIAL IS ENCOUNTERED IN THE BOTTOM OF THE TRENCH, WHICH IN THE OPINION OF THE ENGINEER IS INCAPABLE OF PROPERLY SUPPORTING THE PIPE OR STRUCTURES, SUCH MATERIAL SHALL BE REMOVED AND BACKFILLED WITH TRENCH STABILIZATION MATERIAL AND SHALL BE GRADED TO ALLOW FOR THE COMPACTED BEDDING

 ALL UNAUTHORIZED OVERDEPTHS OF EXCAVATION SHALL BE BACKFILLED, AT THE CONTRACTOR'S EXPENSE, WITH TRENCH STABILIZATION MATERIAL AND SHALL BE GRADED TO ALLOW FOR THE COMPACTED BEDDING MATERIAL.

C. COMPACTED BEDDING MATERIAL

BEDDING MATERIAL SHALL BE COARSE AGGREGATE SIZE NUMBER 57 AND SHALL CONFORM WITH VDOT SECTION 203 AND/OR ASTM C 33. 2. THE BOTTOM OF THE PIPE TRENCH SHALL BE EXCAVATED TO

MINIMUM OVERDEPTH OF SIX (6) INCHES BELOW THE BOTTOM OF THE PIPE, TO PROVIDE FOR THE COMPACTED BEDDING MATERIAL.

BEDDING MATERIAL SHALL BE PLACED, SHAPED AND COMPACTED. BELL HOLES AND DEPRESSIONS REQUIRED FOR THE JOINTING OF THE PIPE SHALL BE DUG AFTER THE COMPACTED BEDDING MATERIAL HAS BEEN GRADED AND SHAPED AND SHALL BE ONLY OF THE LENGTH, DEPTH AND WIDTH REQUIRED TO MAKE THE JOINT

PIPE, JOINTS AND FITTINGS

A. SCOPE OF WORK

ALL MATERIALS AND APPURTENANCES REQUIRED FOR THE WORK SHALL BE NEW, OR FIRST CLASS QUALITY AND SHALL BE TURNISHED, DELIVERED, ERECTED CONNECTED AND FINISHED IN EVERY DETAIL AS SPECIFIED OR INDICATED. ALL MATERIALS
FOUND DEFECTIVE, REGARDLESS OF THE CIRCUMSTANCES, SHALL BE REPLACED WITH NEW MATERIAL AT THE EXPENSE OF THE

THE MATERIALS SPECIFIED FOR THE CONSTRUCTION SHALL COMPLY WITH THE LATEST REVISIONS OF THE APPLICABLE AMERICAN SOCIETY FOR TESTING MATERIALS (ASTM), AMERICAN NATIONAL STANDARDS INSTITUTE (ANSI) AND/OR THE VIRGINIA DEPARTMENT OF TRANSPORTATION (VDOT) STANDARDS.

 THE CONTRACTOR SHALL INSTALL ONLY ONE (1) TYPE OF PIPE BETWEEN STRUCTURES EXCEPT WHERE DUCTILE IRON PIPE IS SPECIFIED OR INDICATED. WHERE EXISTING PIPE IS TO BE REPLACED OR EXTENDED THE SAME TYPE OF PIPE SHALL BE INSTALLED, UNLESS SPECIFIED OR INDICATED OTHERWISE 2. WATER LINE SHALL BE EITHER PVC OR DUCTILE IRON

3. SANITARY SEWERS WITH AN INSIDE DIAMETER LESS THAN OR EQUAL TO TWELVE (12) INCHES SHALL BE EITHER POLYVINYL CHLORIDE OR DUCTILE IRON PIPE, AT THE CONTRACTOR'S OPTION, UNLESS SPECIFIED OR INDICATED OTHERWISE. CONTRACTOR SHALL OBTAIN APPROVAL OF PIPE MATERIAL BY BOTETOURT COUNTY ENGINEER PRIOR TO BEGINNING

4. SERVICE LATERALS SHALL BE SCHEDULE 40 POLYVINYL CHLORIDE

1. POLYVINYL CHLORIDE (PVC) WATER PIPE SHALL BE AWWA C900 DF 18 MINIMUM, UNLESS SPECIFIED OR INDICATED OTHERWISE. 2. DUCTILE IRON PIPE SHALL CONFORM WITH AWWA C 151/ANS 21.51 AND FITTINGS SHALL CONFORM WITH AWWA C 110/ANSI 21.10. THE PIPE AND FITTINGS SHALL BE BITUMINOUS COATED AND CEMENT LINED IN ACCORDANCE WITH AWWA C 104/ANSI 21.40. THE PIPE THICKNESS SHALL CONFORM WITH AWWA C 150/ANSI 21.50 AND SHALL BE CLASS 50, AS A MINIMUM, UNLESS SPECIFIED OR INDICATED OTHERWISE.

3. PVC SEWER PIPE AND FITTINGS SHALL BE SDR 35 (ASTM D

D. JOINTS COUPLINGS AND APPURTENANCES

1. PVC PIPE AND :FITTINGS SHALL BE BELL AND SPIGOT TYPE JOINTS. THE BELL AND SPIGOT JOINT SHALL BE SEALED WITH ELASTOMERIC GASKETS CONFORMING TO ASTM D 3212. THE JOINTS SHALL BE MADE IN STRICT ACCORDANCE WITH THE RECOMMENDATION OF THE PIPE MANUFACTURER.

2. DUCTILE IRON PIPE AND FITTINGS SHALL BE EITHER MECHANICAL OR BELL AND SPIGOT TYPE JOINTS AS SPECIFIED OR INDICATED. JOINTS SHALL BE MADE WITH A SINGLE WATERTIGHT RUBBER GASKET MANUFACTURED IN ACCORDANCE WITH AWWA C 111/ANSI 21.11. THE JOINTS SHALL BE MADE IN STRICT ACCORDANCE WITH THE RECOMMENDATIONS OF THE PIPE MANUFACTURER.

3. GATE VALVES SHALL BE IRON-BODY, BRONZE-MOUNTED, DOUBLE-DISC, PARALLEL-SEAL, O-RING SEALED, INSIDE-SCREW, NON-RISING STEM, FITTING WITH 2 INCH SQUARE OPERATING NUT FOR VALVE VAULT SERVICE, ALL IN ACCORDANCE WITH AVVA STANDARD C500 (LATEST REVISION). CONNECTIONS SHALL BE SUITABLE FOR THE PIPE WITH WHICH IT IS USED. THE VALVES SHALL BE SUITABLE FOR THE PIPE WITH WHICH IT IS USED. THE VALVES SHALL BE SUITABLE FOR 200 P.S.I. WATER WORKING PRESSURE AND SHA BE TESTED AT TWICE THE RATED WORKING PRESSURE. ALL GATE VALVES SHALL BE INSTALLED IN VALVE VAULTS AND EQUIPPED WITH A 2-INCH SQUARE OPERATING NUT. THE NUT SHALL BE MARKED WITH AN ARROW AND THE WORD "OPEN" AND SHALL OPEN BY TURNING TO THE RIGHT (CLOCKWISE).

ALL OTHER MATERIALS AND APPURTENCES TO BE IN ACCORDANCE WITH DETAILS SHOWN ON PLANS.

PIPE INSTALLATION

THE CONTRACTOR SHALL NOT LAY PIPE OR PLACE MANHOLES UNTIL ALL WATER HAS BEEN REMOVED FROM THE TRENCH, OR WHEN IN THE OPINION OF THE ENGINEER, THE TRENCH OR THE WEATHER

2. PIPE THAT MAY REQUIRE FIELD CUTTING SHALL BE DONE SO IN A NEAT AND WORKMANLIKE MANNER, SO AS TO LEAVE A SMOOTH END AT RIGHT ANGLES TO THE AXIS OF THE PIPE. CARE SHALL BE TAKEN TO AVOID DAMAGING THE PIPE AND ANY COATINGS OR LININGS. DUCTILE IRON PIPE SHALL NOT BE CUT WITH AN OXYACETYLENE TORCH.

3. THE MATERIALS SHALL BE VISUALLY INSPECTED FOR DEFECTS BEFORE LOWERING THE PIPE OR PLACING THE MANHOLES INTO THE TRENCH. DURING THE LAYING OPERATION NO TOOLS, CLOTHING OR OTHER MATERIAL SHALL BE PLACED IN THE PIPE OR MANHOLE. THE INTERIOR OF THE PIPE SHALL BE CLEAR OF ALL SOIL, DEBRIS AND SUPERFLUOUS MATERIALS PRIOR TO AND DURING THE

4. THE CONTRACTOR SHALL EXERCISE EVERY PRECAUTION TO PREVENT FOREIGN MATERIAL FROM ENTERING THE PIPE WHIL IT IS BEING PLACED IN THE TRENCH. FAILURE BY THE CONTRACTOR TO TAK SUCH PRECAUTIONS MAY RESULT IN THE ENGINEER REQUIRING A HEAVY, TIGHTLY WOVEN CANVAS BAG OF SUITABLE SIZE BE PLACED OVER EACH END OF THE PIPE AND REMOVED ONLY WHEN THE JOINT CAN BE MADE PROPERLY.

5. THE PIPE AND MANHOLES SHALL BE LOWERED CAREFULLY INTO THE TRENCH BY SUITABLE MEANS AND HANDLED WITH CARE AT ALL. TIMES TO AVOID DAMAGE. UNDER NO CIRCUMSTANCES SHALL THE MATERIALS BE DROPPED OR DUMPED INTO THE TRENCHES.

WHEN WORK IS NOT IN PROGRESS, THE CONTRACTOR SHALL PLUG THE OPEN ENDS OF THE PIPE TO PREVENT TRENCH WATER OR OTHER SUBSTANCES FROM ENTERING THE PIPE. THE PLUG SHALL BE WATERTIGHT AND SHALL REMAIN IN PLACE UNTIL ANY REQUIRED DEWATERING HAS BEEN COMPLETED.

7. PARALLEL INSTALLATION - WATER LINES SHALL BE LAID AT LEAST TEN FEET HORIZONTALLY FROM A SEWER OR SEWER MANHOLE WHENEVER POSSIBLE. WHEN LOCAL CONDITIONS PREVENT A HORIZONTAL SEPARATION OF TEN FEET, THE WATER LINE MAY BE LAID CLOSER TO A SEWER OR SEWER MANHOLE PROVIDED THAT:

THE INVERT OF THE WATER MAIN SHALL BE AT LEAST 18 INCHES ABOVE THE CROWN OF THE SEWER.

WHERE THIS VERTICAL SEPARATION CANNOT BE OBTAINED, THE SEWER SHALL BE CONSTRUCTED OF AWWA APPROVED

III. THE SEWER MANHOLE SHALL BE OF WATER-TIGHT CONSTRUCTION AND TESTED IN PLACE.

WATER PIPE, PRESSURE TESTED IN PLACE WITHOUT

CROSSING - WATER LINES CROSSING SEWERS SHALL BE LAID TO PROVIDE A SEPARATION OF AT LEAST 18 INCHES BETWEEN THE BOTTOM OF THE WATER LINE AND THE TOP OF THE SEWER WHENEVER POSSIBLE. WHEN LOCAL CONDITIONS PREVENT VERTICAL SEPARATION, THE FOLLOWING CONSTRUCTION SHALL BE

LEAKAGE PRIOR TO BACKFILLING.

SEWERS PASSING OVER OR UNDER WATER LINES SHALL BE CONSTRUCTED OF AWWA APPROVED WATER PIPE, PRESSURE TESTED IN PLACE WITHOUT LEAKAGE PRIOR TO

II. WATER LINES PASSING UNDER SEWERS SHALL, IN ADDITION, BE PROTECTED BY PROVIDING:

(A) A VERTICAL SEPARATION OF AT LEAST 18 INCHES BETWEEN THE BOTTOM OF THE SEWER AND THE TOP OF THE WATER LINE.

THE SETTLING ON AND BREAKING OF THE WATERLINE, (C) THAT THE LENGTH OF THE WATER LINE BE CENTERED AT THE POINT OF THE CROSSING SO THAT JOINTS SHALL BE EQUAL DISTANCE AND AS FAR AS POSSIBLE

(B) ADEQUATE STRUCTURAL SUPPORT FOR THE SEWERS TO PREVENT EXCESSIVE DEFLECTION OF THE JOINTS AND

NO WATER PIPES SHALL PASS THROUGH OR COME IN CONTACT WITH ANY PART OF A SANITARY SEWER MANHOLE. ALL SANITARY SEWER MANHOLES SHALL BE PLACED A MINIMUM OF 10 FEET HORIZONTALLY FROM ALL WATER MAINS WHENEVER POSSIBLE. WHEN THIS HORIZONTAL SEPERATION CAN NOT BE MAINTAINED, THE MANHOLE SHALL BE OF WATERTIGHT CONSTRUCTION

BEFORE JOINTS ARE MADE THE PIPE SHALL BE WELL BEDDED ON A FIRM FOUNDATION AND NO PIPE SHALL BE BROUGHT INTO POSITION UNTIL THE PRECEDING LENGTH HAS BEEN THOROUGHLY MBEDDED AND SECURED IN PLACE. ANY DEFECTS DUE TO SETTLEMENT SHALL BE MADE GOOD BY THE CONTRACTOR AT HIS EXPENSE. BELL HOLES SHALL BE DUG SUFFICIENTLY LARGE TO INSURE THE MAKING OF PROPER JOINTS.

9. PIPE SHALL BE JOINTED IN FULL ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. PUSH—ON JOINTS SHALL BE THOUROUGHLY CLEANED, THE RUBBER GASKET INSERTED IN THE BELL SOCKET, A THIN FILM OF APPROVED GASKET LUBRICANT APPLIED, THE SPIGOT END OF THE PIPE CENTERED INTO THE SPIGOT END OF THE PIPE CENTERED INTO THE SPIGOT END OF THE PIPE CENTERED INTO THE SOCKET AND THE JOINT COMPLETED BY FORCING THE SPIGOT END SOCKET AND THE JOINT COMPLETED BY FORCING THE SPIGOT END TO THE BOTTOM OF THE SOCKET BY A JACK-TYPE TOOL OR OTHER DEVICE APPROVED BY THE ENGINEER. MECHANICAL JOINTS SHALL BE THOROUGHLY CLEANED, THE GLAND SUPPED OVER THE SPIGOT END OF THE PIPE, THE RUBBER GASKET PAINTED WITH SOAP SOLUTION AND PLACED ON THE SPIGOT END, THE SPIGOT END OF THE PIPE SEATED IN THE BELL, THE GASKET PRESSED INTO PLACE WITHIN THE BELL, THE GASKET PRESSED INTO PLACE WITHIN THE BELL, THE GLAND MOVED INTO POSITION, AND BOLTS AND NUTS ASSEMBLED BY HAND AND TIGHTENED WITH AN APPROVED TORQUE-LIMITING WRENCH.

1. THE WATER MAIN SHALL BE LAID AND MAINTAINED AT THE REQUIRED LINES AND GRADES WITH FITTINGS AND VALVES AT THE

2. DEFLECTION OF THE LINE OF PIPE, IN EITHER, THE VERTICAL OR HORIZONTAL PLANE TO AVOID OBSTRUCTIONS, OR IN LOCATIONS WHERE LONG-RADIUS CURVES ARE REQUIRED, TH AMOUNT OF DEFLECTION SHALL NOT EXCEED APPROVED AWWA STANDARDS. ALIGNMENT THAT MAY REQUIRE REFLECTIONS IN EXCESS OF THE RECOMMENDED LIMITATIONS, SPECIAL BENDS, OR A SUFFICIENT NUMBER OF SHORTER LENGTHS OF PIPE TO PROVIDE THE ANGULAR DEFLECTIONS WITHIN THE LIMITS AS SET FORTH, SHALL BE APPROVED BY THE ENGINEER.

