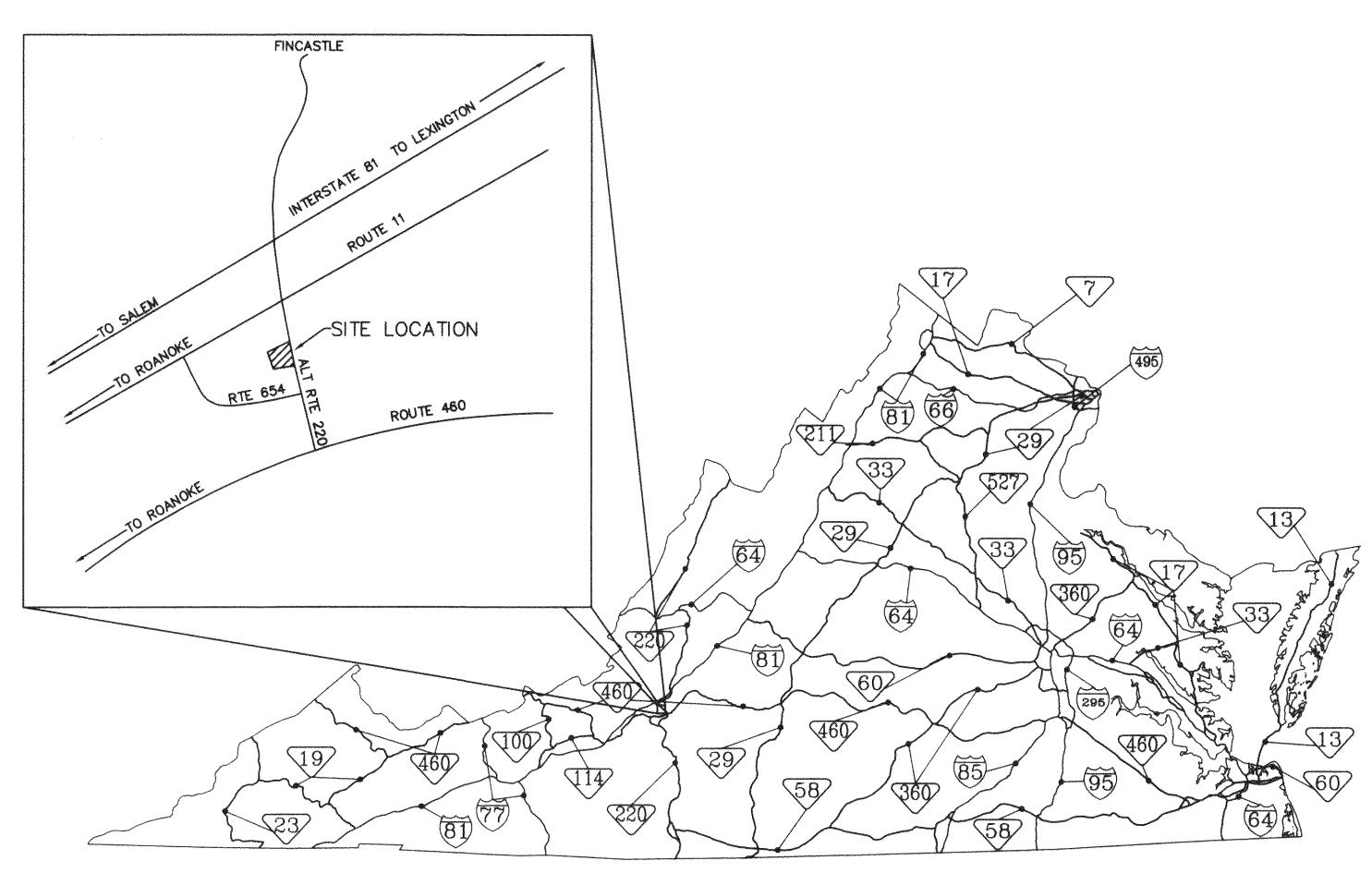
SUMMERFIELD VILLAGE SITE PLAN

BOTETOURT COUNTY, VIRGINIA

DALE WILKINSON/BO TRUMBO/DAVID SPIGLE
960 PARK DRIVE
ROANOKE, VIRGINIA 24012

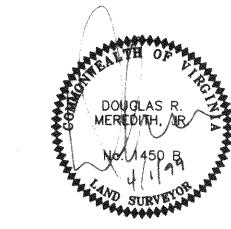
VICINITY MAP NO SCALE

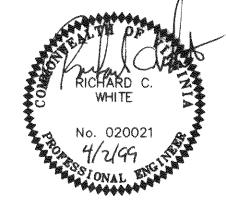


LOCATION MAP NO SCALE

INDEX OF SHEETS NO SCALE

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- . UTILITY PLAN
- '. OFF-SITE SANITARY SEWER PLAN & PROFILE
- 8. STREET & STORM SEWER PROFILES
- SANITARY SEWER PROFILES
- 10. SEDIMENT POND/DETENTION POND OUTLET STRUCTURE DETAILS
- 11. UTILITY SPECIFICATIONS
- 12. STANDARD UTILITY DETAILS
- 12 A. 84" RCP EXTENSION





P.C.

ENGINEERING-ARCHITECTURE SURVEYING

(540) 345-0675

FAX (540) 342-4456

ROANOKE, VIRGINIA 24013

SUMMERFIELD
VILLAGE
SITE PLAN

COMM. NO. 1702

DATE: 4/1/99

SET NO.