3. ALL PLUGS, EXCEPT MECHANICAL JOINT PLUGS AT CONNECTIONS FOR FUTURE LINES, ALL TEES, AND ALL BENDS IN WATER MAINS UNDER PRESSURE SHALL BE PROVIDED WITH REACTION BACKING CONSISTING OF CONCRETE THRUST BLOCKS. VALVES FOR CONNECTIONS TO FUTURE LINES AND FIRE HYDRANTS SHALL BE ANCHORED TO THE WATER MAIN WITH THE RODS.

4. DETECTION TAPE TO BE INSTALLED 12"-18" ABOVE ALL NEW PVC WATER LINES.

ALL PIPE SHALL BE DISINFECTED, TESTED AND FLUSHED IN ACCORDANCE WITH AWWA STANDARD C601 (LATEST REVISION). 2. CONTRACTOR SHALL PROVIDE ALL MATERIALS, EQUIPMENT, NECESSARY TAPS AND PERFORM ALL WORK REQUIRED FOR THE STERILIZATION, TESTING AND FLUSHING OF THE WATER MAIN.

NO TESTED SECTION OF WATER LINE SHALL BE APPROVED TO DELIVER WATER SERVICE UNTIL A FAVORABLE LABORATORY REPORT HAS BEEN ACHIEVED. ANY TESTED SECTION OF WATER LINE FAILING TO MEET THE REQUIREMENTS SPECIFIED SHALL BE REQUIRED BY THE CONTRACTOR AND RETESTED UNTIL THE RESULTS

4. THE WATER MAIN OR VALVED OFF SECTION THAT HAS BEEN COMPLETED SHALL BE FILLED, TESTED AND FLUSHED. TEST LOCATIONS SHALL BE SUBJECT TO THE DISCRETION OF THE Engineer and as valves and blow-offs permit

COMPLETED SYSTEMS, WATER MAINS AND SERVICE LATERALS SHALL BE FLUSHED AND DISINFECTED IN ACCORDANCE WITH AWWA SPECIFICATIONS C601(LATEST REVISION). FLUSHING SHALL BE ACCOMPLISHED AT A FLOW VELOCITY OF NOT LESS THAN 2.5 FEET

DISINFECTION AS DESCRIBED IN AWWA C651 - "PLACING OF CALCIUM HYPOCHLORITE TABLETS" SHALL BE USED. 5 GRAM CALCIUM HYPOCHLORITE TABLETS WITH 3.25 GRAM AVAILABLE CHLORINE PER TABLET SHALL BE ATTACHED AT THE INSIDE TOP OF THE PIPE BY AN ADHESIVE SUCH AS PERMATEX NO. 2 OR EQUAL. THE FOLLOWING NUMBER OF TABLETS FOR THE GIVEN PIPE SIZE SHALL BE USED FOR AN INITIAL DOSE OF 25 MG/1

PIPE DIAMETER

OR THE NUMBER OF TABLETS EQUAL TO 0.0012D2L ROUNDED T THE NEXT HIGHER INTEGER, WHERE D IS THE INSIDE DIAMETER, IN INCHES AND L IS THE LENGTH OF THE PIPE SECTION, IN FEET. USE OF THE CONTINUOUS FEED OR SLUG METHOD OF DISINFECTING MAY ONLY BE USED TO RE-CHLORINATE A WATER PIPE AFTER THE INITIAL DISINFECTION OR IN OTHER SPECIFIC CASES APPROVED BY THE DESIGN ENGINEER. WHEN FILLING THE PIPELINE FOR DISINFECTION, THE RATE OF FILLING MUST RESULT IN A VELOCITY OF LESS THAN 1 FT./SEC.

THE DISINFECTION SOLUTION SHALL REMAIN IN THE PIPE LINE FOR NOT LESS THAN TWENTY-FOUR (24) HOURS, AFTER WHIC TIME A CHLORINE RESIDUAL OF 10 PPM AT ALL PARTS OF THE LINE SHALL BE REQUIRED.

FOLLOWING CHLORINATION, THE PIPING SHALL BE THOROUGHLY FLUSHED. THE MRGINIA WATERWORKS REGULATIONS REQUIRE AT LEAST TWO CONSECUTIVE SATISFACTORY BACTERIOLOGICAL LEAST TWO CONSECUTIVE SATISFACTORY BACTERIOLOGICAL SAMPLES AT 24 HOUR INTERVALS FROM THE DISTRIBUTION SYSTEM AT MAXIMUM SPACING OF 2000 FEET BEFORE THE SYSTEM CAN BE PLACED IN SERVICE. IF THE INITIAL TESTING IS NOT SATISFACTORY THE NEW LINES WILL BE RETESTED UNTIL SATISFACTORY RESULTS ARE ACHIEVED. THE CONTRACTOR SHALL PAY ALL COSTS ASSOCIATED WITH DISINFECTION AND TESTING OF INISTALLED FACILITIES INCLUDING ANY BACTERIOLOGICAL INSTALLED FACILITIES INCLUDING ANY BACTERIOLOGICAL SAMPLES AND RETESTING IF REQUIRED. SAMPLES WILL BE COLLECTED IN ACCORDANCE WITH THE VIRGINIA WATERWORKS

D. INSTALLING SEWER PIPE & MANHOLES

1. THE INSTALLATION OF THE SANITARY SEWER SYSTEM SHALL BEGIN AT THE DOWNSTREAM MANHOLE AND PROCEED UPSTREAM. THE DOWNSTREAM SECTIONS SHALL BE COMPLETED, TESTED, AN APPROVED PRIOR TO ALLOWING SANITARY SEWAGE TO ENTER THE

2. THE PIPE SHALL BE INSTALLED IN ACCORDANCE WITH THE PIPE MANUFACTURER'S RECOMMENDATIONS AND AS DIRECTED BY THE ENGINEER. THE PIPE SHALL BE LAID IN TRUE STRAIGHT LINES WITH THE BELL ENDS UPSTREAM AND WITH THE INVERT OF THE PIPE BEING THE TRUE ELEVATION AND GRADE OF THE SYSTEM.

3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING AND MAINTAINING THE HORIZONTAL ALIGNMENT AND VERTICAL ELEVATION AND GRADE OF THE SYSTEM IN ACCORDANCE WITH THE

SURVEY INFORMATION INDICATED ON THE PLANS 4. THE HORIZONTAL ALIGNMENT OF THE PIPE SHALL BE MAINTAINED BY A TRANSIT OR THEODOLITE PLUMBED OVER THE CENTER OF THE DOWNSTREAM MANHOLE. THE VERTICAL ELEVATION AND GRADE SHALL BE MAINTAINED BY NOT LESS THAN THREE (3) BATTER
BOARDS PLACED BETWEEN MANHOLES OR BY AN ADJUSTABLE LASER
LEVEL MOUNTED AT THE INVERT OF THE DOWNSTREAM MANHOLE
WITH TARGET(S) PLACED IN THE BELL END OF THE PIPE BEING

SEWER PIPE SHALL BE INSTALLED IN 4 INCH GRAVEL BEDDING EXTENDING TO THE SPRINGLINE OF PIPE AND IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.

6. THE SANITARY SEWER SYSTEM SHALL BE LAID AND JOINED COMPLETE—IN-PLACE SO THAT EACH LENGTH AND SECTION OF PIPE BETWEEN THE MANHOLES SHALL HAVE A SMOOTH AND UNIFORM

7. THE PIPE SHALL BE CONNECTED TO MANHOLES THROUGH PRECAST OPENINGS AND JOINED WITH EITHER A FLEXIBLE BOOT ADAPTER OR A PIPE SEAL GASKET.

8. DETECTION TAPE TO BE INSTALLED 12" TO 18" ABOVE ALL NEW SEWER PIPE MAINS AND SEWER SERVICE LATERALS.

E. CONNECTION TO EXISTING SYSTEMS 1. THE NEW PIPE CONNECTION TO BE MADE TO AN EXISTING MANHOLE WHERE NO STUB OR OPENING EXISTS, SHALL BE MADE THROUGH AN OPENING OF MAXIMUM DIAMETER CUT INTO THE MANHOLE WALL AT

2. THE EXISTING INVERT CHANNELS AND BENCHES SHALL BE REWORKED AS REQUIRED TO FORM A NEW FLOW CHANNEL FROM THE NEW CONNECTION TO THE EXISTING FLOW CHANNEL.

3. THE NEW PIPE CONNECTED TO AN EXISTING MANHOLE SHALL BE SECURED IN POSITION AND THE REMAINING OPENING SHALL BE FILLED AND SEALED WITH BRICK AND MORTAR. THE OUTER SURFACE OF THE CONNECTION SHALL BE GIVEN A COAT OF HEAVY BITUMASTIC WATERPROOFING COMPOUND.

THE CONTRACTOR SHALL MAKE ALL SERVICE CONNECTIONS TO THE SEWER PIPE AND FROM MANHOLES WHERE SHOWN ON THE PLANS AND/OR WHERE LOCATED IN THE FIELD. THE SERVICE CONNECTIONS TO THE SEWER PIPE SHALL BE MADE WITH A WYE OR

2. THE WYE AND TEE WYE BRANCH FITTINGS FOR SERVICE CONNECTIONS SHALL BE COMMERCIALLY MANUFACTURED AND INSTALLED IN STRICT ACCORDANCE WITH THE RECOMMENDATIONS OF THE PIPE MANUFACTURER.

3. THE SEWER PIPE SHALL NOT BE CUT OR TAPPED FOR SERVICE CONNECTIONS EXCEPT WHEN AND WHERE PERMITTED BY THE 4. ALL SERVICE CONNECTIONS SHALL BE MADE WITH FOUR (4) INCH PIPE AS A MINIMUM, UNLESS THE SIZE OF AN EXISTING SERVICE CONNECTION DICTATES OTHERWISE, AND SHALL BE INSTALLED ON A MINIMUM GRADE OF ONE—QUARTER (1/4) INCH PER ONE (1) FOOT FROM THE SEWER PIPE OR MANHOLE TO THE PROPERTY OR

5. FUTURE SERVICE CONNECTIONS SHALL EXTEND TO THE PROPERT OR EASEMENT LINE WITH CLEANOUT AND BE PROPERLY CAPPED WITH A WATERTIGHT FITTING TO PREVENT INFILITRATION INTO THE SEWERAGE SYSTEM. THE FITTING SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE RECOMMENDATIONS OF THE PIPE MANUFACTURER.

6. FUTURE SERVICE CONNECTIONS SHALL BE FIELD MARKED BY A TREATED, SOLID WOODEN (2 X 4) MARKER THREE (3) FEET LONG SET VERTICALLY PLUMB WITH THE END OF THE CAPPED EXTENSION. THE TOPS OF THE MARKERS SHALL BE PAINTED YELLOW AND SET 24" ABOVE THE FINISHED GRADE. THE LOCATION AND INVERT DEPTH OF THE SERVICE CONNECTION SHALL

BACKFILLING

BE SHOWN ON THE AS-BUILT PLANS.

A. JOB CONDITIONS

1. PRIOR TO PLACING BACKFILL, ALL ORGANIC, RUBBISH, DEBRIS, OR TRENCH SHALL BE REMOVED. ALL CONCRETE FORMS SHALL BE REMOVED. ALL SHORING OR SHEETING SHALL BE REMOVED OR CUT OFF AT THE DEPTH STIPULATED BY THE ENGINEER.

2. PRIOR TO PLACING BACKFILL, THE TRENCH BOX SHALL BE REMOVED. ALL CONCRETE FORMS SHALL BE REMOVED. ALL HORING OR SHEETING SHALL BE REMOVED OR CUT OFF AT THE DEPTH STIPULATED BY THE ENGINEER. 3. BACKFILL MATERIAL SHALL BE PLACED IN UNIFORM HORIZONTAL LAYERS AND THOROUGHLY COMPACTED WITH PROPER MECHANICAL OR HAND OPERATED TAMPERS OR OTHER EQUIPMENT AS APPROVED BY

HE ENGINEER TO PERFORM SUCH WORK. BACKFILL MATERIAL SHALL BE PLACED AND COMPACTED SO AS TO NOT UNEVENLY SUPPORT, DAMAGE OR DISPLACE THE ALIGNMENT OF THE PIPE OR STRUCTURES.

5. BACKFILL SHALL NOT BE PLACED OR COMPACTED AGAINST CAST IN PLACE CONCRETE UNTIL IT HAS OBTAINED SUFFICIENT STRENGTH TO WITHSTAND THE BACKFILLED PRESSURE PLACED UPON IT. 6. UPON THE COMPLETION OF BACKFILLING, ALL EXCESS SOI STONES AND DEBRIS SHALL BE REMOVED FROM THE SITE AND DISPOSED OF BY THE CONTRACTOR.

B. BACKFILL MATERIAL

MATERIALS FOR BACKFILL SHALL BE APPROVED EXCAVATED MATERIAL OR APPROVED SUITABLE MATERIAL OBTAINED FROM OTHER SOURCES. ALL MATERIAL SHALL BE APPROVED BY A SOILS

2. MATERIAL SHALL CONSIST OF DURABLE NATURAL GRANULAR MATERIAL OR GRANULAR AGGREGATES FREE OF ORGANIC MATERIAL, LOAM, DEBRIS, OR OTHER OBJECTIONABLE MATERIAL WHICH ANNOT BE THOROUGHLY COMPACTED.

MATERIAL SHALL NOT CONTAIN STONES LARGER IN DIAMETER THAN THOSE SPECIFIED HEREIN, GRANITE, BROKEN CONCRETE, MASONRY RUBBLE OR OTHER MATERIAL WHICH IN THE OPINION OF THE

ENGINEER IS UNSUITABLE FOR BACKFILL. . EXCESSIVELY WET EXCAVATED MATERIAL SHALL NOT BE USED AS BACKFILL. FROZEN MATERIAL SHALL NOT BE PLACED IN THE TRENCH, NOR SHALL APPROVED BACKFILL BE PLACED UPON FROZEN MATERIAL. HOWEVER, BACKFILLING MAY BE ALLOWED IN REEZING WEATHER WITH PRIOR APPROVAL OF THE ENGINEER.

C. BACKFILL BELOW UNPAVED AREAS

THE PIPE TRENCH TO ONE (1) FOOT ABOVE THE TOP OF THE PIPE SHALL BE FREE OF STONES LARGER THAN ONE (1) INCH IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED SIX (6) INCHES AND COMPACTED WITH HAND OPERATED TAMPERS.

2. BACKFILL FROM ONE (1) FOOT ABOVE THE TOP OF THE PIPE TO THE TOPSOIL SUBGRADE SHALL BE FREE OF STONES LARGER THAN FIVE (5) INCHES IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED TWELVE (12) INCHES AND COMPACTED WITH

3. DRAINAGE CHANNELS TO BE CONSTRUCTED OF FILL MATERIAL SHALL BE GRADED AND SHAPED TO THE TOPSOIL SUBGRADE WITH MATERIAL FREE OF STONES LARGER THAN FOUR (4) INCHES IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED EIGHT (8) INCHES AND COMPACTED WITH MECHANICAL TAMPERS

D. BACKFILL BELOW EXISTING OR NEW PAVED AREAS AND SIDEWALKS BACKFILL FROM THE TOP OF THE PIPE BEDDING OR BOTTOM OF THE PIPE TRENCH TO ONE (1) FOOT ABOVE THE TOP OF THE PIPE SHALL BE FREE OF STONES LARGER THAN ONE (1) INCH IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED SIX

2. BACKFILL FROM ONE (1) FOOT ABOVE THE TOP OF THE PIPE TO THE PAVEMENT SUBGRADE SHALL BE FREE OF STONES LARGER THAN FOUR (4) INCHES IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED EIGHT (8) INCHES AND COMPACTED WITH

INSPECTION AND TESTS

A. TESTING OF SANITARY SEWER

1. THE CONTRACTOR SHALL PROVE THE WATERTIGHNESS OF THE SEWER SYSTEM OR PORTIONS THEREOF BY ONE OF THE FOLLOWING TESTS, AT SUCH TIMES AS THE ENGINEER MAY DIRECT. TESTS SHALL BE MADE ONLY IN THE PRESENCE OF THE ENGINEER. THE CONTRACTOR SHALL FURNISH ALL LABOR AND EQUIPMENT REQUIRE FOR THE TEST AND SHALL MAKE REPAIRS NECESSARY UNTIL TEST SHALL BE NOTIFIED OF ALL TESTS 48 HOURS PRIOR TO CONDUCTING SUCH TESTS. ALL TESTS SHALL BE COORDINATED WITH THE DESIGN ENGINEER FOR HIS ATTENDANCE AND

THE TESTING EQUIPMENT, PROCEDURE, AND RESULTS WILL ALL BE SUBJECT TO THE STRICT APPROVAL OF THE ENGINEER. RESULTS OF THE AIR TEST WILL BE REVIEWED FOR COMPLIANCE WITH ASTM DESIGNATION C-828, CURRENT REVISION. THE AIR TEST IS TO BE CONDUCTED BETWEEN TWO (2) CONSECUTIVE MANHOLES. THE TEST EQUIPMENT SHALL CONSIST OF TWO (2) PLUGS (ONE TAPPED AND EQUIPPED FOR AIR INLET CONNECTION), A SHUT-OFF VALVE, A PRESSURE REGULATING VALVE, A PRESSURE REDUCTION VALVE, AND A MONITORING PRESSURE GAUGE HAVING A PRESSURE FROM 0 TO 5 DEL GRADUATED IN 0 10 DEL WITH AN ACCURACY OF BUILS AMMILE OF MONITORING PRESSURE GAUGE HAVING A PRESSURE RANGE FROM O TO PSI, GRADUATED IN 0.10 PSI WITH AN ACCURACY OF PLUS/MINUS .04 PSI. THE TEST EQUIPMENT SHALL BE SET UP OUTSIDE THE MANHOLE FOR EASY ACCESS AND READING. AIR SHALL BE SUPPLIED TO THE TEST SLOWLY AND SHALL BE REGULATED TO PREVENT THE PRESSURE INSIDE THE PIPE FROM EXCEEDING 5.0 PSIG. THE PIPELINE SHALL BE S BE FILLED UNTIL A CONSTANT INTERNAL PRESSURE OF 3.5 PSIG IS MAINTAINED. THE INTERNAL PRESSURE SHALL BE MAINTAINED AT 3.5 PSIG OR SLIGHTLY ABOVE FOR A FIVE (5) MINUTE STABILIZATION PERIOD, AFTER WHICH TIME THE INTERNAL PRESSURE WILL BE ADJUSTED TO 3.5 PSIG, THE AIR SUPPLY SHUT OFF AND THE TEST BEGUN. NO PERSON SHALL REMAIN IN THE MANHOLE WHILE PIPE IS BEING PRESSURIZED OR THROUGHOUT THE TEST FOR SAFETY PURPOSES A PRESSURE DROP OF 1.0 PSI FROM 3.5 TO 2.5 PSIG SHALL BE allowed for the test times specified in the following table, BASED UPON THE DESIGNATED PIPE SIZE AND TEST SEGMENT LENGTH.

BASED ON EQUATIONS FROM ASTM C-828-80 SPECIFICATIONS TIME (MIN: SEC) REQUIRED FOR PRESSURE DROP FROM 3.5 TO 2.5 PSI WHEN TESTING ONE PIPE DIAMETER ONLY.

PIPE DIAMETER, INCHES LENGTH OF 6 8 10 12 15 18 SEGMENT 0:04 0:10 0:18 0:28 0:40 1:02 1:29 0:09 0:20 0:35 0:55 1:19 2:04 2:58 0:013 0:30 0:53 1:23 1:59 3:06 4:27 0:00 0:18 0:40 1:10 1:50 2:28 4:08 5:56 100 0:18 0:40 125 0:22 0:50 1: 28 2: 18 1: 46 2: 45 2: 03 3: 13 150 0:26 0:59 3:58 6:11 2: 21 3: 40 225 0:40 1:29 2: 38 4: 08 275 0:48 1:49 300 0:53 1:59 3:14 4:43 350 1:02 2:19 11:54 6: 03 9: 27 13: 36 6: 48 10: 38 15: 19 400 1:10 2:38 5:14 7:34 11:49 17:01 500 1:28

SHOULD THE 1.0 PSI DROP OCCUR IN LESS TIME THAN THAT SPECIFIED IN THE TABLE, THE SEWER SEGMENT SHALL HAVE FAILED. IF THE TIME REQUIRED FOR THE PRESSURE TO DROP 1.0 PSI IS GREATER THAN

FOR A MORE DETAILED DESCRIPTION OF THE AIR TEST METHOD REFER TO ASTM DESIGNATION C-828, CURRENT REVISION. AN AIR PRESSURE CORRECTION SHALL BE REQUIRED WHEN THE PREVAILING GROUND WATER IS ABOVE THE SEWER LINE BEING TESTED. IT SHALL BE CALCULATED GROUND WATER DEPTH (FT) + 3.5 = STARTING TEST PRESSURE

ENDING TEST PRESSURE - STARTING PRESSURE - 1.0 PSI THERE IS NO CHANGE FROM TIME REQUIREMENTS ESTABLISHED FOR THE

C. MANHOLE VACUUM TESTS MANHOLES SHALL BE TESTED BY VACUUM TEST, AFTER ASSEMBLY BUT PRIOR TO BACKFILLLING. TEST SHALL COMPLY WITH ASTM STANDARDS OR DIVISION OF WATER PROGRAMS WORKING MEMO #550, DATED NOVEMBER 4.
1987. MANHOLES SHALL BE TESTED BY VACUUM ONLY IF
CONSTRUCTED OF PRECAST CONCRETE. TESTING SHALL

NCLUDE ALL CONCRETE RISER, INCLUDING SPACER

RINGS, AND THE JOINT BETWEEN THE CONCRETE AND THE

2. STUBOUTS, MANHOLE BOOTS AND PIPE PLUGS SHALL BE SECURED TO PREVENT MOVEMENT WHILE THE VACUUM IS

INSTALLATION AND OPERATION OF VACUUM EQUIPMENT AND INDICATING DEVICES SHALL BE IN ACCORDANCE WITH EQUIPMENT SPECIFICATIONS FOR WHICH PERFORMANCE INFORMATION HAS BEEN PROVIDED BY THE MANUFACTURER AND APPROVED BY THE VIRGINIA STATE DEPARTMENT OF

4. A MEASURABLE VACUUM OF 10 INCHES OF MERCURY SHALL BE ESTABLISHED IN THE MANHOLE. THE TIME FOR THE VACUUM TO DROP TO NINE INCHES OF MERCURY SHALL BE RECORDED.

MINIMUM ELAPSED TIME FOR A PRESSURE CHANGE OF 1 INCH HG

75 SECONDS

90 SECONDS

5. ACCEPTANCE STANDARDS FOR LEAKAGE SHALL BE ESTABLISHED FROM THE ELAPSED TIME FOR A NEGATIVE PRESSURE CHANGE FROM 10 INCHES TO NINE INCHES OF MERCURY. THE MAXIMUM ALLOWABLE LEAKAGE RATE FOR FOUR-FOOT DIAMETER MANHOLE SHALL BE IN ACCORDANCE

MANHOLE DEPTH 10 FT. OR LESS > 10 FT. BUT < 15 FT > 15 FT. BUT < 25 F

FOR MANHOLES FIVE FEET IN DIAMETER, ADD AN ADDITIONAL 30 SECONDS TO THE TIME REQUIREMENTS FOR 6. IF THE MANHOLE FAILS THE TEST, NECESSARY REPAIRS SHALL BE MADE, AND THE VACUUM TEST AND REPAIRS SHALL BE REPEATED UNTIL THE MANHOLE PASSES THE

IF A MANHOLE JOINT MASTIC IS COMPLETELY PULLED OUT DURING THE TEST, THE MANHOLE SHALL BE DISASSEMBLED AND THE MASTIC REPLACED.

D. MANDREL TEST ALL SEWER LINES SHALL BE TESTED BY PULLING A STANDARD TEST MANDREL BETWEEN TEST SECTIONS.

E. TESTING OF WATER LINES 1. AFTER PLACING ALL HARNESSING AND ALL VALVE SUPPORT CONCRETE, SUFFICIENT BACKFILL SHALL BE PLACED PRIOR TO FILLING THE PIPE WITH WATER AND FIELD TESTING TO PREVENT LIFTING OF THE PIPE. WHEN LOCAL CONDITIONS REQUIRE THAT THE TRENCHES BE BACKFILLED IMMEDIATELY AFTER THE PIPE HAS BEEN LAID, THE TESTING SHALL BE CARRIED OUT AFTER ACKFILLING HAS BEEN COMPLETED BUT PRIOR TO PLACEMENT OF HE PERMANENT SURFACE. AT LEAST FOURTEEN (14) DAYS SHAL ELAPSE AFTER THE LAST VALVE SUPPORT OR HYDRANT BLOCK HAS BEEN CAST (TYPE 1 PORTLAND CEMENT) PRIOR TO TESTING, UNLESS HIGH EARLY STRENGTH CONCRETE (TYPE III) IS USED, IN WHICH CASE THREE (3) DAYS SHALL ELAPSE.

2. ALL TESTING WILL BE PERFORMED IN ACCORDANCE WITH THE AWWA C600-82 OR CURRENT REVISION. 3. PRESSURE TEST: AFTER THE PIPE HAS BEEN LAID, ALL NEWLY LAID PIPE OR ANY VALVED SECTION THEREOF SHALL BE SUBJECTED TO A HYDROSTATIC PRESSURE OF AT LEAST 1.5 TIMES THE WORKING PRESSURE AT THE POINT OF TESTING.

TEST PRESSURE RESTRICTIONS. TEST PRESSURES SHALL: A. NOT BE LESS THAN 1.50 TIMES THE WORKING PRESSURE AT THE HIGHEST POINT ALONG THE TEST SECTION: B. NOT EXCEED PIPE OR THRUST RESTRAINT DESIGN

C. BE OF AT LEAST 2-HOUR DURATION E. NOT EXCEED TWICE THE RATED PRESSURE OF THE VALVES OR HYDRANTS WHEN THE PRESSURE BOUNDARY OF THE TEST SECTION INCLUDES CLOSED GATE VALVES OR HYDRANTS:

EACH VALVED SECTION OF PIPE SHALL BE FILLED WITH PROPERLY DISINFECTED WATER SLOWLY, AND THE SPECIFIED TEST PRESSURE SHALL BE APPLIED BY MEANS OF A PUMP CONNECTED TO THE PIPE IN A MANNER SATISFACTORY TO THE ENGINEER.

F. NOT EXCEED THE RATED PRESSURE OF THE VALVE.

BEFORE APPLYING THE SPECIFIED TEST PRESSURE, AIR SHALL BE EXPELLED COMPLETELY FROM THE PIPE, VALVES, AND HYDRANTS. ALL EXPOSED PIPE, FITTINGS, VALVES, HYDRANTS AND JOINTS OR DEFECTIVE PIPE, FITTINGS, VALVES, OR HYDRANTS THAT ARE DISCOVERED FOLLOWING THE PRESSURE TEST SHALL BE REPEATED UNTIL IT IS SATISFACTORY TO THE ENGINEER.

4. A LEAKAGE TEST SHALL BE CONDUCTED CONCURRENTLY WITH THE PRESSURE TEST. LEAKAGE SHALL BE DEFINED AS THE QUANTITY OF WATER THAT MUST BE SUPPLIED INTO THE NEWLY LAID PIPE. OR ANY VALVED SECTION THEREOF, TO MAINTAIN PRESSURE WITHIN 5 PSI OF THE SPECIFIED TEST PRESSURE AFTER THE AIR IN THE PIPELINE HAS BEEN EXPELLED AND THE PIPE HAS BEEN FILLED WITH WATER. NO PIPE INSTALLATION WILL BE ACCEPTED F THE LEAKAGE IS GREATER THAN THAT DETERMINED BY THE FOLLOWING FORMULA:

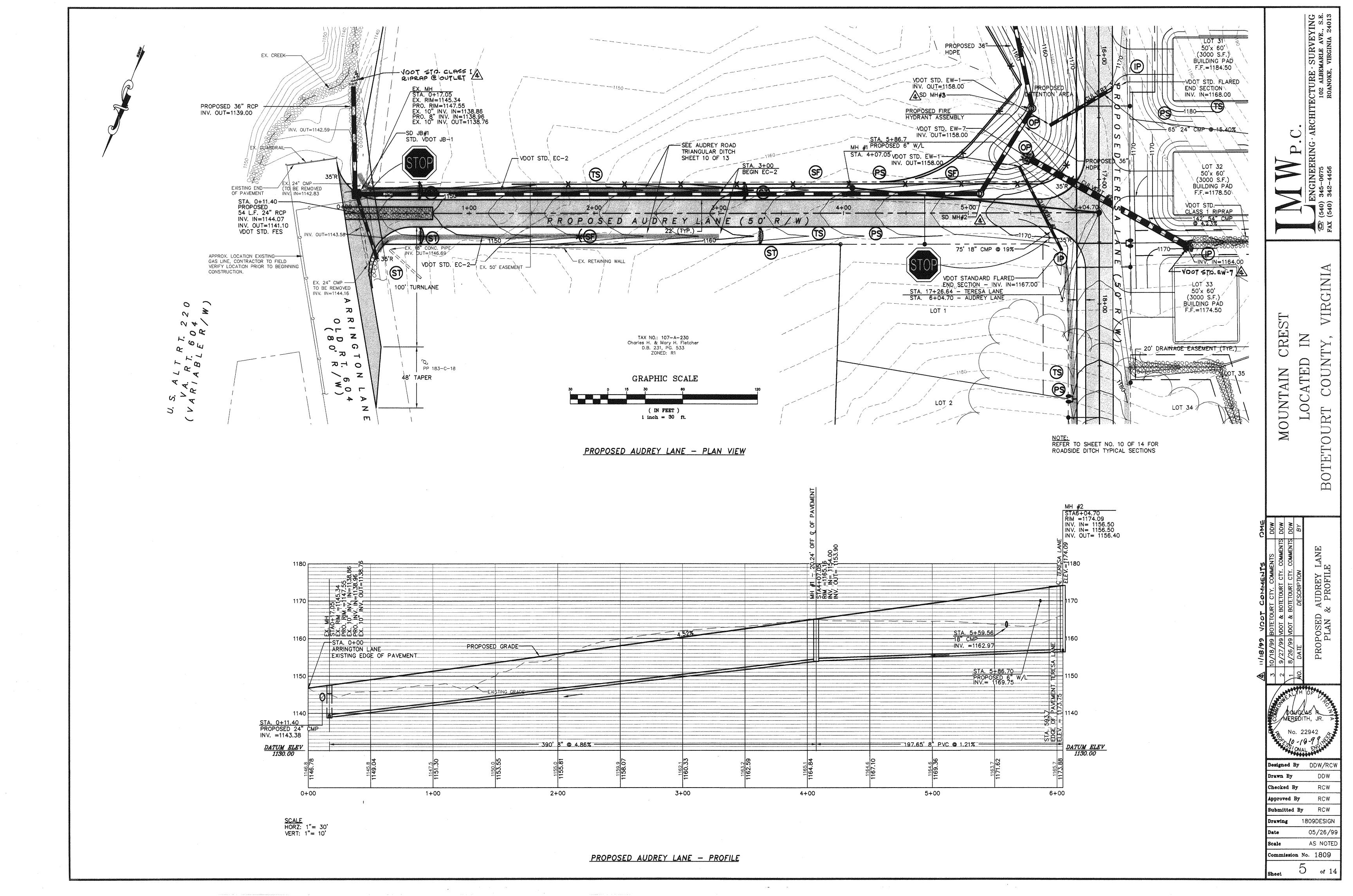
> L = SDV(P)133,200

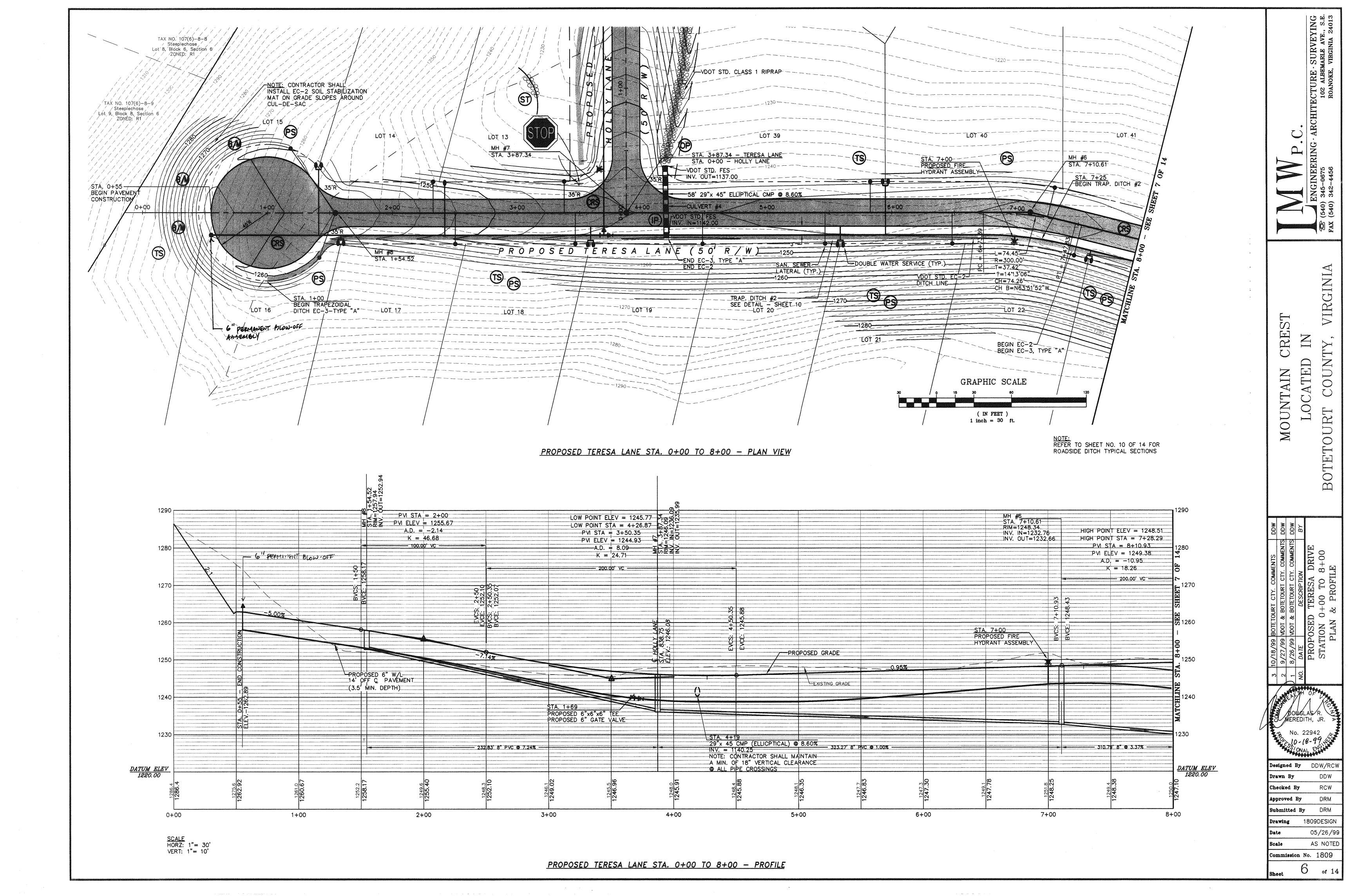
IN WHICH L IS THE ALLOWABLE LEAKAGE, IN GALLONS PER HOUR; S IS THE LENGTH OF PIPELINE TESTED IN FEET; D IS THE NOMINAL DIAMETER OF THE PIPE, IN INCHES; AND P IS THE AVERAGE TEST PRESSURE DURING THE LEAKAGE TEST, IN POUNDS PER SQUARE INCH GAUGE. WHEN TESTING AGAINST CLOSED WETAL-SEATED VALVES, AN ADDITIONAL LEAKAGE PER ENCLOSED VALVE OF 0.0078 GAL/HR/IN, OF NOMINAL VALVE SIZE SHALL BE ALLOWED. WHEN HYDRANTS ARE IN THE TEST SECTION, THE TEST SHALL BE MADE ON THE BASIS OF ALLOWABLE LEAKAGE. IF ANY TEST OF PIPE LAID DISCLOSES LEAKAGE GREATER THAN THI ALLOWABLE AMOUNT, THE CONTRACTOR SHALL, AT HIS OWN EXPENSE, LOCATE AND REPAIR THE DEFECTIVE MATERIAL UNTIL VISIBLE LEAKS ARE TO BE REPAIRED REGARDLESS OF THE AMOUNT