EP

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FIG

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DME RCW

Designed By Drawn By Checked By Approved By

Submitted By RCW Drawing 1702ABBR.DWG 11/2/98

NONE Commission No. 1702

EXISTING ABAN NEW DESCRIPTION ABANDON, ABANDONED MECHANICAL ABUT ABUTMENT MFR MANUFACTURER ADJ ADJACENT MH MANHOLE AGGR AGGREGATE MIN MINIMUM BUILDING WITH PORCH OR STOOP ANC ANCHOR MJ MECHANICAL JOINT APPROX APPROXIMATE MON MONUMENT ____ BIT FOUNDATION ONLY BITUMINOUS MTL BJ BELL JOINT N & C NAIL AND CAP 35 CONTOUR, CONTOUR WITH ELEVATION BASE LINE NIC NOT IN CONTRACT BEG BEGIN, BEGINNING NO NUMBER 20.0 E OR X 1025 SPOT ELEVATION BLDG BUILDING NPW NON POTABLE WATER BM BENCH MARK NTS NOT TO SCALE CONCRETE CURB BSP BLACK STEEL PIPE ON CENTERS ΒV BUTTERFLY VALVE OUTSIDE DIAMETER CONCRETE CURB & GUTTER BEGIN VERTICAL CURVE ELEVATION BVCE PVMT PAVEMENT BVCS BEGIN VERTICAL CURVE STATION PC POINT OF CURVE 4 4 CONCRETE WALK OR SLAB CURB AND GUTTER C & G PCC POINT OF COMPOUND CURVE CAST IRON man amounted I mountain betalerin magas magasas as consiste the language committee as see the language of the la PER PERIMETER PAVEMENT CENTER LINE PERF manklikarisiainimmäkkkominimimmimm PERFORATED CONST CONSTRUCTION PERP processors constant appropriate constants constants whether otherwises PERPENDICULAR entimenta non approximation and the entire e UNPAVED OR GRAVEL ROAD CMP CORRUGATED METAL PIPE WARRING STREET, STREET POINT OF INTERSECTION commence and commence and commence and commence CONCRETE MASONRY UNITS CMU PLATE, PROPERTY LINE CONSTRUCTION EASEMENT CND CONDUIT POL POINT ON LINE CO CLEANOUT POINT OF TANGENCY And Control of Control PERMANENT EASEMENT COMB COMBINATION POT POINT ON TANGENT CONC CONCRETE (PORTLAND CEMENT) POWER POLE (YYYY) TREE LINE CONN CONNECT, CONNECTION PRC POINT OF REVERSE CURVE CONTR CONTRACTOR 9 × 👸 PSI POUNDS PER SQUARE INCH OR A TREE OR SHRUB CONVEYOR CONV POINT OF TANGENT COR CORNER PVC POLYVINYL CHLORIDE more and the second sec FENCE (EXISTING OR PROPOSED NOTED) CR STONE CRUSHED STONE PVI POINT OF VERTICAL INTERSECTION CTR CENTER PUE Minimum of the American Company of the American Company of the American Company of the Company o PUBLIC UTILITY EASEMENT CENTERLINE CREEK, SWALE, DITCH CULV CULVERT RADIUS. RISER DEPTH OR DEGREE OF CURVE RAILROAD PROPERTY LINE DRAINAGE EASEMENT RCP REINFORCED CONCRETE PIPE DROP INLET, DUCTILE IRON RD ROAD --- \(\) ----- \(\) -----CENTERLINE OR BASELINE DIA DIAMETER RDCR REDUCER DIM DIMENSION REINF REINFORCE, REINFORCEMENT DISC DISCONNECT REF REFERENCE FIELD SURVEY TRAVERSE POINT DMH DROP MANHOLE REL RELOCATED DN DOWN REQD REQUIRED P.C. OR P.T. DTL DETAIL REV REVISION DW, D/W DRIVEWAY RTE ROUTE GEOLOGIC BORE HOLE DWL DWELLING RIGHT DWG DRAWING RIGHT OF WAY R/W BENCH MARK (EXISTING OR SET NOTED) EACH SANITARY SEWER E.B.L. EASTBOUND LANE SAN SANITARY ____ SD ____ STORM DRAIN AND ENDWALL EL. ELEV ELEVATION S/W SIDEWALK ELEC ELECTRICAL STORM DRAIN SANITARY SEWER ENGR ENGINEER SLOPE EASEMENT ENTR ENTRANCE SECT SECTION manuscriptor agreement FM approximation FORCE MAIN EOL END OF LINE SER SERVICE EDGE OF PAVEMENT SHEET GAS MAIN OR SERVICE LINE EQ EQUAL SPEC SPECIFICATION EQPT EQUIPMENT SPECS SPECIFICATIONS WATER MAIN OR SERVICE LINE END VERTICAL CURVE ELEVATION EVCE SQUARE EVCS END VERTICAL CURVE STATION SSTL STAINLESS STEEL OVERHEAD ELECTRICAL LINE EW EACH WAY, ENDWALL STREET EXIST EXISTING STAOT STATION OVERHEAD TELEPHONE LINE FES FLARED END SECTION STANDARD FINISH FLOOR --- UE --- ---STEEL UNDERGROUND ELECTRICAL LINE FINISHED FLOOR ELEVATION STRUCT STRUCTURAL FIGURE SUR --- UT----SURVEY UNDERGROUND TELEPHONE LINE FLOOR T & B TOP AND BOTTOM FLEX FLEXIBLE PIPE FITTINGS TELE TELEPHONE FLANGE FLG TEMP TEMPORARY FOOT THK FIRE HYDRANT THICK FTG FOOTING TP TELEPHONE POLE FUT FUTURE TRTD GATE VALVE GAL GALLON TV TELE VISION GALV GALVANIZED TW CLEANOUT TOP OF WALL GAR GARAGE TYP TYPICAL GND GROUND UG MANHOLE UNDERGROUND GRAVEL UNLESS OTHERWISE NOTED UON GOVT GOVERNMENT DROP INLET (CURB AND GRATING TYPES) U.S.C.&G.S UNITED STATES COAST AND GPM GALLONS PER MINUTE GEODETIC SURVEY GRTG $Q_{\mathbf{Q}}$ GRATING V. VAL VALVE, VENT WM - WATER METER GATE VALVE VARIABLE VAR DWM - DOUBLE WATER METER H&T HUB AND TAC VERTICAL CURVE HORIZ HORIZONTAL VERT VERTICAL HIGH POINT VESCR VIRGINIA EROSION AND SEDIMENT TELEPHONE POLE, GUY AND ANCHOR HYD HYDRANT CONTROL REGULATIONS INSIDE DIAMETER POWER POLE, GUY AND ANCHOR VOL VOLUME INCH VIRGINIA DEPARTMENT OF TRANSPORTATION VDOT INSUL INSULATION \(\frac{1}{2}\) LIGHT POLE)—**1** V.S.D. VERTICAL SIGHT DISTANCE INVERT W.B.L. WESTBOUND LANE IRON PIN (FOUND OR SET NOTED) T TELEPHONE PEDESTAL WDE FLANGE, WDE, WASTE, WATER LENGTH, LONG T LINEAL FOOT ① WATER LINE BURIED TELEPHONE VAULT WITHOUT LIGHT POLE WATER SURFACE PAVED DITCH LONG RADIUS WATERTIGHT, WEIGHT WEST VIRGINIA DEPARTMENT Millionary, American millionaria, Managama, American American millionaria, American Mill STORM PIPE (SIZE / TYPE NOTED) MASONRY OF HIGHWAYS MATL MATERIAL MAX MAXIMUM MAIL BOX CULVERT WITH FLARED END SECTION MINIMUM BUILDING LINE AIR RELEASE VALVE / VAULT ASSEMBLY PROFILE PLAN PROFILE BLOW OFF VALVE / VAULT ASSEMBLY PROFILE PLAN PROFILE commence of the second control of the second STEEL ENCASEMENT

AND THE PROPERTY OF THE PROPER

CONCRETE ENCASEMENT

LIMITS OF CONSTRUCTION

ABANDON OR REMOVE

THE LOCATION OF EXISTING UTILITIES, INCLUDING UNDERGROUND UTILITIES, IS INDICATED ON THE DRAWINGS IN SO FAR AS THEIR EXISTENCE AND LOCATION WERE KNOWN AT THE TIME OF PREPARATION OF THESE DRAWINGS, HOWEVER, NOTHING IN THESE CONTRACT DOCUMENTS SHALL BE CONSTRUED AS A GUARANTEE THAT SUCH UTILITIES ARE IN THE LOCATION INDICATED OR THAT THEY ACTUALLY EXIST OR THAT OTHER UTILITIES ARE NOT WITHIN THE AREA OF OPERATIONS. THE CONTRACTOR SHALL MAKE ALL NECESSARY INVESTIGATIONS TO DETERMINE THE EXISTENCE AND LOCATIONS OF SUCH UTILITIES. THE CONTRACTOR SHALL PAY FOR ANY DAMAGE TO AND FOR MAINTENANCE AND PROTECTION OF EXISTING UTILITIES AND STRUCTURES.

> EXISTING WATER LINE LOCATIONS BOTH HORIZONTAL AND VERTICAL ARE APPROXIMATE. THE LOCATION IS NOT THE RESULT OF A FIELD SURVEY.

THE CONTRACTOR IS DIRECTED TO DIG AND LOCATE ALL UTILITIES IN ADVANCE OF PIPELAYING TO ALLOW FOR ADJUSTMENTS DUE TO CONFLICTS WITH EXISTING UTILITIES. SHOULD A CONFLICT ARISE THE ENGINEER IS TO BE NOTIFIED IMMEDIATELY.

THE CONTRACTOR IS REQUIRED TO NOTIFY "MISS UTILITY" AT 1-800-552-7001 AT LEAST TWO, BUT NOT MORE THAN TEN. WORKING DAYS IN ADVANCE OF CONSTRUCTION.

1. QUALITY CONTROL

STREETS TO BE GRADED. PAVED AND ALL STRUCTURAL COMPONENTS ERECTED IN ACCORDANCE WITH CURRENT VIRGINIA DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE SPECIFICATIONS AND ROAD DESIGN STANDARDS, BOTETOURT COUNTY, VIRGINIA DESIGN STANDARDS AND SPECIFICATIONS. ALL MATERIALS USED SHALL BE TESTED IN ACCORDANCE WITH STANDARD POLICIES. THE DEVELOPER MUST CONTACT THE OFFICE OF THE COUNTY ENGINEER PRIOR TO BEGINNING ANY CONSTRUCTION AT WHICH TIME AN INSPECTION AND TESTING PROCEDURE POLICY WILL BE DRAWN. THE DEVELOPER WILL PRODUCE TEXT REPORTS FROM APPROVED INDEPENDENT LABORATORIES AT THE DEVELOPER'S EXPENSE.

THE PAVEMENT DESIGNS SHOWN ARE BASED ON A SUBGRADE CBR VALUE OF 10 OR GREATER. THE SUBGRADE SOIL IS TO BE TESTED BY AN INDEPENDENT LABORATORY AND THE RESULTS SUBMITTED TO THE VIRGINIA DEPARTMENT OF TRANSPORTATION PRIOR TO BASE CONSTRUCTION. SHOULD THE SUBGRADE CBR VALUES BE LESS THAN 10, THEN ADDITIONAL BASE MATERIAL WILL BE REQUIRED IN ACCORDANCE WITH DEPARTMENTAL SPECIFICATIONS.

THE SUBGRADE MUST BE APPROVED BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION PRIOR TO PLACEMENT OF THE BASE. BASE MUST BE APPROVED BY VIRGINIA DEPARTMENT OF TRANSPORTATION FOR DEPTH, TEMPLATE AND COMPACTION BEFORE SURFACE IS APPLIED.

2. UTILITIES

ALL NECESSARY UTILITY LATERALS ALONG WITH PROVISIONS FOR CONDUITS (I.E. WATER, SEWER, STORM, GAS, AND TELEPHONE) WILL BE CONSTRUCTED PRIOR TO PLACEMENT OF BASE MATERIAL.

GAS OR PETROLEUM TRANSMISSION LINES WILL NOT BE PERMITTED WITHIN THE PAVEMENT OR SHOULDER ELEMENT (BACK OF CURB TO BACK OF CURB) OF THIS DEVELOPMENT. SERVICE LATERALS CROSSING AND PIPE LINES LOCATED OUTSIDE THE PAVEMENT, BUT INSIDE THE RIGHT OF WAY, WILL BE CONSTRUCTED IN CONFORMITY WITH ASA B 31.8 SPECIFICATIONS AND SAFETY REGULATIONS. DISTRIBUTION LINES WITH PRESSURES LESS THAN 120 LBS. ARE UNAFFECTED BY THE ABOVE.

PERMITS WILL BE REQUIRED FOR ALL UTILITIES WITHIN STREET RIGHT OF WAY PRIOR TO ACCEPTANCE INTO THE SECONDARY HIGHWAY SYSTEM.

ANY EASEMENTS GRANTED TO A UTILITY COMPANY FOR PLACEMENT OF POWER, TELEPHONE, ETC. MUST BE RELEASED PRIOR TO ACCEPTANCE.

3. PRIVATE ENTRANCES

MODIFIED CG-9D GUTTER WILL BE PROVIDED AT ALL ENTRANCES TO PRIVATE LOTS WHERE STANDARD CG-6 CURB AND GUTTER IS APPROVED FOR

DRIVEWAYS CONNECTING TO ROADS WITHOUT CURB & GUTTER SHALL CONFORM TO THE PAVEMENT, SHOULDER & SLOPE.

PERMITS WILL BE REQUIRED FOR ALL PRIVATE ENTRANCES CONSTRUCTED ON STREET RIGHTS-OF-WAY AFTER ACCEPTANCE INTO THE SECONDARY HIGHWAY SYSTEM.

4. EROSION CONTROL AND LANDSCAPING

CARE MUST BE TAKEN DURING CONSTRUCTION TO PREVENT EROSION, DUST AND MUD FROM DAMAGING ADJACENT PROPERTY, CLOGGING DITCHES. STREAKING PUBLIC STREETS AND OTHERWISE CREATING A PUBLIC OR PRIVATE NUISANCE TO SURROUNDING AREAS.

THE ENTIRE CONSTRUCTION AREA INCLUDING DITCHES, CHANNELS, BACK OF CURBS AND OR PAVEMENT ARE TO BE BACKFILLED AND SEEDED AT THE EARLIEST POSSIBLE TIME AFTER FINAL GRADING.

DRAINAGE EASEMENTS MUST BE DEFINED BY EXCAVATED DITCHES OR CHANNELS FOR THEIR FULL LENGTH TO WELL DEFINED EXISTING NATURAL WATERCOURSES.

THE ROAD WILL BE REVIEWED DURING CONSTRUCTION FOR THE NEED OF PAVED DITCHES. IF EROSION IS ENCOUNTERED IN ANY DRAINAGE EASEMENT, IT WILL BE THE RESPONSIBILITY OF THE DEVELOPER TO SOD, RIP RAP, GROUT, PAVE, OR TO DO WHATEVER IS NECESSARY TO CORRECT THE PROBLEM.

ALL VEGETATION AND OVERBURDEN TO BE REMOVED FROM SHOULDER TO SHOULDER PRIOR TO THE CONDITIONING (CUTTING AND/OR PREPARATION) OF THE SUBGRADE.

5. INTERSECTION PAVEMENT RADIUS

MINIMUM PAVEMENT RADIUS OF 35 FEET IS REQUIRED AT ALL STREET INTERSECTIONS.

6. CONNECTIONS TO STATE-MAINTAINED ROADS

WHILE THESE PLANS HAVE BEEN APPROVED, SUCH APPROVAL DOES NOT EXEMPT CONNECTIONS WITH EXISTING STATE-MAINTAINED ROADS FROM CRITICAL REVIEW AT THE TIME PERMIT APPLICATIONS ARE MADE. THIS IS NECESSARY IN ORDER THAT THE PREVAILING CONDITIONS BE TAKEN INTO CONSIDERATION REGARDING SAFETY ACCOMPANIMENTS SUCH AS TURNING LANES.

7. GUARDRAILS

STANDARD GUARDRAIL WITH SAFETY END SECTIONS MAY BE REQUIRED ON FILLS AS DEEMED NECESSARY BY THE TOWN ENGINEER. AFTER COMPLETION OF ROUGH GRADING OPERATIONS, THE OFFICE OF THE VDOT ENGINEER SHALL BE NOTIFIED SO THAT A FIELD REVAIEW MAY BE MADE OF THE PROPOSED LOCATIONS.

WHERE GUARDRAILS ARE TO BE INSTALLED, THE SHOULDER WIDTH SHALL BE INCREASED IN ACCORDANCE WITH VDOT ROAD AND BRIDGE STANDARDS.

8. STORM DRAINAGE

FIELD REVIEW WILL BE MADE DURING CONSTRUCTION TO DETERMINE THE NEED AND LIMITS OF PAVED DITCHES AND/OR DITCH STABILIZATION TREATMENTS, AND TO DETERMINE THE NEED AND LIMITS OF ADDITIONAL DRAINAGE EASEMENTS. ALL DRAINAGE EASEMENTS MUST BE CUT AND MADE TO FUNCTION TO A NATURAL WATERCOURSE. ANY EROSION PROBLEMS ENCOUNTERED IN AN EASEMENT MUST BE CORRECTED. DITCH SLOPES ARE TO BE FOUR TO ONE (4:1) FOR SHOULDER WIDTHS OF SIX FEET (6') OR GREATER AND THREE TO ONE (3:1) FOR SHOULDER WIDTHS OF FOUR FEET (4') OR FIVE FEET (5'), UNLESS OTHERWISE SPECIFIED IN THE PLANS.

9. ENTRANCE PERMIT

CONTRACTOR SHALL OBTAIN ENTRANCE PERMIT TO THE EXISTING PERMITTING AGENT PRIOR TO ROAD CONSTRUCTION.

RIGHT-OF-WAY FROM VDOT

10. INSPECTION

AN INSPECTOR WILL NOT BE FURNISHED EXCEPT FOR PERIODIC PROGRESS INSPECTION, THE ABOVE MENTIONED FIELD REVIEWS AND CHECKING FOR REQUIRED STONE DEPTHS. THE DEVELOPER WILL BE REQUIRED TO POST A SURETY TO GUARANTEE THE ROAD FREE OF DEFECTS FOR ONE YEAR AFTER ACCEPTANCE BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION

11. STREET MAINTENANCE

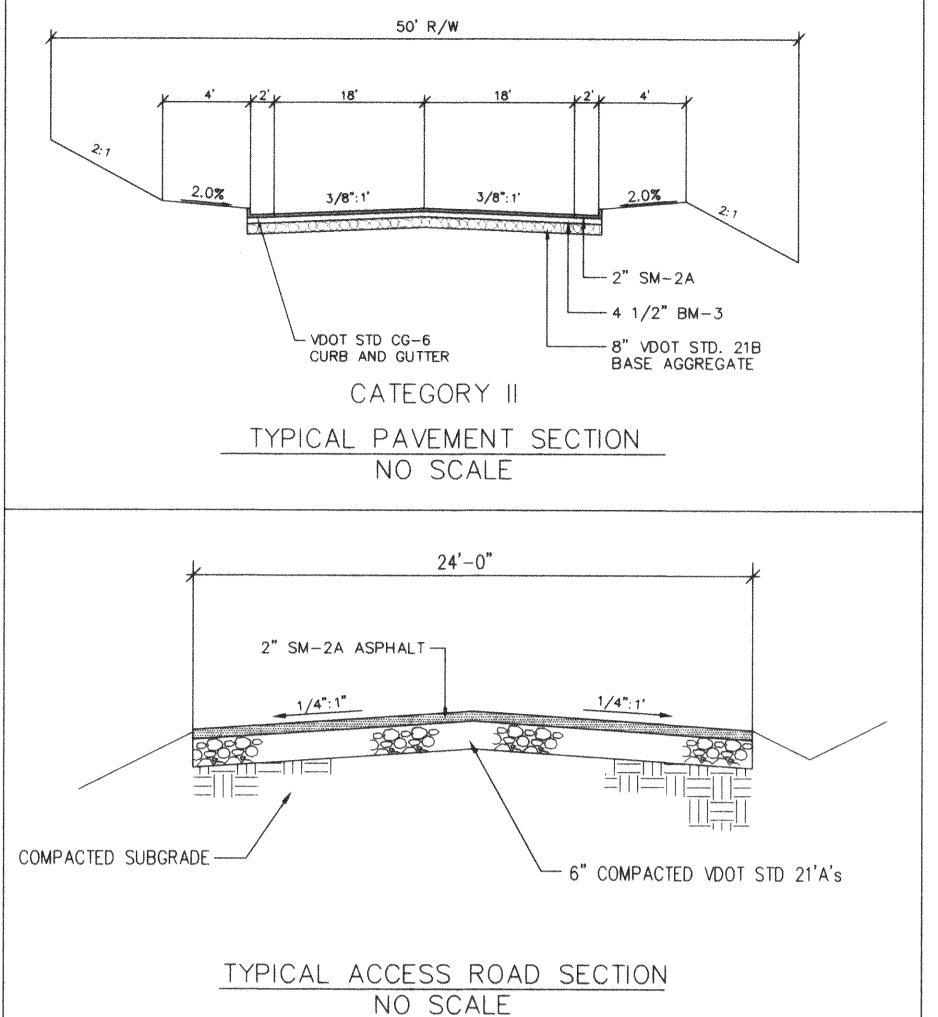
THE STREETS MUST BE PROPERLY MAINTAINED UNTIL ACCEPTANCE. AT SUCH TIME AS ALL REQUIREMENTS HAVE BEEN MET FOR ACCEPTANCE. ANOTHER INSPECTION WILL BE MADE TO DETERMINE THAT THE STREET HAS BEEN PROPERLY MAINTAINED.