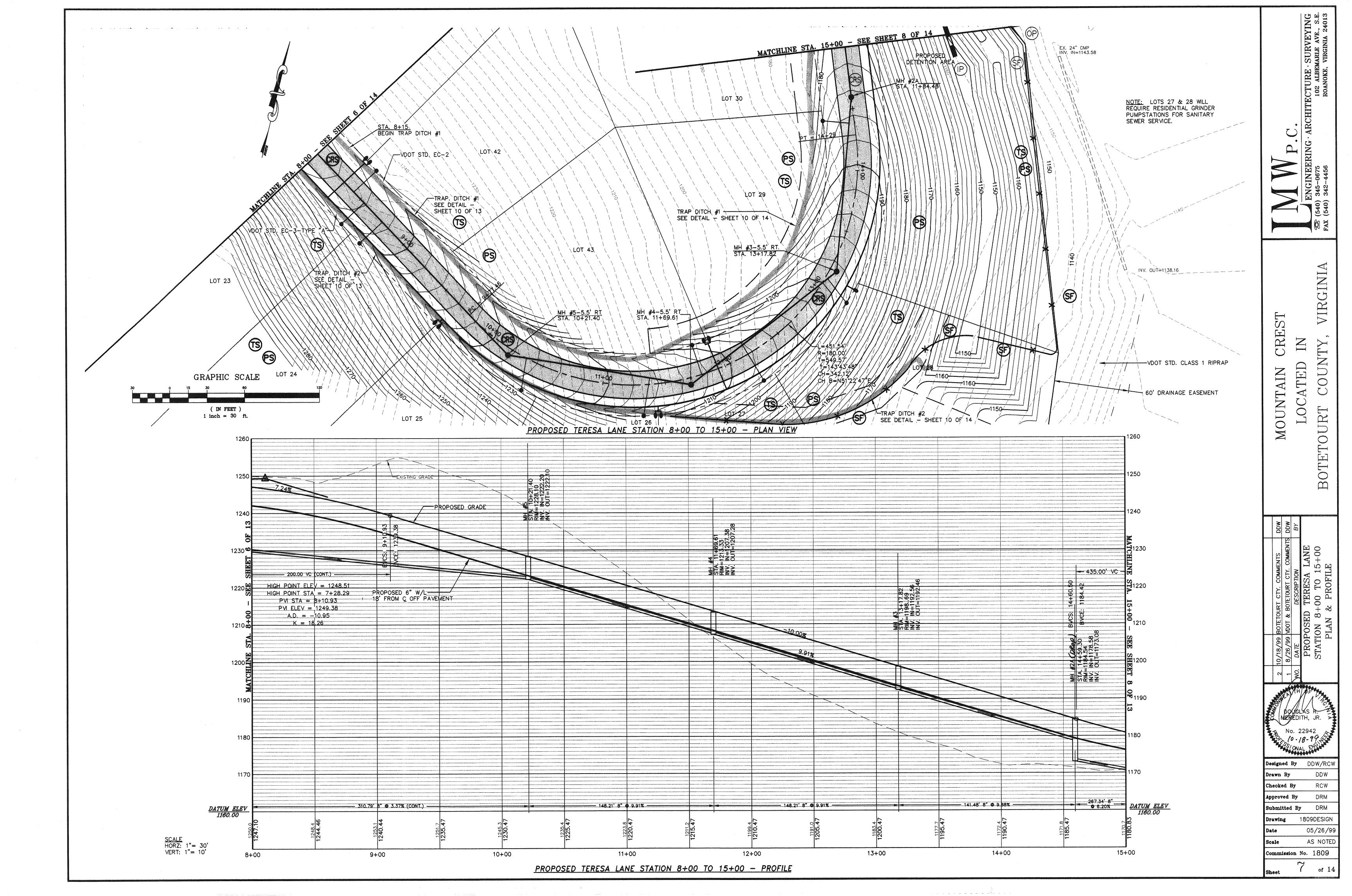
S

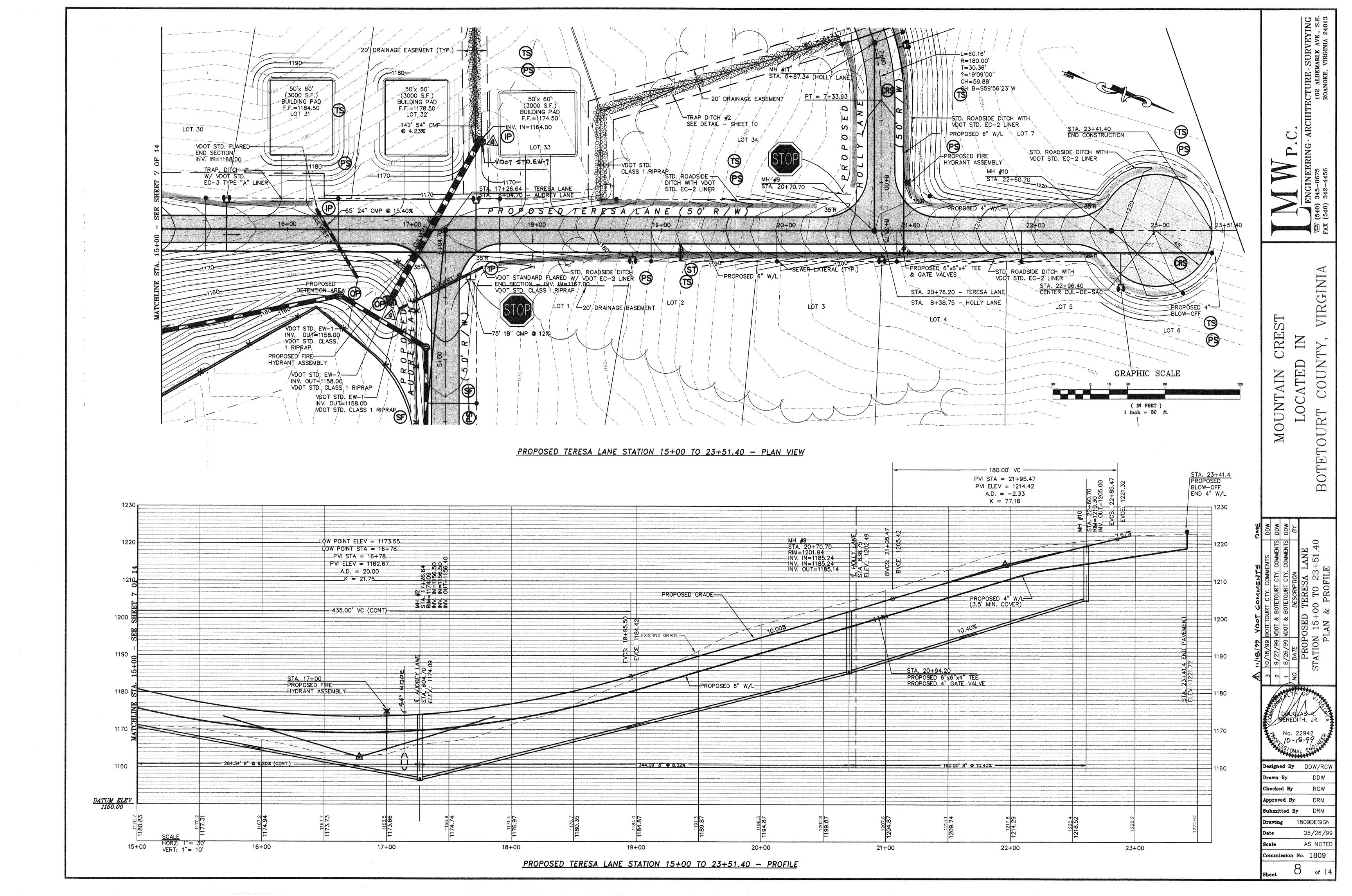
No. 22942 RCW

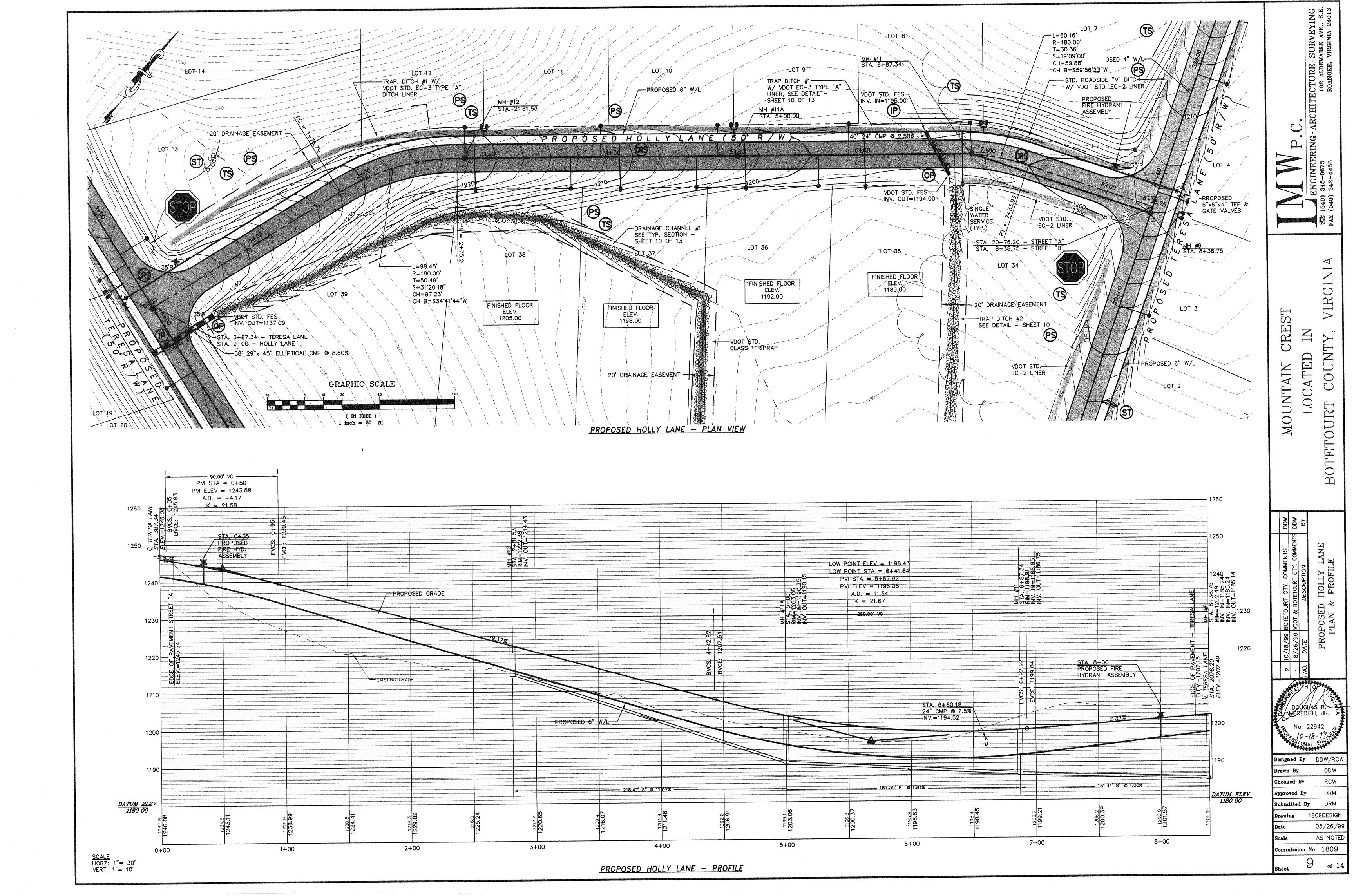
Designed By DDW Drawn By RCW Checked By Approved By RCW RCW Submitted By 1809UTIL Drawing 05/26/99 NONE 1809 Comm. No:

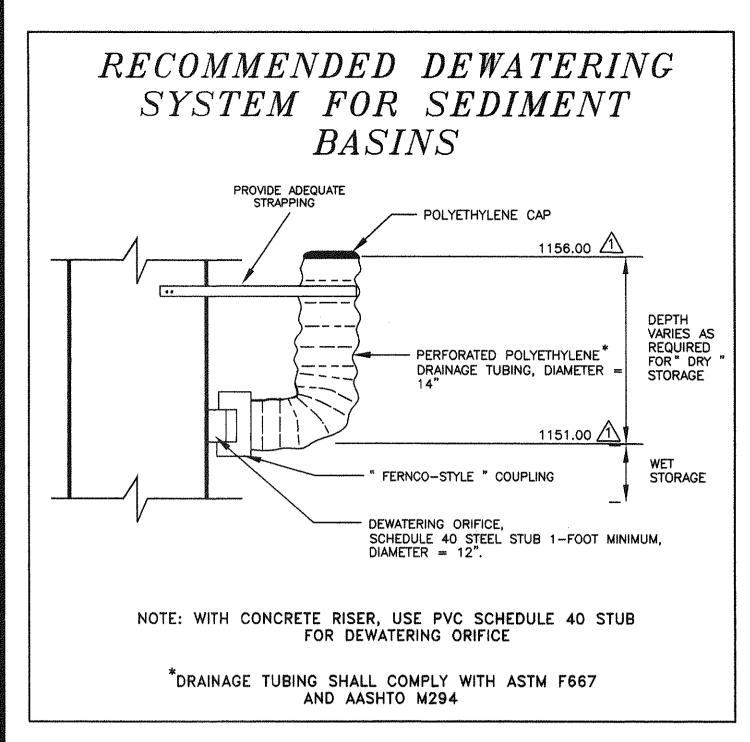


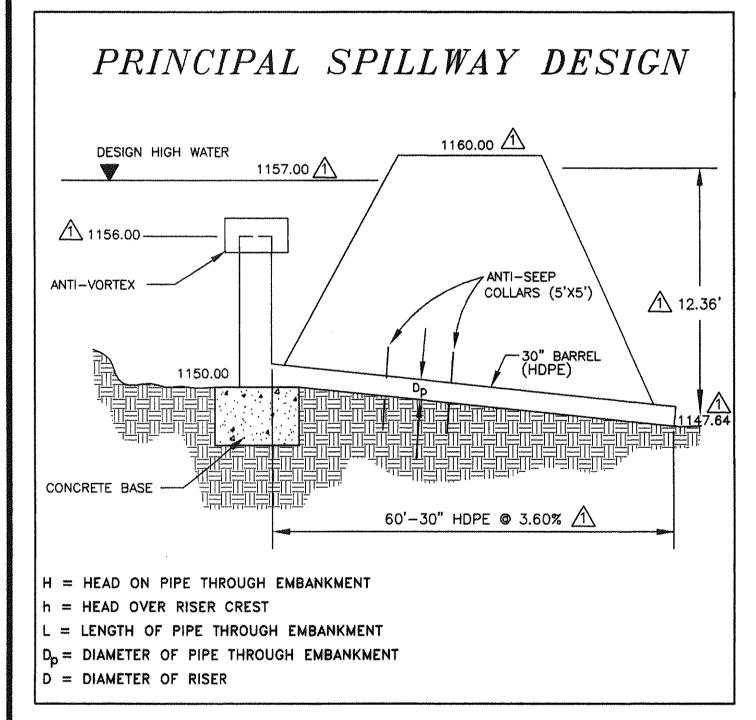


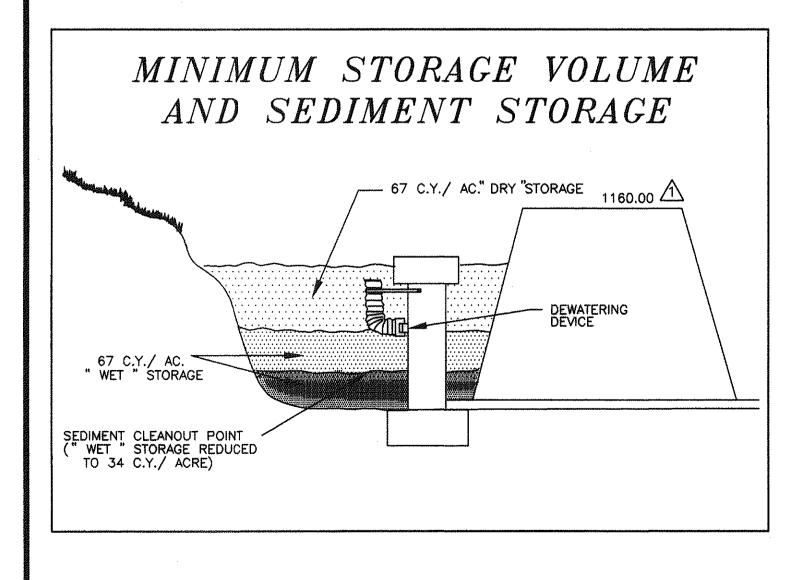




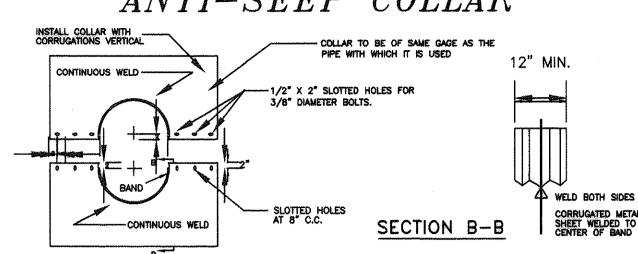








DETAILS OF CORRUGATED METAL ANTI-SEEP COLLAR

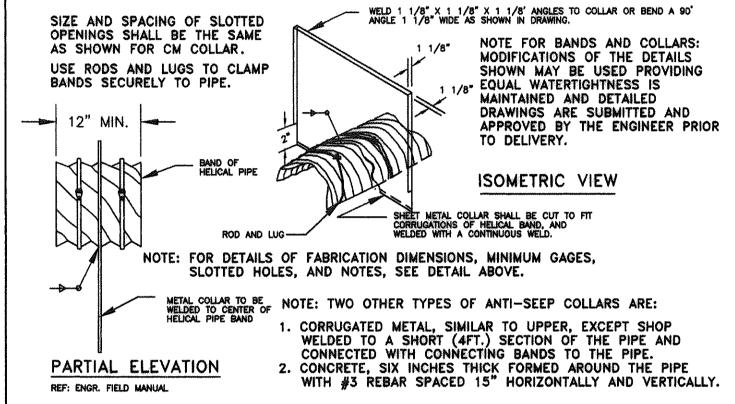


ELEVATION OF UNASSEMBLED COLLAR

NOTES FOR COLLARS:

3. UNASSEMBLED COLLARS SHALL BE MARKED BY PAINTING OR TAGGING TO IDENTIFY MATCHING 1. ALL MATERIALS TO BE IN ACCORDANCE WITH 4. THE LAP BETWEEN THE TWO HALF SECTIONS CONSTRUCTION AND CONSTRUCTION MATERIAL AND BETWEEN THE PIPE AND CONNECTING BAND SHALL BE CAULKED WITH ASPHALT MASTIC AT 2. WHEN SPECIFIED ON THE PLANS, COATING OF COLLARS SHALL BE IN ACCORDANCE WITH 5. EACH COLLAR SHALL BE FURNISHED WITH TWO 1/2" DIAMETER RODS WITH STANDARD TANK LUGS FOR CONNECTING COLLARS TO PIPE. CONSTRUCTION AND CONSTRUCTION MATERIAL

DETAIL OF HELICAL PIPE ANTI-SEEP COLLAR



SOURCE: USDA-SCS

PLATE. 3.14-13

3.14

NORTH AMERICAN GREEN CHANNEL PROTECTION MATERIAL SPECIFICATIONS

C350 Specification

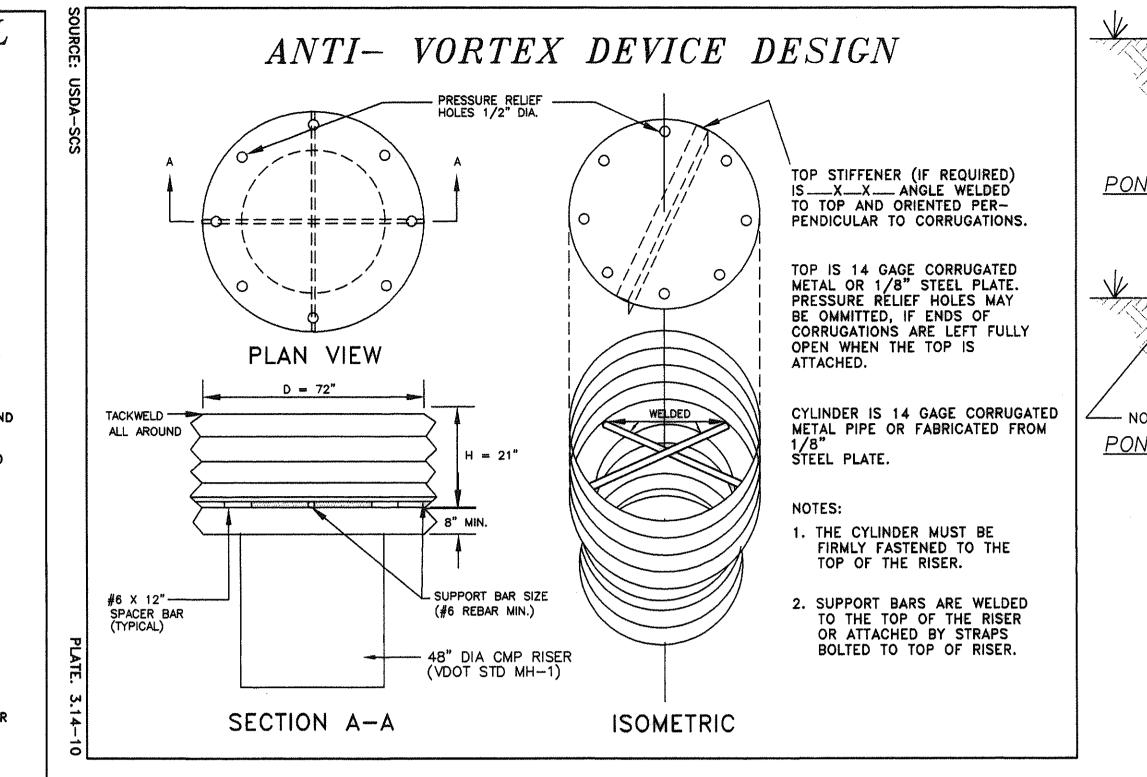
North American Green C350 permanent erosion control/turf reinforcement mat is constructed of 100% coconut fiber stitch bonded between a heavy duty UV stabilized bottom net, and a heavy duty UV stabilized cuspated (crimped) middle netting overlaid with a heavy duty UV stabilized top net. The cuspated netting forms prominent closely spaced ridges across the entire width of the mat. The three nettings are stitched together on 1.5 in (3.8 cm) centers with UV stabilized polypropylene thread to form a permanent three dimensional structure. The following list contains further physical properties of the C350 Erosion Control/Turf Reinforcement Mat.

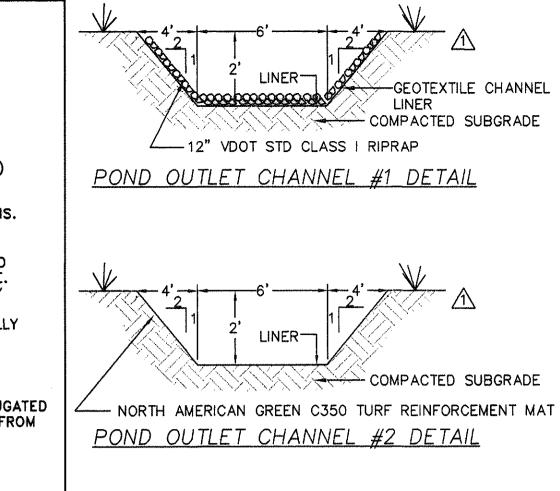
Property	Test Method	Value	Unit	
Ground Cover	Image Analysis	93	%	
Thickness	ASTM D1777	.63 (1.6)	in (cm)	
Mass Per Unit Area	ASTM D3776	.92 (.50)	lb/sy (kg/m^2)	
Tensile Strength	ASTM D5035	480 (714)	lb/ft (kg/m)	
Elongation	ASTM D5035	49	%	
Tensile Strength	ASTM D5035	960 (1429)	lb/ft (kg/m)	
Elongation	ASTM D5035	31	%	
Tensile Strength	ASTM D1682	177 (80)	lbs (kg)	
Elongation	ASTM D1682	22	%	
Resiliency	ASTM D1777	>80	%	
UV Stability*	ASTM D4355	151 (68) 86	lbs (kg)	
Color(permanent net)		UV Black	70	
Porosity(permanent net)	Calculated	>95	%	
Minimum Filament	Measured	.03 (.08)	in (cm)	
Diameter (permanent net)				

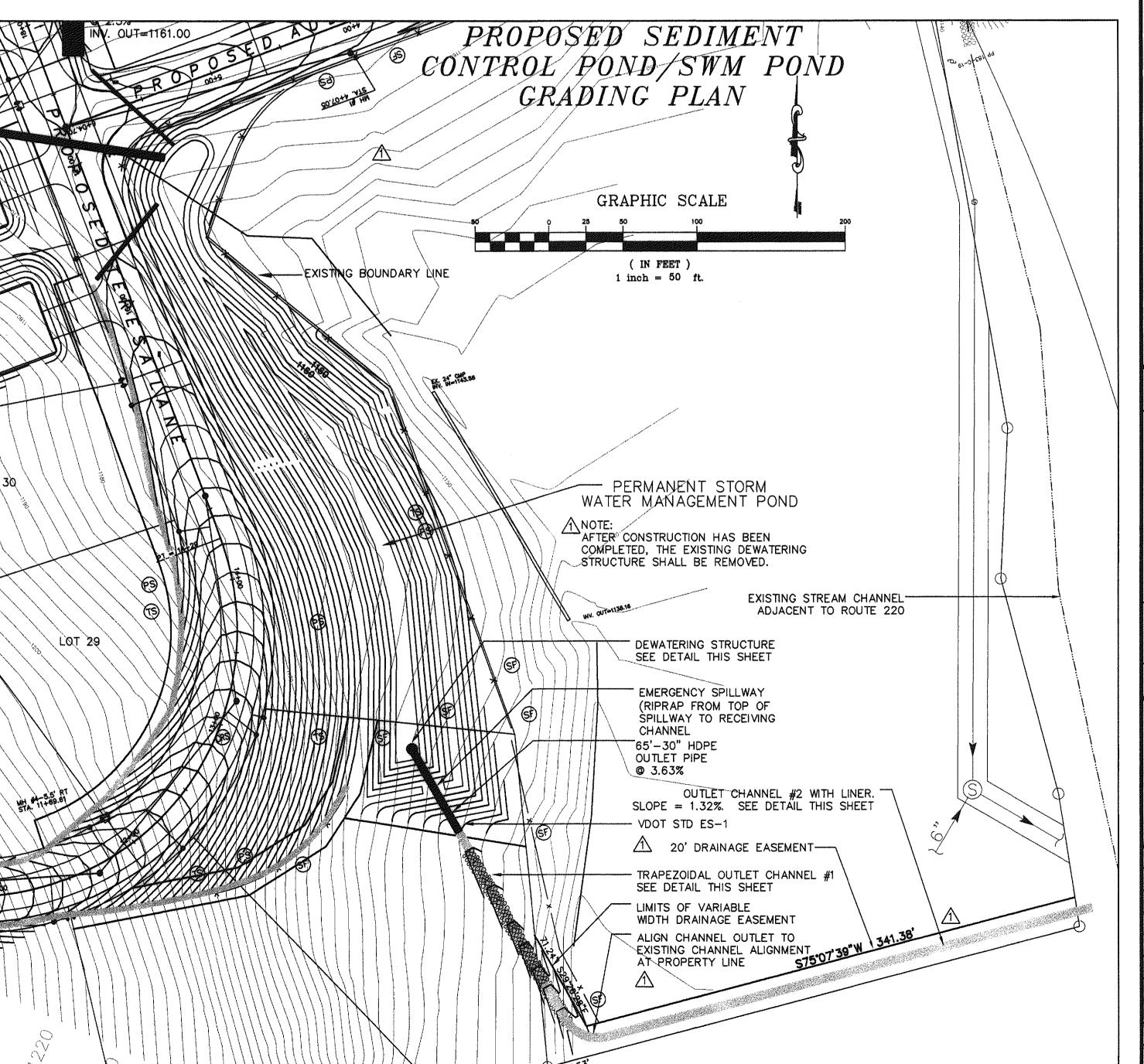
*ASTM D1682 Tensile Strength and % Strength Retention of material after 1000 hours of exposure in Xenon—Arc Weatherometer

Physical Specifications (Roll)

6.5 feet (2 m) 55.5 feet (16.9 m) 37 lbs + /- 10% (16.8 kg)40 yd^2 (33.4 m²)



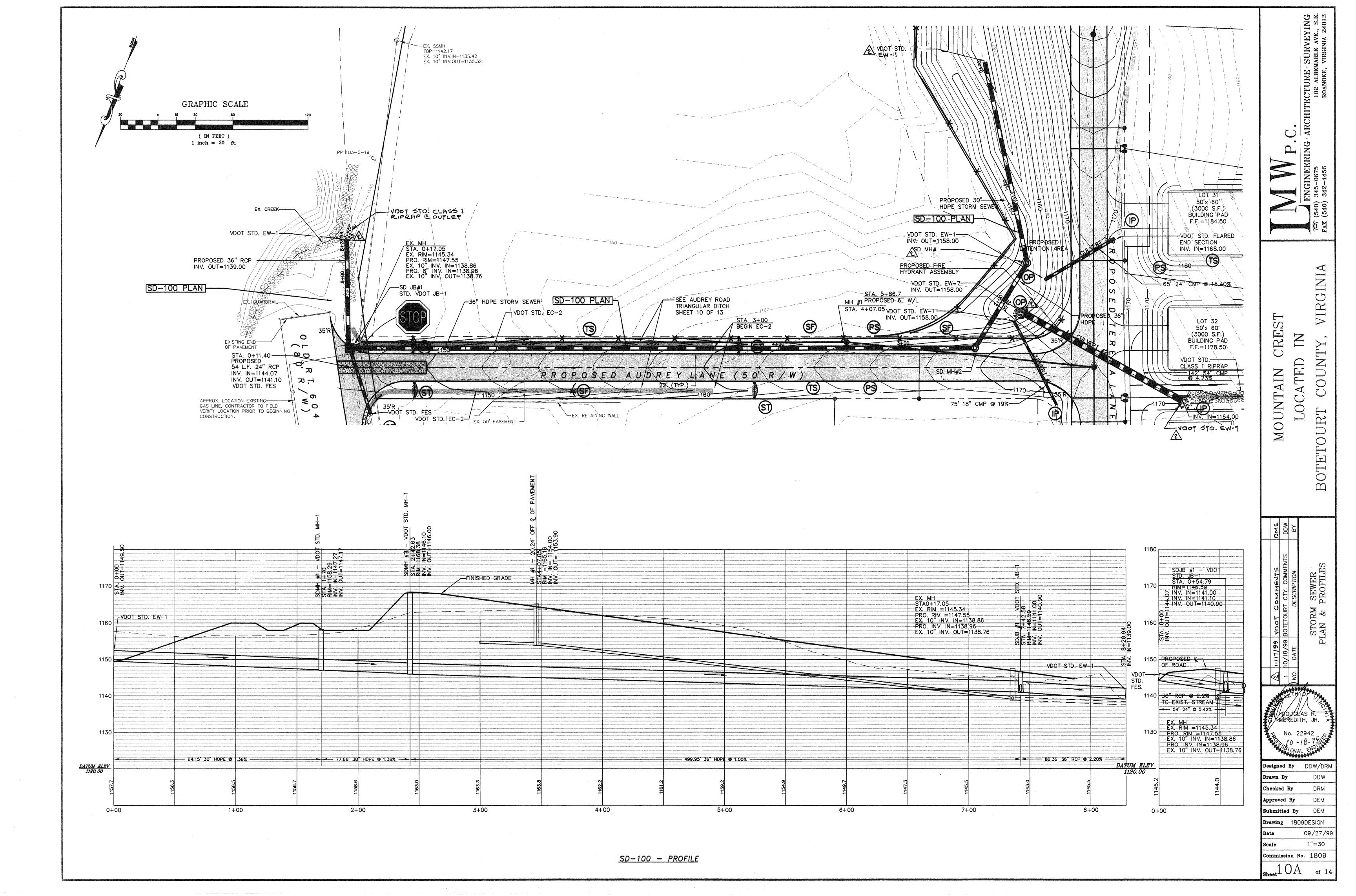


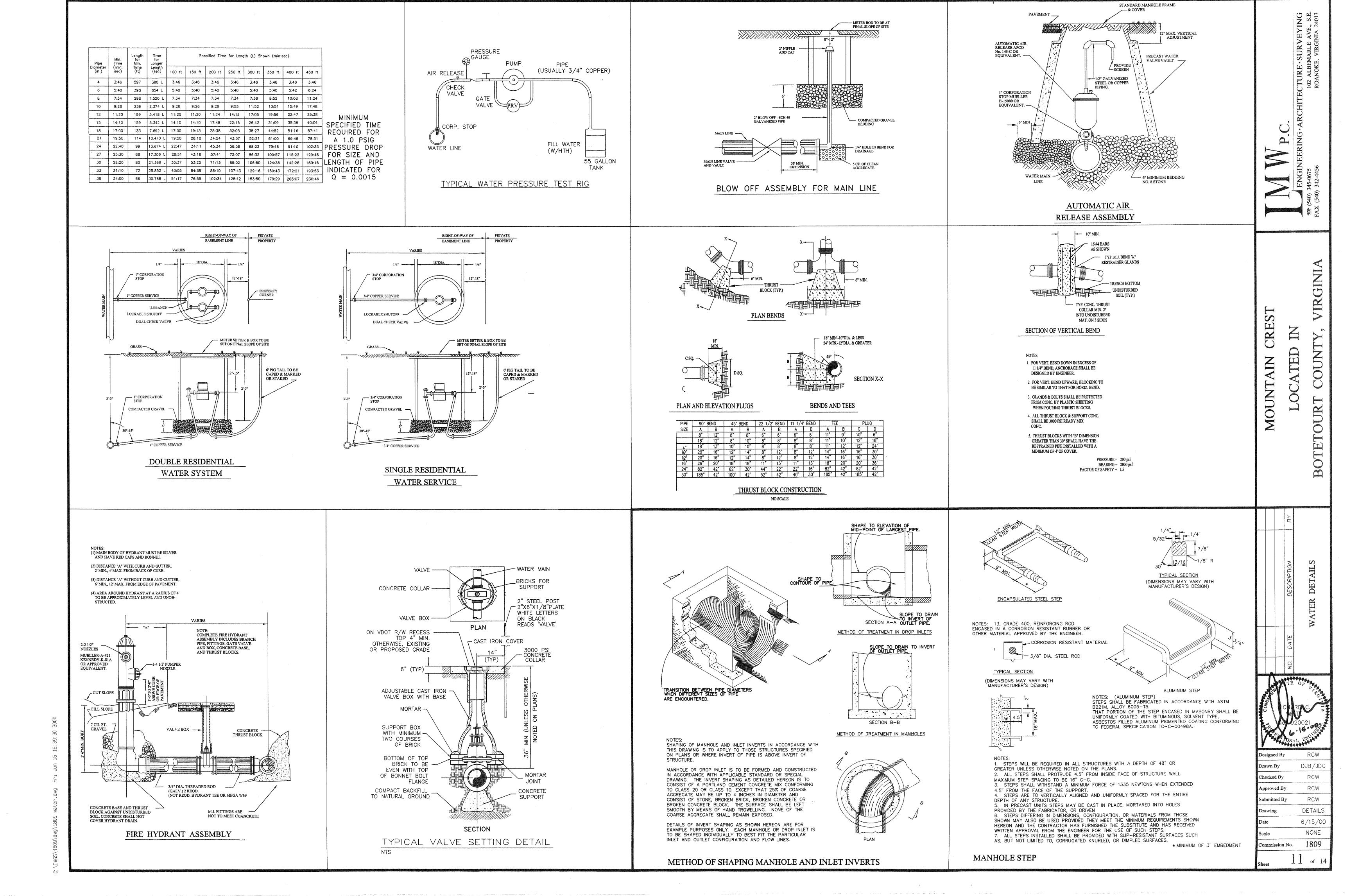


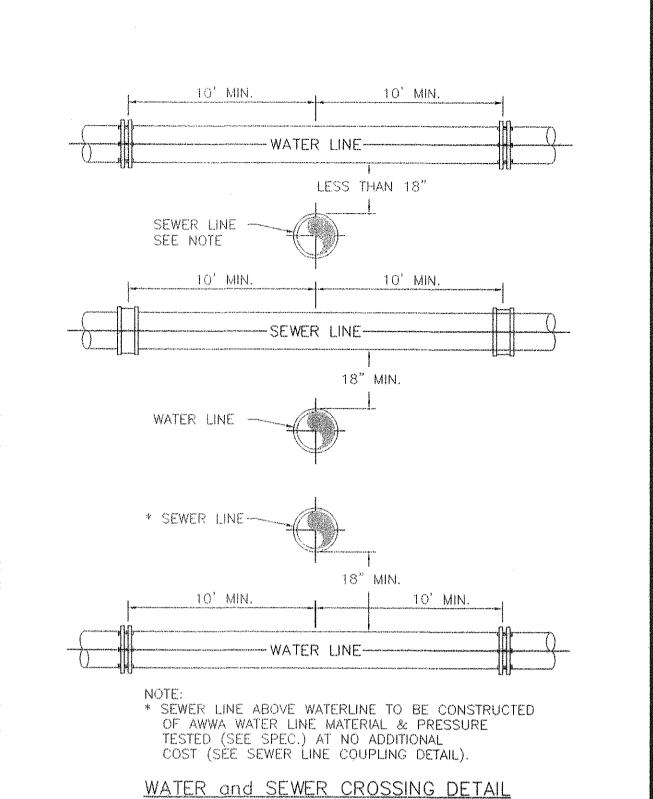
REST

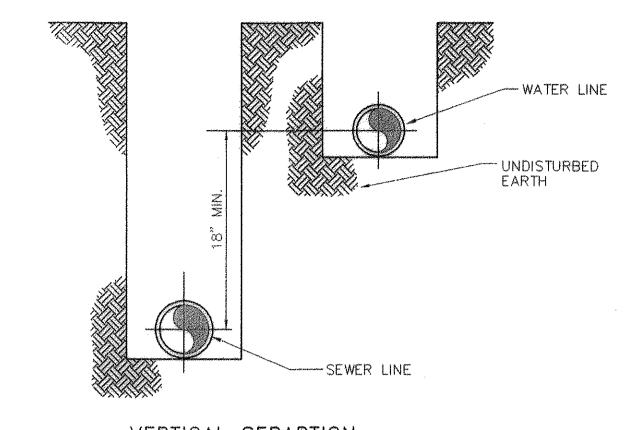
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DJB Checked By Approved By DRM Submitted By 1809 POND Drawing 7/25/00 AS NOTED Commission No. 1809