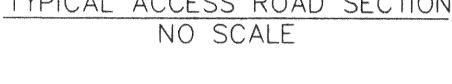
12. UNDERGROUND UTILITIES

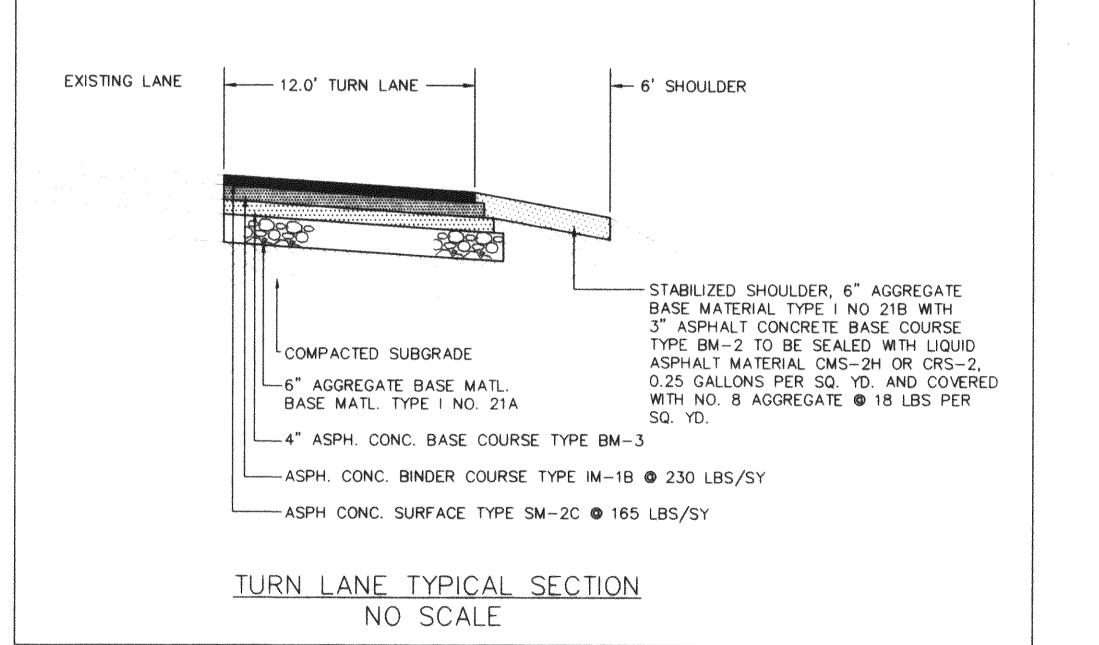
CONTRACTOR SHALL VERIFY LOCATION AND ELEVATION OF ALL UNDERGROUND UTILITIES SHOWN ON THE PLANS IN AREAS OF CONSTRUCTION PRIOR TO STARTING WORK BY CONTACTING MISS UTILITY. CONTACT CONSULTING ENGINEER IMMEDIATELY IF LOCATION OR ELEVATION IS DIFFERENT FROM THAT SHOWN ON THE PLANS. IF THERE APPEARS TO BE A CONFLICT AND UPON DISCOVERY OF ANY UTILITY NOT SHOWN ON THIS PLAN. CALL "MISS UTILITY" OF CENTRAL VIRGINIA AT 1-800-552-7001.

13. REVISIONS OF SPECIFICATIONS AND STANDARDS

APPROVAL OF THESE PLANS WILL BE BASED ON SPECIFICATIONS AND STANDARDS IN EFFECT AT THE TIME OF APPROVAL AND WILL BE SUBJECT, UNTIL COMPLETION OF THE ROADWAY AND ACCEPTANCE BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION, TO FUTURE REVISIONS OF THE SPECIFICATIONS AND STANDARDS.







SUMMERFIELD VILLAGE

Project: 1702 Mon Oct 26 12:46:40 1998 Horizontal Incremental Stationing Report.

Alignment: street10 Desc: MAIN PUBLIC STREET INTO SUMMERFIELD

Station	Northing	Easting	Tangential Direction
10+00	4915.0652	7314.9007	N 89-56-43 W
10+25	4915.0891	7289.9007	N 89-56-43 W
10+50	4915.1130	7264.9007	N 89-56-43 W
10+75	4915.1369	7239.9007	N 89-56-43 W
11+00	4915.1383	7214.9008	S 89-11-50 W
11+25	4913.2286	7189.9902	S 82-02-07 W
11+50	4908.7245	7165.4028	S 78-57-29 W
11+75	4903.9363	7140.8656	S 78-57-29 W
12+00	4899.1481	7116.3285	S 78-57-29 W
12+25	4894.3600	7091.7913	S 78-57-29 W
12+50	4889.5718	7067.2541	S 78-57-29 W
12+75	4884.7836	7042.7169	S 78-57-29 W
13+00	4879.9954	7018.1797	S 78-57-29 W
13+25	4875.2073	6993.6425	S 78-57-29 W
13+50	4869.4937	6969.3174	S 73-22-29 W
13+75	4860.8643	6945.8713	S 66-12-46 W
14+00	4849.3790	6923.6840	S 59-03-03 W
14+25	4835.2172	6903.1017	S 51-53-19 W
14+50	4818.5998	6884.4456	S 44-43-36 W
14+75	4799.7861	6868.0069	S 37-33-53 W
15+00	4779.0697	6854.0420	S 30-24-10 W
15+25	4756.7739	6842.7689	S 23-14-27 W
15+50	4733.2466	6834.3635	S 16-04-44 W
15+75	4708.8549	6828.9569	S 08-55-01 W
16+00	4683.9794	6826.6336	S 01-45-18 W
16+25	4658.9859	6826.0648	S 01-17-18 W
16+50	4633.9922	6825.5027	S 01-17-18 W
16+75	4608.9985	6824.9405	S 01-17-18 W
17+00	4584.0048	6824.3784	S 01-17-18 W
17+25	4559.0112	6823.8162	S 01-17-18 W
17+50	4534.0175	6823.2541	S 01-17-18 W
17+75	4509.0238	6822.6919	S 01-17-18 W
18+00	4484.0301	6822.1298	S 01-17-18 W
18+25	4459.0364	6821.5676	S 01-17-18 W
18+50	4434.0428	6821.0055	S 01-17-18 W
18+75	4409.0491	6820.4433	S 01-17-18 W
19+00	4384.0554	6819.8812	S 01-17-18 W
19+25	4359.0617	6819.3190	S 01-17-18 W
19+50	4334.0681	6818.7569	S 01-17-18 W
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RICHARD C WHITE No. 020021 6-18-4 esigned By JDC DRM

Drawn By hecked By Approved By Drawing 1702TRN2.DW 11/2/98

NONE

Commission No. 1702

PROJECT DESCRIPTION

The purpose of this project is to construct a combination commercial park, office park, and residential subdivision with associated streets, water, sewer, storm sewer, and swm facilities.

EXISTING SITE CONDITIONS

The proposed development exists on approximately 12 acres of gently rolling farmland. Some moderate woods exist on site.

ADJACENT AREAS

The site is bordered on the north by residential areas. To the east by the south bound lane of Rte. 220 / Rte. 654. To the south and west by wooded areas. Access can be obtained from Rte. 220 and Rte. 654 to the west.

SOILS

Soils found at this site are common to the area. None of these soils have high erosion tendencies.

CRITICAL EROSION AREAS

The potential critical erosion areas are: 1. Steep roadside ditch slopes along proposed roads. 2. The outlet of all culverts.

EROSION AND SEDIMENT CONTROL MEASURES

Unless otherwise indicated, all vegetative and structural erosion and sediment control practices shall be constructed and maintained according to minimum standards and specifications of the handbook. The minimum standards of the VESCR shall be adhered to unless otherwise waived or approved by a variance.

STRUCTURAL PRACTICES

- 1. Temporary Construction Entrance (Section 3.02) One temporary construction entrance will be installed. Vehicles will be washed to limit tracking onto public roads. Should tracking occur the road will be immediately cleaned. Temporary straw bale barriers will be placed and entrenched and anchored as indicated on the site plan.
- 2. Straw Bale Barrier (Section 3.04)
- 3. Silt Fence (Section 3.05) Temporary silt fences will be installed as indicated on the site plan.
- 4. Outlet Protection (Section 3.18) Outlet protection will be placed at all discharge points from controlled flow to open flow. All outlet protection will be permanently designed and installed.
- 5. Surface Roughening (Section 3.29) Surface roughening will be employed on all slopes exceeding 2:1.
- Temporary Seeding (Section 3.31) Temporary seeding will be placed on all disturbed areas that will not be brought to final grade within one year or less. Temporary seeding will aid in the reduction of dust and sediment. Temporary seeding will be Annual Ryegrass (100 #/ac), Feb 16 - April 30, German Millet (60 #/ac), May 1 - Aug. 31.
- 7. Permanent Seeding (Section 3.32) After final grading permanent seeding will be employed to reduce erosion and sediment yield.

Seeding Specifications:

Permanent seeding will be Kentucky Bluegrass, blended to contain 4 or more varieties, with no one variety exceeding 30%. The seeding will be applied at 140 lb. per acre. On slopes 2:1 or greater a mixture of Crown Vetch (50%), Perennial Ryegrass (40%), and Redtop (10%) will be used.

All seeding, with required associated practices, will be in accordance with all applicable sections of the Virginia Erosion and Sediment Control

- 8. Dust Control (Section 3.39) If arid conditions prevail dust control practices will be employed as required.
- 9. Construction Road Stabilization (Section 3.03) All roads and parking areas on the site shall be stabilized with gravel immediately after grading. Traffic is prohibited from entering drainage swales or streams unless absolutely necessary.
- 10. Temporary Sediment Basin (Section 3.14)
- 11. Riprap (Section 3.19) Riprap shall be placed at the outlet of all pipes in accordance with VDOT standard EC-2 as indicated on the plans. Riprap along the ditches shall be VDOT Class 1 riprap installed over a six inch filter consisting of #57 stone.
- 12. Check Dams (Section 3.20)

MANAGEMENT

- 1. Construction should be sequenced so that grading operations can begin and end as quickly as possible.
- 2. Erosion and Sediment control devices shall be installed as the first step of construction.
- 3. Areas which are not to be disturbed shall be clearly marked by flags, signs, etc.
- 4. The grading contractor shall be responsible for the installation and maintenance of all erosion and sediment control practices. Inspections are to be made periodically and after every significant rainfall.
- 5. After achieving adequate stabilization, the temporary E&S controls will be cleaned up and removed, and the sediment basins will be cleaned out and converted to permanent stormwater management basins.

PERMANENT STABILIZATION

All areas disturbed by construction shall be stabilized with permanent seeding immediately following finish grading. Seeding shall be done with Kentucky 31 Tall Fescue according to Std. & Spec. 3.32, PERMANENT SEEDING, of the handbook. Erosion control blankets will be installed over fill slopes which have been brought to final grade and have been seeded to protect the slopes from rill and gully erosion and to allow seed to germinate properly. Mulch (straw or fiber) will be used on relatively flat areas. In all seeding operations, seed, fertilizer and lime will be applied prior to mulching.

MAINTENANCE OF DENTENTION FACILITIES

The applicant shall obtain approval from the locality of a plan for maintenance of the dentention facilities. The plan shall set forth the maintenance requirements of the facility and the person responsible for performing the maintenance.

STORMWATER MANAGEMENT

Calculation of runoff before and after development indicates that there will be a net increase in peak runoff as a result of project development. Consequently, stormwater management basins have been designed to detain and release the runoff at the 2-year pre-developed rate. (See attached calculations)

MAINTENANCE

In general, all erosion and sediment control measures will be checked daily and after each significant rainfall. Any items not found in accordance with the Virginia Erosion and Sediment Control Handbook will be immediately replaced and/or repaired. The following items will be checked in particular:

- 1. The sediment basin will be cleaned out when the level of sediment buildup reaches the cleanout point indicated on the riser pipe.
- 2. The gravel outlets will be checked regularly for sediment buildup which will prevent drainage. If the gravel is clogged by sediment, it shall be removed and cleaned or replaced.
- 3. The silt fence barrier will be checked regularly for undermining or deterioration of the fabric. Sediment shall be removed when the level of sediment deposition reaches half way to the top of the barrier.
- 4. The seeded areas will be checked regularly to ensure that a good stand is maintained. Areas should be fertilized and reseeded as needed.

GENERAL

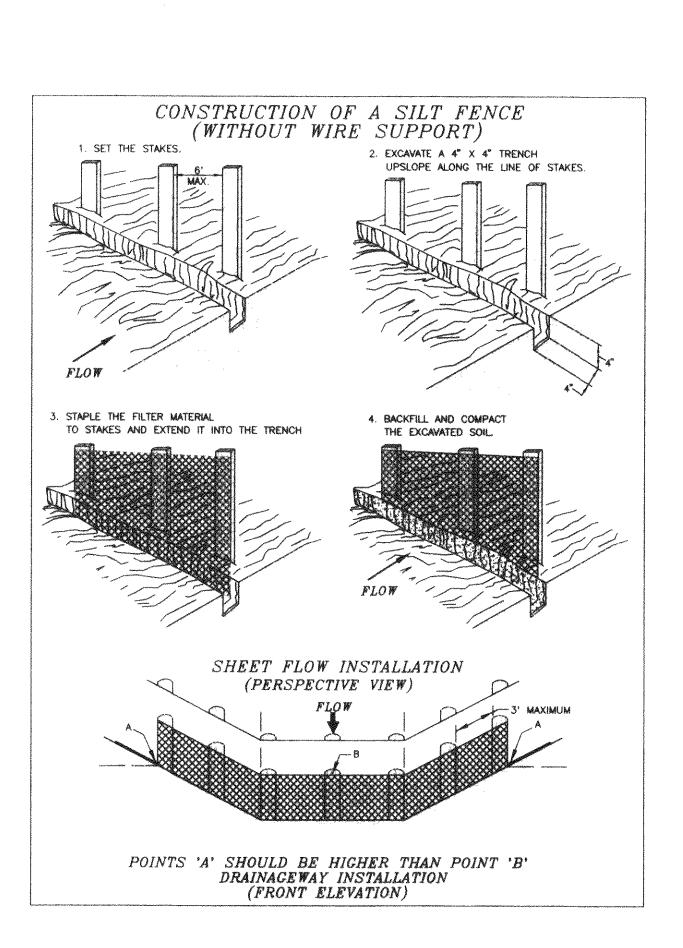
The erosion and sediment control measures shown on the construction plans are the minimum measures required. Due to construction phasing and other considerations all measures can not be shown. The owner, through his contractor, will employ whatever measures which may be required to assure that sediment laden runoff does not leave the site.

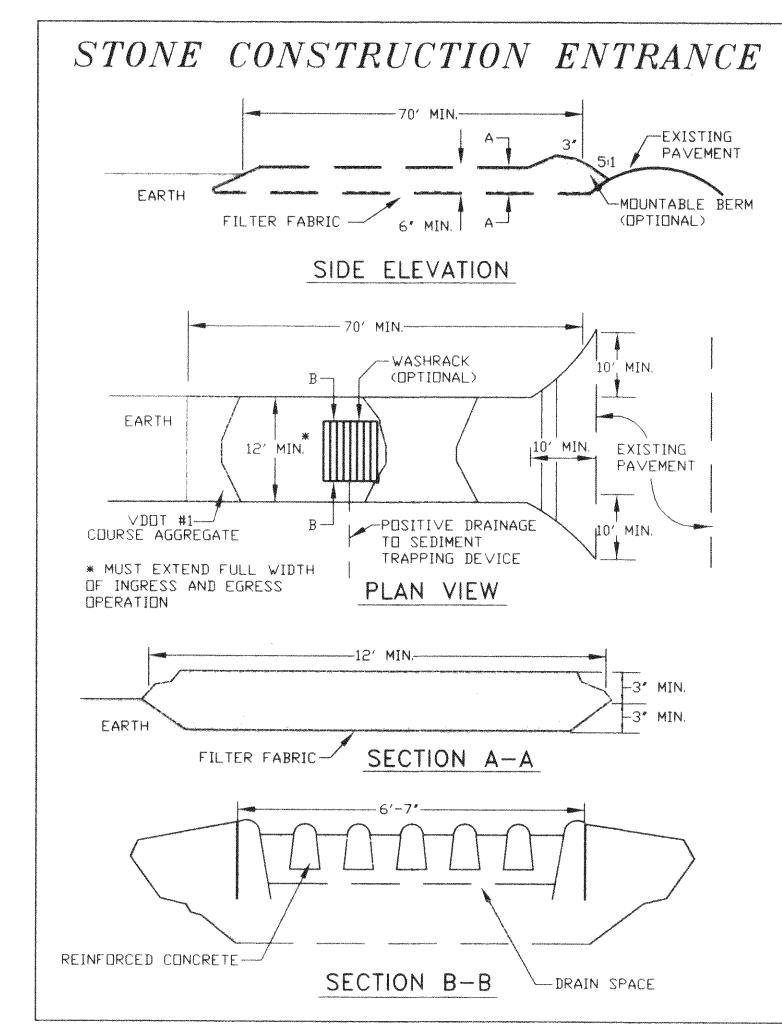
All materials and measures employed for erosion and sediment control will be in accordance with the Virginia Erosion and Sediment Control Handbook, latest edition.