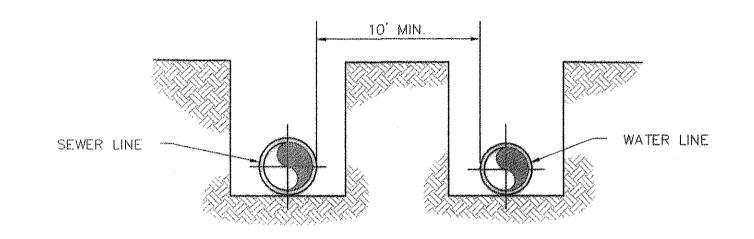






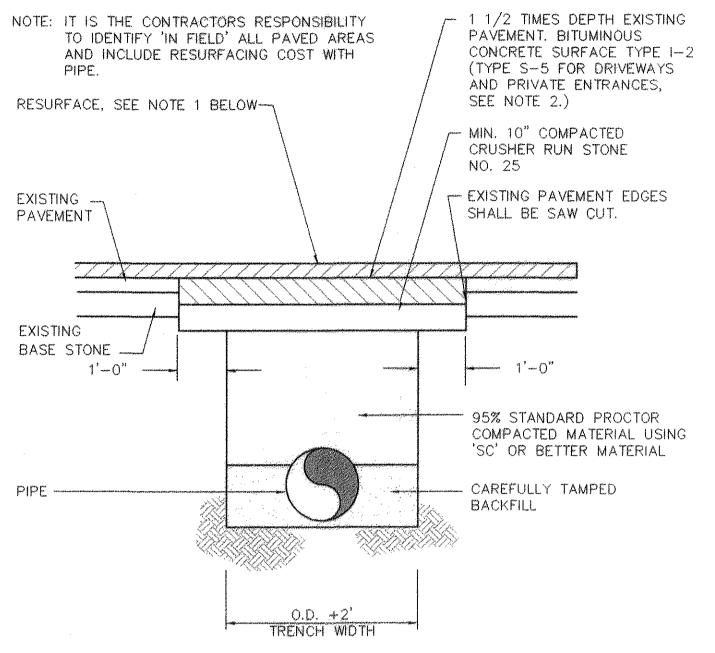


VERTICAL SEPARTION



WATER and SEWER SEPARATION DETAIL NTS

HORIZONTAL SEPARATION



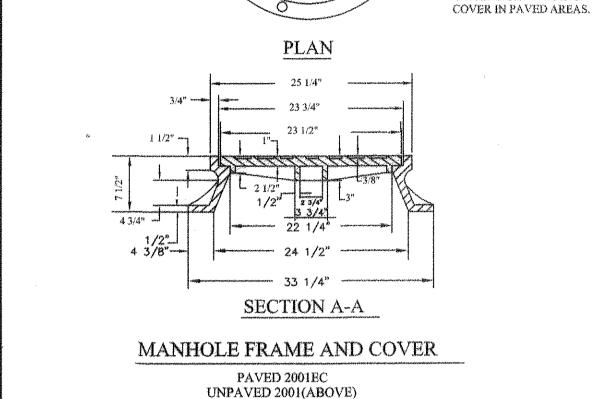
OPEN CUT ROAD and CROSSING DETAIL

NOTES: 1) ALLPAVED SURFACES OWNED BY THE STATE SHALL BE RESURFACED 25' EACH SIDE OF CUT, AS PER V.D.O.T. SPECIFICATIONS.

CRUSHER RUN STONE NO. 25.

2) THIS DETAIL ALSO APPLIES TO ALL PAVED DRIVEWAYS AND ENTRANCES-EXCLUDING 25' RESURFACING EACH WAY.

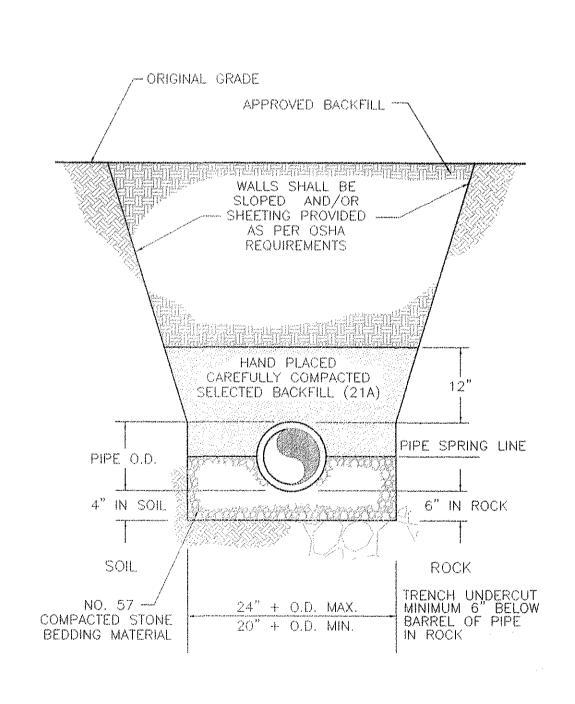
3) WHERE EXISTING SURFACE IS GRAVEL, REPLACEMENT SURFACE TO BE



· 3 - 1" DIA. HOLES @ 120

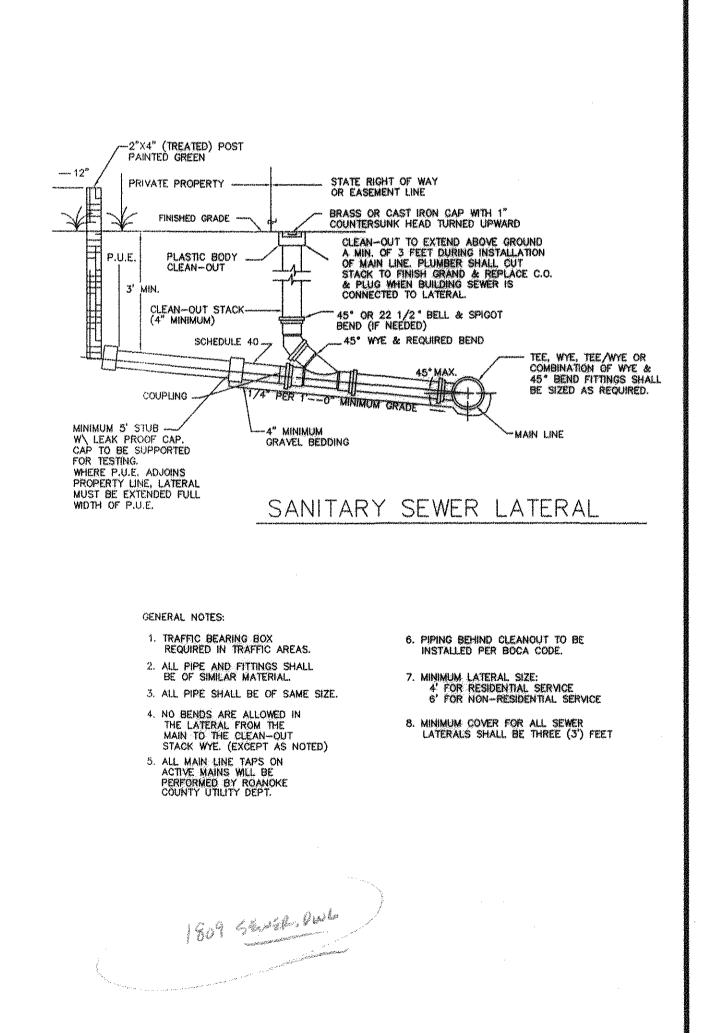
USE DEWEY BROTHERS INC. MH-RCR-2000 OR 2001 WATERTIGHT FRAME &

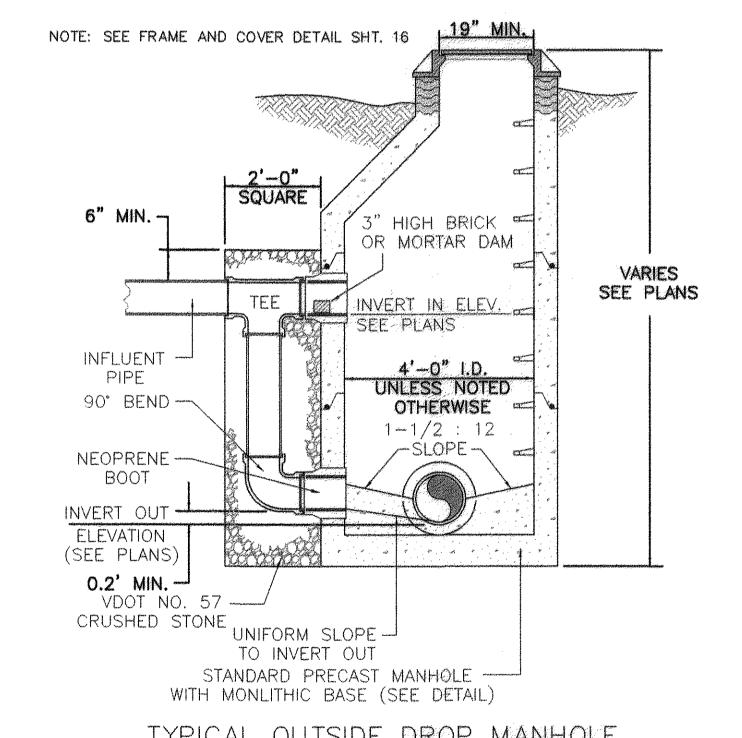
S



PIPE LAYING CONDITION for GRAVITY SEWER

NTS



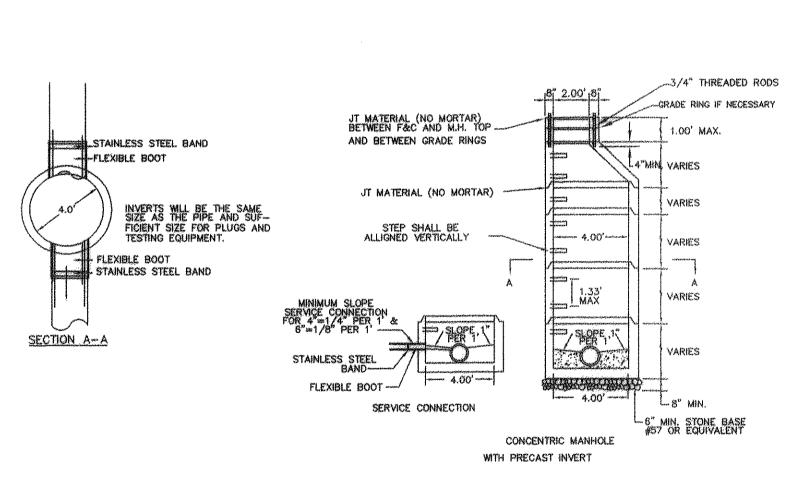


TYPICAL OUTSIDE DROP MANHOLE

1. MANHOLE UNIT JOINTS SHALL BE MADE WITH EITHER FLEXIBLE BUTYL SEALANTS OR GASKETS. AT THE CONTRACTOR'S OPTION. FLEXIBLE BUTYL SEALANTS SHALL BE MANUFACTURED BY CONCRETE SEALANTS, INC. (CS-302) OR EQUAL AND FLEXIBLE BUTYL GASKETS SHALL BE MANUFACTURED BY CONCRETE PRODUCTS SUPPLY COMPANY (E-Z STICK) OR EQUAL. THE GASKETS OR SEALANTS SHALL BE INSTALLED AND THE JOINT MADE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER(S).

2. ALL MANHOLES DEEPER THAN 10' SHALL BE PROVIDED WITH A SAFETY SLAB.

3. TEE, BEND, AND ALL PIPING SHALL BE SAME SIZE AS INFLUENT PIPE (SEE PLANS)

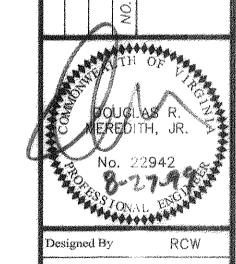


4' STANDARD MANHOLE FOR PIPE 15" OR SMALLER

1. ALL MANHOLE FRAMES AND COVERS SHALL BE DEWEY BROTHERS, INC. MH-RCR-3000W IN NON-PAVED AREAS REQUIRING WATERTIGHT FRAME & COVERS AND MH-RCR-3000EC-WT IN PAVED AREAS, OR APPROVED EQUAL. 2. STEPS TO BE VERTICALLY ALIGNED.

3. THE FRAME AND COVER SHALL BE PROPERLY ALIGNED WITH THE 2 FOOT OPENING OF THE MANHOLE STRUCTURE AND BOLTED IN PLACE.

4. MANHOLE UNIT JOINTS SHALL BE MADE WITH EITHER FLEXIBLE BUTYL SEALANTS OR GASKETS, AT THE CONTRACTOR'S OPTION. FLEXIBLE BUTYL SEALANTS SHALL BE MANUFACTURED BY CONCRETE SEALANTS, INC.
(CS-302) OR EQUAL AND FLEXIBLE BUTYL GASKETS SHALL
BE MANUFACTURED BY CONCRETE PRODUCTS SUPPLY COMPANY (E-Z STICK) OR EQUAL. THE GASKETS OR SEALANTS SHALL BE INSTALLED AND THE JOINT MADE IN ACCORDANCE WITH THE 5. ALL MANHOLES DEEPER THAN 10' SHALL BE PROVIDED WITH A SAFETY SLAB.



DUB/UDC Drawn By RCW Checked By Approved By DRM Submitted By DETAILS 05/26/99 NONE 1809 Commission No

The purpose of this project is to construct a single family subdivision. This project will include 50 lots, with a minimum lot size of 20,000 SF. There will be approximately 6000 LF of new roadway having a typical pavement width of width of 22 feet. The total acreage of this project is approximately 50 acres.

EXISTING SITE CONDITIONS

The proposed development is located along Route 849, Arrington Rd., in the Blue Ridge District of Botetourt County. The existing site is wooded at this time and is not being used at this time.

ADJACENT AREAS

The site is bordered on the east by Route 849, the north and west by Steeplechase subdivision and to the south by private property.

SOILS

Soils found at this site are common to the area. None of these soils have high erosion tendencies.

CRITICAL EROSION AREAS

2. The outlet of all culverts.

The potential critical erosion areas are:

1. Steep roadside ditch slopes along proposed roads.

EROSION AND SEDIMENT CONTROL MEASURES

Unless otherwise indicated, all vegetative and structural erosion and sediment control practices shall be constructed and maintained according to minimum standards and specifications of the handbook. The minimum standards of the VESCR shall be adhered to unless otherwise waived or approved by a variance.