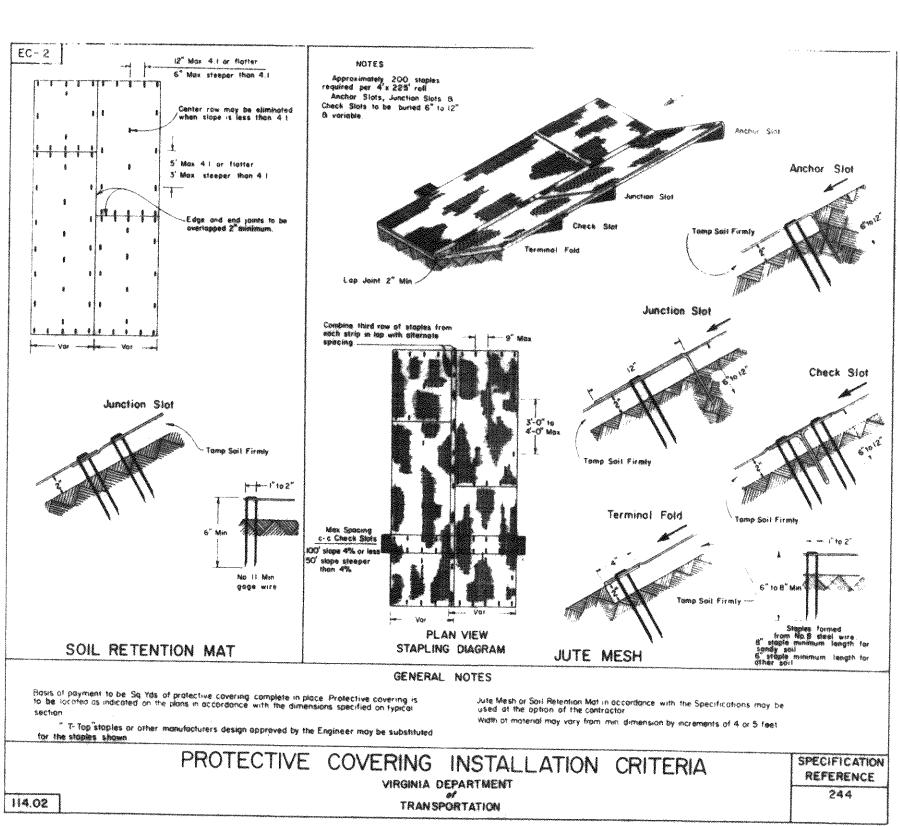
If, during construction, additional Erosion and Sediment Control measures are deemed necessary, they shall be installed as directed by the Owner, Engineer or County agent.

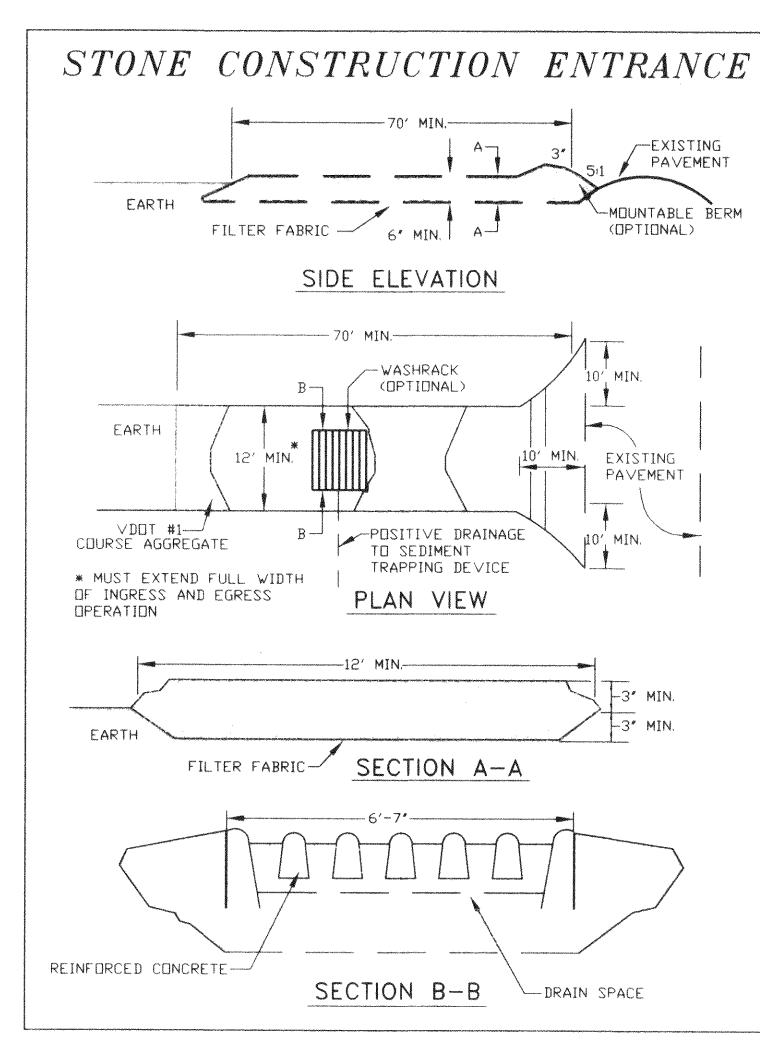
This project is to be constructed consistent with the 1992 Virginia Erosion And Sediment Control Regulations.

CONSTRUCTION OF A SILT FENCE (WITH WIRE SUPPORT) 1. SET POSTS AND EXCAVATE A 4"X4" 2. STAPLE WIRE FENCING TO THE POSTS. TRENCH UPSLOPE ALONG THE LINE FLOW . ATTACH THE FILTER FABRIC TO THE WIRE 4. BACKFILL AND COMPACT THE FENCE AND EXTEND IT INTO THE TRENCH. EXCAVATED SOIL. FILTER FABRIC ----EXTENSION OF FABRIC AND WIRE INTO THE TRENCH.





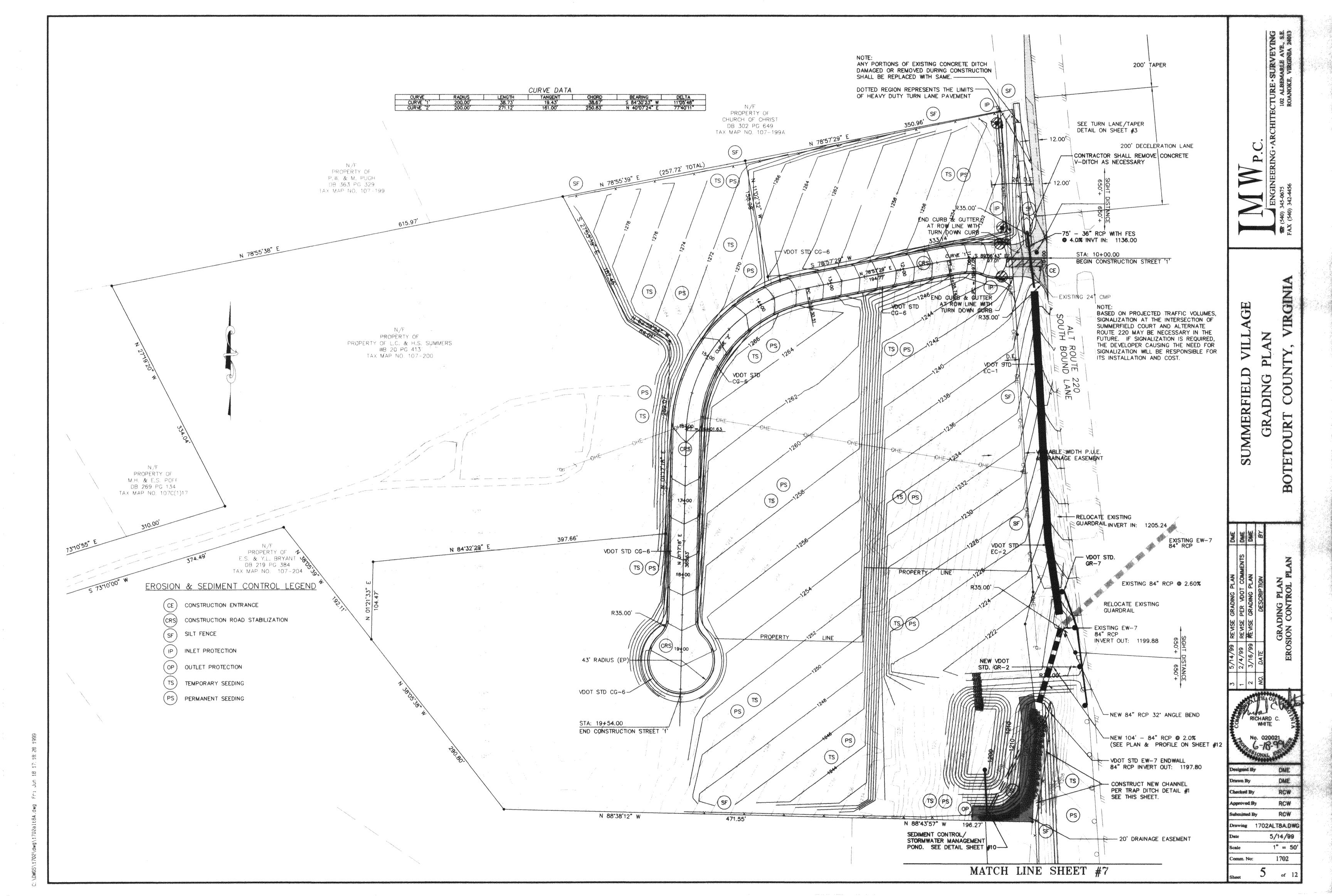


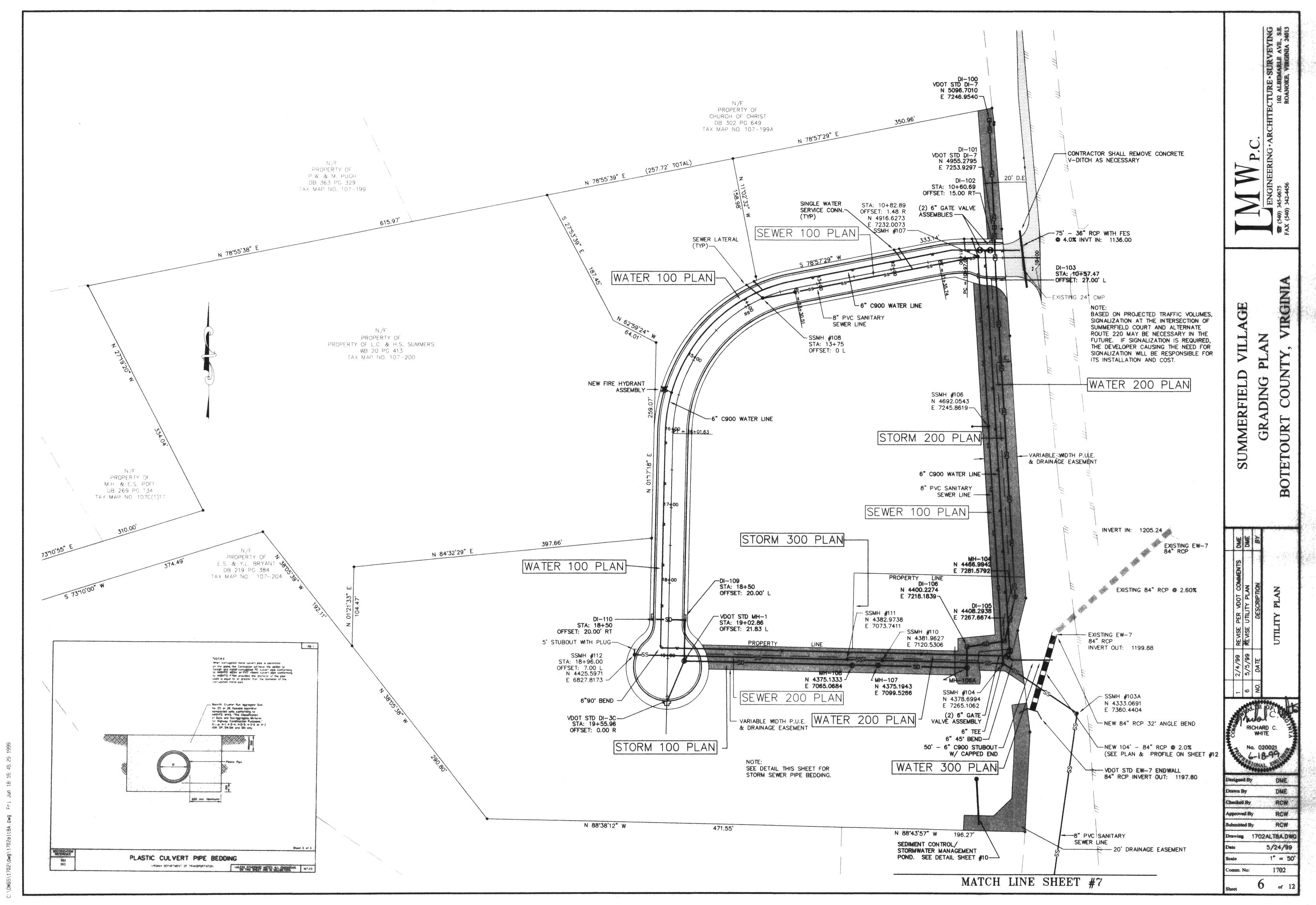


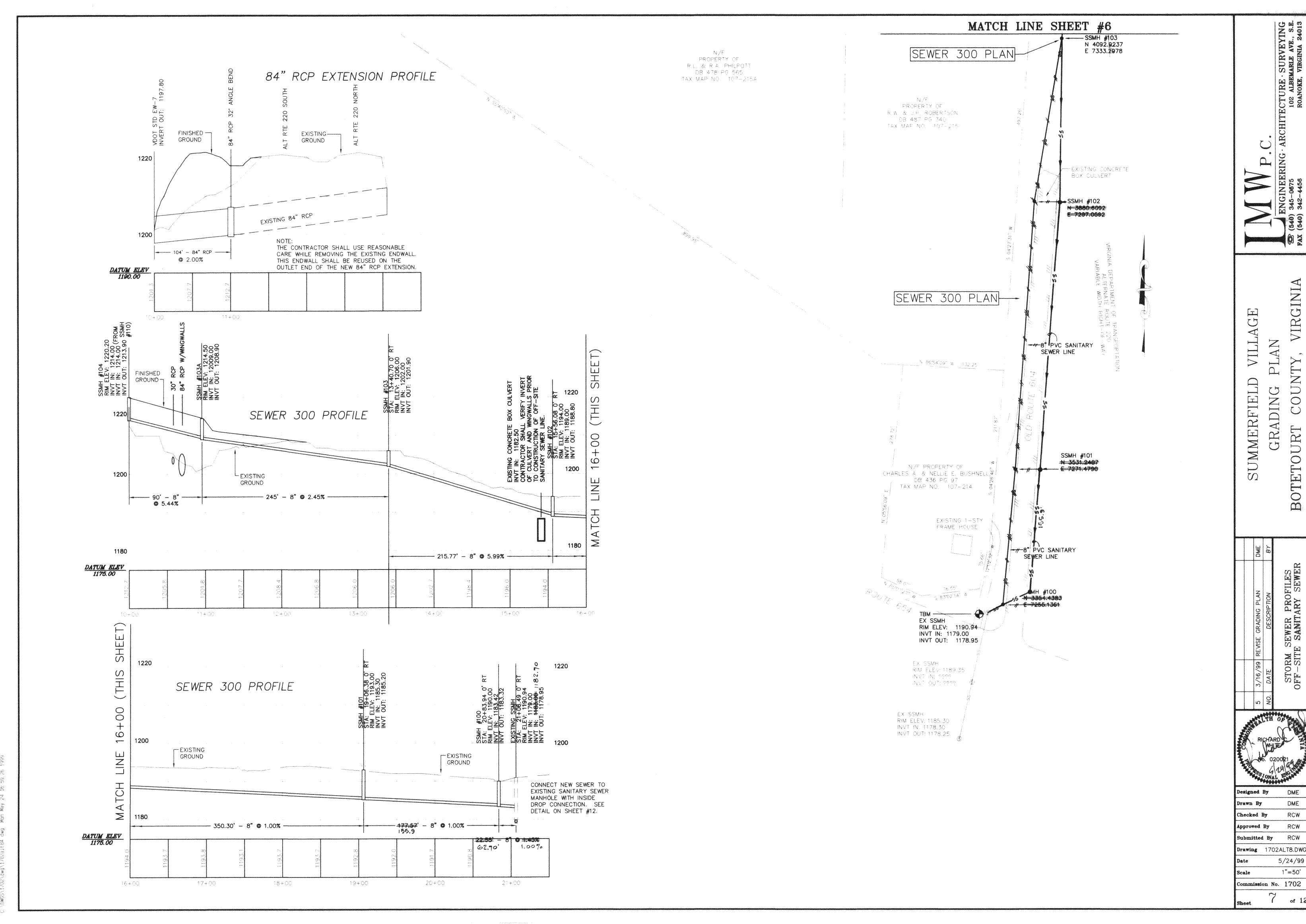
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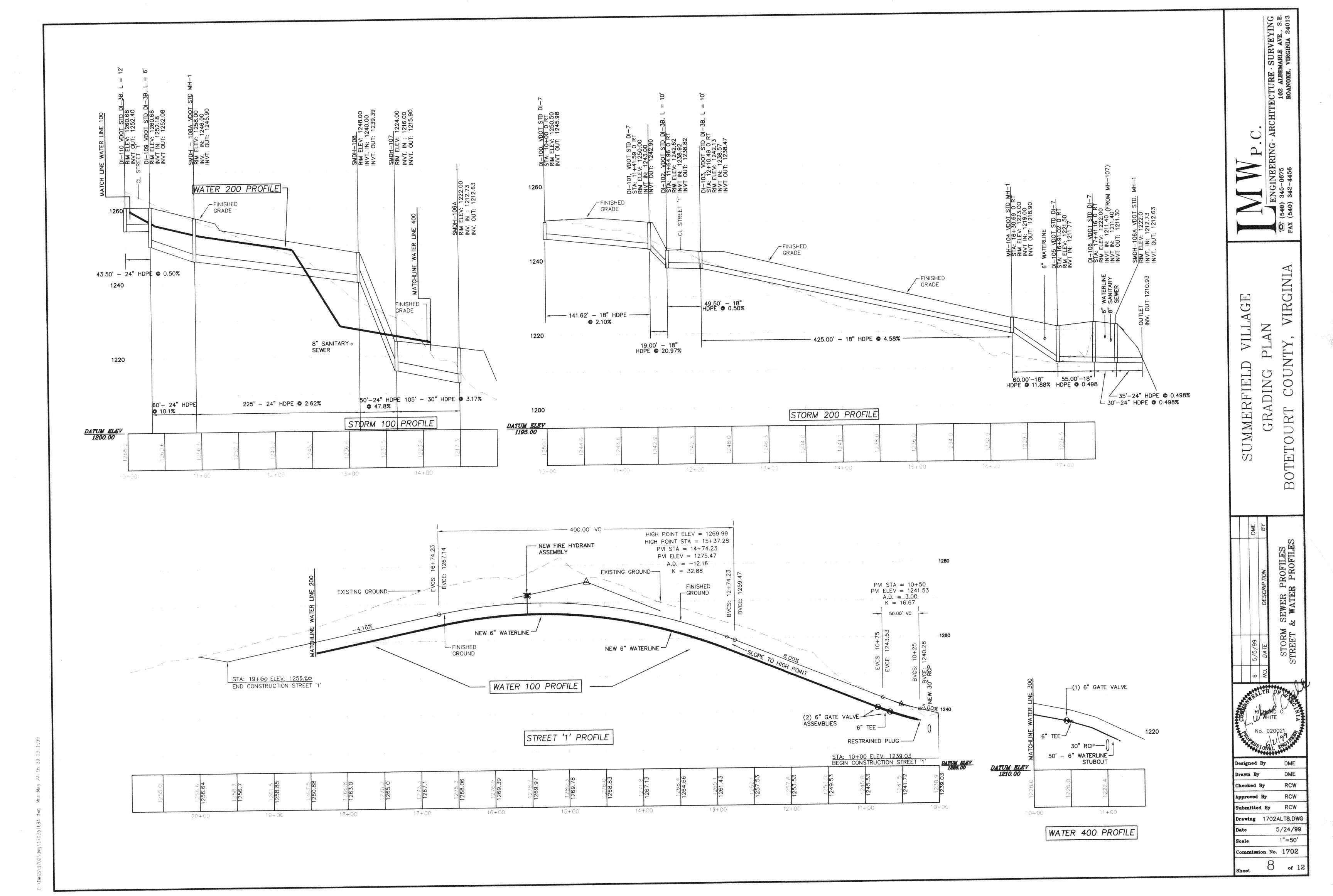