STRUCTURAL PRACTICES

- Temporary Construction Entrance (Section 3.02)
 One temporary construction entrance will be installed. Should tracking occur the road will be immediately cleaned.
- 2. Silt Fence (Section 3.05)
 Temporary silt fences will be installed as indicated on the site plan.
- 3. Straw Bale Barrier (Section 3.04)
 Temporary straw bale barriers will be placed and entrenched and anchored as indicated on the site plan.
- 4. Outlet Protection (Section 3.18)
 Outlet protection will be placed at all discharge points from controlled flow to open flow. All outlet protection will be permanently designed and installed
- 5. Surface Roughening (Section 3.29)
 Surface roughening will be employed on all slopes exceeding 2:1.
- 6. Temporary Seeding (Section 3.31)
 Temporary seeding will be placed on all disturbed areas that will not be brought to final grade within 30 days. Temporary seeding will aid in the reduction of dust and sediment.
 Temporary seeding will be Annual Ryegrass (100 #/ac), Feb 16 April 30, German Millet (60 #/ac), May 1 Aug. 31.
- Permanent Seeding (Section 3.32)
 Within 7 days after final grading permanent, seeding will be employed to reduce erosion and sediment yield.

Seeding Specifications:

Permanent seeding will be Kentucky Bluegrass, blended to contain 4 or more varieties, with no one variety exceeding 30%. The seeding will be applied at 140 lb. per acre. On slopes 2:1 or greater a mixture of Crown Vetch (50%), Perennial Ryegrass (40%), and Redtop (10%) will be used.

All seeding, with required associated practices, will be in accordance with all applicable sections of the Virginia Erosion and Sediment Control

- 8. Dust Control (Section 3.39)
 If arid conditions prevail dust control practices will be employed as required.
- Construction Road Stabilization (Section 3.03)
 All roads and parking areas on the site shall be stabilized with gravel immediately after grading.
- 10. Temporary Sediment Basin (Section 3.14)
- 11. Riprap (Section 3.19)
 Riprap shall be placed at the outlet of all pipes in accordance with VDOT standard EC-2 as indicated on the plans. Riprap along the ditches shall be VDOT Class 1 riprap installed over a six inch filter consisting of #57 stone.

12. Check Dams (Section 3.20)

the first step of construction.

MANAGEMENT

- 1. Construction should be sequenced so that grading operations can begin and end as quickly as possible.
- 2. Erosion and Sediment control devices shall be installed as
- 3. Areas which are not to be disturbed shall be clearly marked by flags, signs, etc.
- 4. The grading contractor shall be responsible for the installation and maintenance of all erosion and sediment control practices. Inspections are to be made periodically and after every significant rainfall.
- 5. After achieving adequate stabilization, the temporary E&S controls will be cleaned up and removed, and the sediment basins will be cleaned out and converted to permanent stormwater management basins. Removal must be approved by Botetourt County.

PERMANENT STABILIZATION

All areas disturbed by construction shall be stabilized with permanent seeding within 7 days following finish grading. Seeding shall be done with Kentucky 31 Tall Fescue according to Std. & Spec. 3.32, PERMANENT SEEDING, of the handbook. Erosion control blankets will be installed over fill slopes which have been brought to final grade and have been seeded to protect the slopes from rill and gully erosion and to allow seed to germinate properly. Mulch (straw or fiber) will be used on relatively flat areas. In all seeding operations, seed, fertilizer and lime will be applied prior to mulching.

MAINTENANCE OF DETENTION FACILITIES

The applicant shall obtain approval from the locality of a plan for maintenance of the detention facilities. The plan shall set forth the maintenance requirements of the facility and the person responsible for performing the maintenance.

STORMWATER MANAGEMENT

Calculation of runoff before and after development indicates that there will be a net increase in peak runoff as a result of project development. Consequently, stormwater management basins have been designed to detain and release the runoff at the 2-year pre-developed rate. (See attached calculations)

MAINTENANCE

In general, all erosion and sediment control measures will be checked daily and after each significant rainfall. Any items not found in accordance with the Virginia Erosion and Sediment Control Handbook will be immediately replaced and/or repaired. The following items will be checked in particular:

- 1. The sediment basin will be cleaned out when the level of sediment buildup reaches the cleanout point indicated on the
- 2. The gravel outlets will be checked regularly for sediment buildup which will prevent drainage. If the gravel is clogged by sediment, it shall be removed and cleaned or replaced.
- 3. The silt fence barrier will be checked regularly for undermining or deterioration of the fabric. Sediment shall be removed when the level of sediment deposition reaches half way to the top of the barrier.
- 4. The seeded areas will be checked regularly to ensure that a good stand is maintained. Areas should be fertilized and reseeded as needed.

GENERAL

The erosion and sediment control measures shown on the construction plans are the minimum measures required. The owner, through his contractor, will employ whatever measures which may be required to assure that sediment laden runoff does not leave the site.

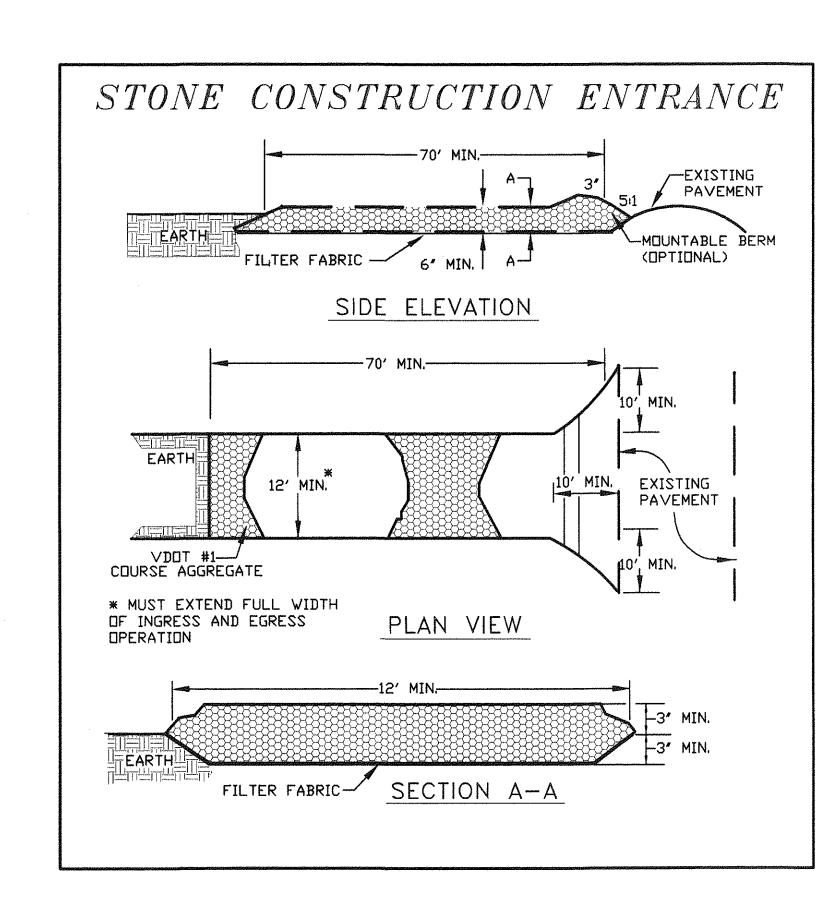
All materials and measures employed for erosion and sediment control will be in accordance with the Virginia Erosion and Sediment Control Handbook, latest edition.

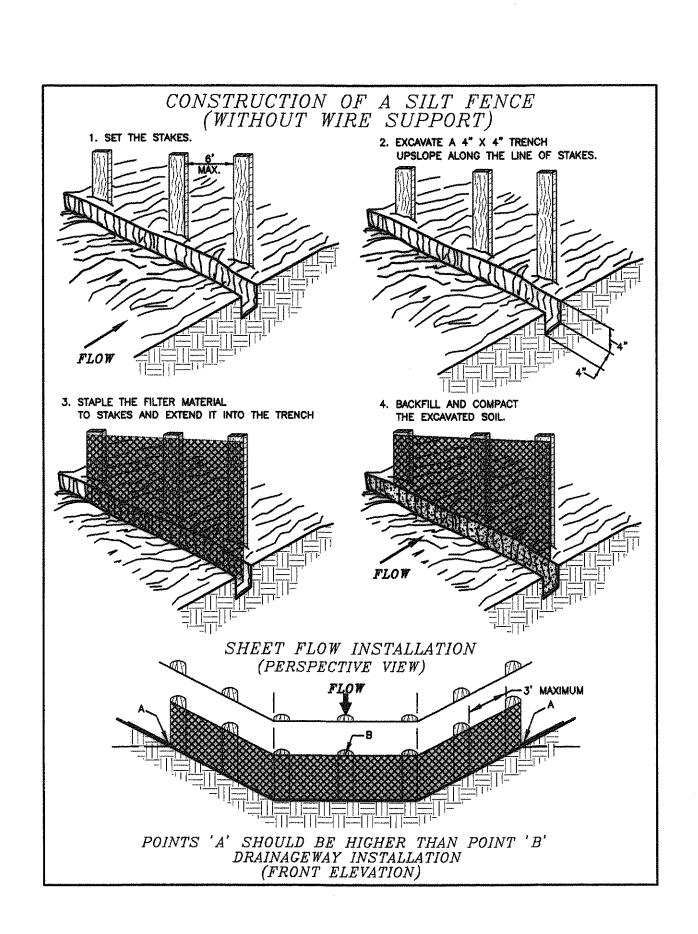
If, during construction, additional Erosion and Sediment Control measures are deemed necessary, they shall be installed as directed by the Owner, Engineer or County agent.

This project is to be constructed consistent with the 1992 Virginia Erosion And Sediment Control Regulations.

Construction traffic shall not enter streams or swales without first installing a temporary stream crossing.

CONSTRUCTION OF A SILT FENCE (WITH WIRE SUPPORT) 1. SET POSTS AND EXCAVATE A 474" TIFENOH UPSLOPE ALONG THE LINE OF POSTS. 2. STAPLE WIRE FENCING TO THE POSTS. FLOW FRANCE AND WIRE INTO THE TRENCH.







MOUNTAIN CREST
LOCATED IN
COUNTAIN
COUN

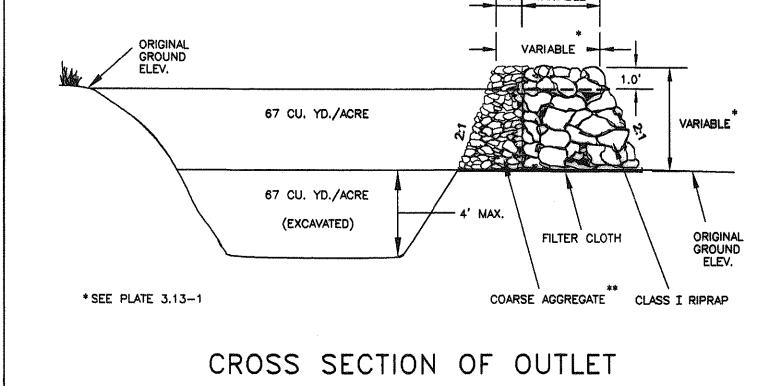
8/26/99 VDOT & BOTETOURT CTY. COMMENTS DDW
0. DATE DESCRIPTION BY
EROSION AND SEDIMENT

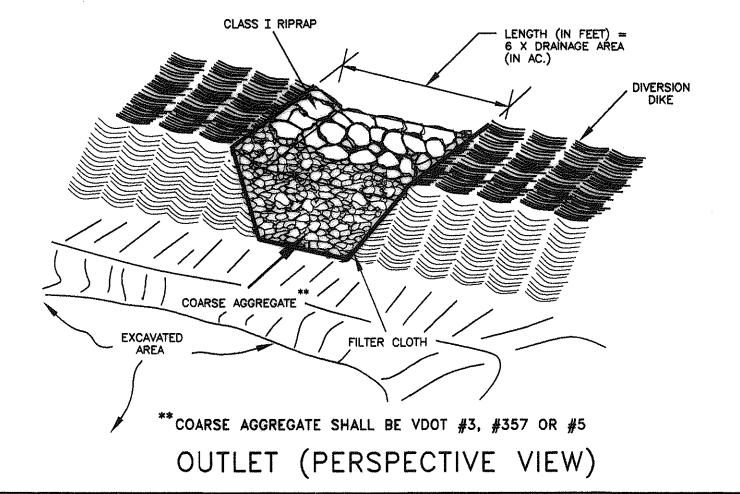
Designed By RCW
Drawn By DJB/JDC
Checked By RCW

Checked By RCW
Approved By RCW
Submitted By RCW
Drawing 1809EROS.DWG
Date 05/26/99
Scale NONE

Commission No. 1809

Sheet 13 of 1

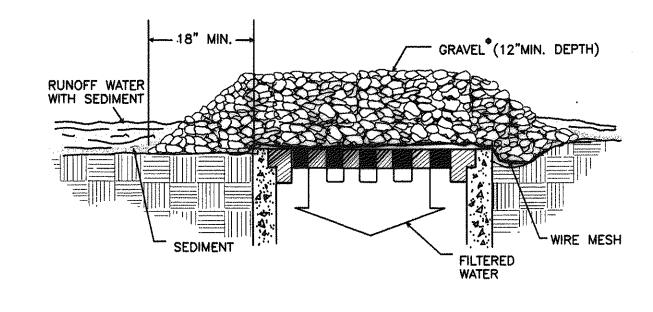




SOURCE: VA. DSWC

PLATE. 3.13-2

GRAVEL AND WIRE MESH DROP INLET SEDIMENT FILTER



SPECIFIC APPLICATION

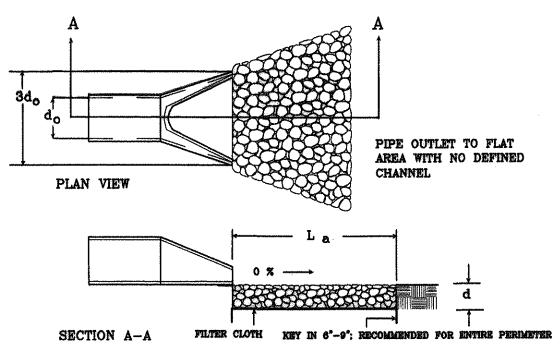
THIS METHOD OF INLET PROTECTION IS APPLICABLE WHERE HEAVY CONCENTRATED FLOWS ARE EXPECTED, BUT NOT WHERE PONDING AROUND THE STRUCTURE MIGHT CAUSE EXCESSIVE INCONVENIENCE OR DAMAGE TO ADJACENT STRUCTURES AND UNPROTECTED AREAS.

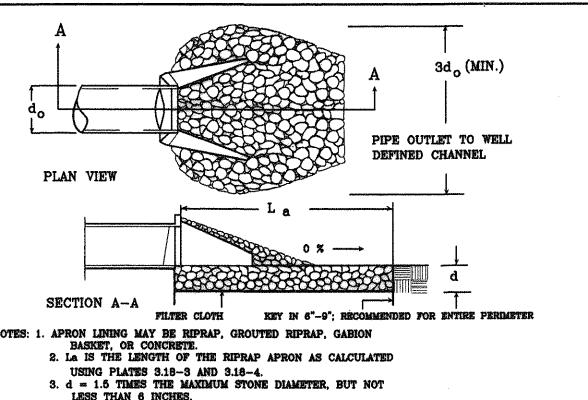
* GRAVEL SHALL BE VDOT #3, #357 OR #5 COARSE AGGREGATE.

SOURCE: VA. DSWC

PLATE. 3.07-2

PIPE OUTLET CONDITIONS





Source: Va. DSWC

Plate 3.18-1

VRGINIA CREST COUNTY MOUNTAIN

SION & SEDIMENT ADDITIONAL DET

Designed By DDW/DME Checked By

09/27/99

Commission No. 1809