esigned By Drawn By JDC Checked By DRM RCW Approved By Submitted By RCW Drawing 1702EROS.DWG 11/2/98 NONE mmission No. 1702

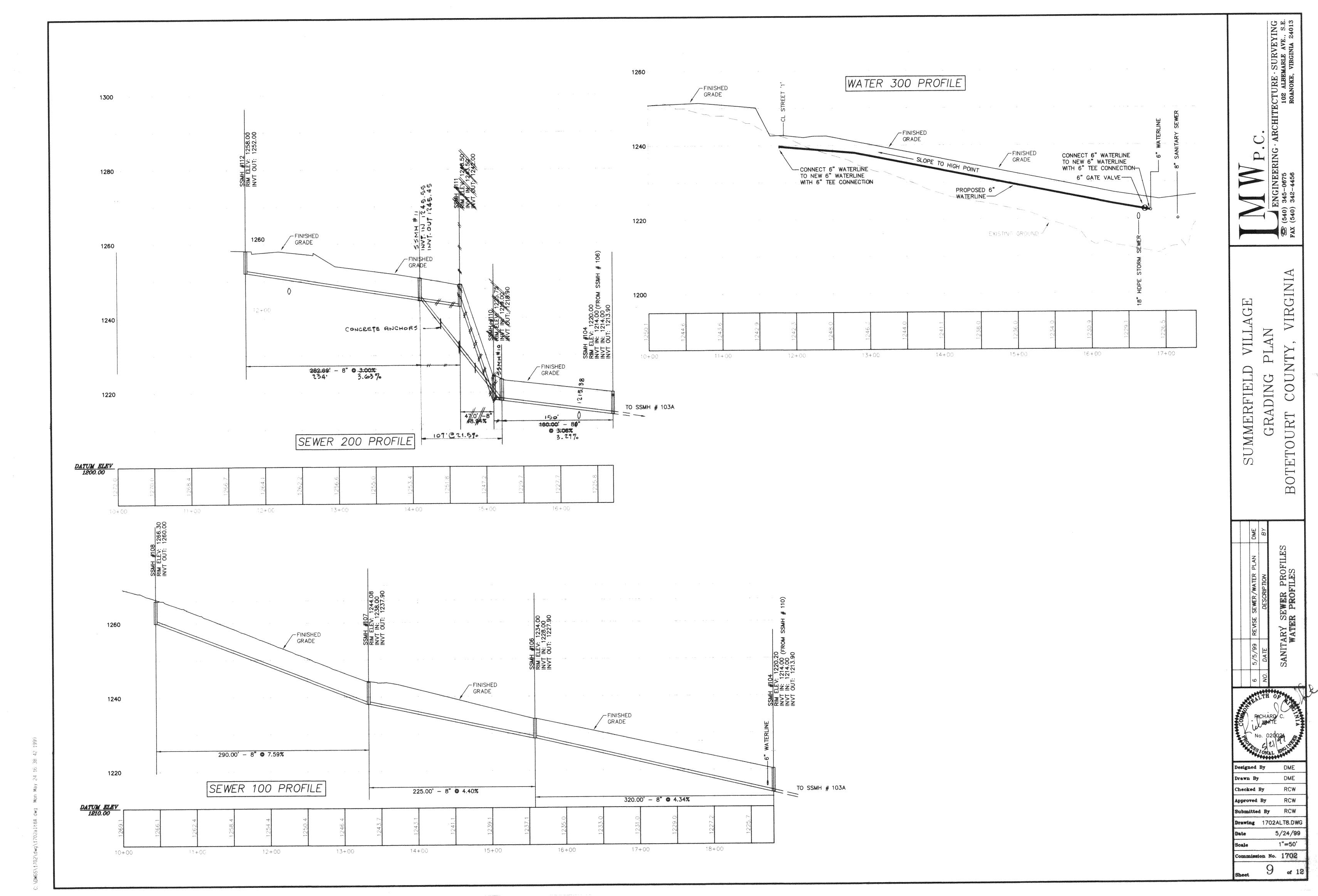




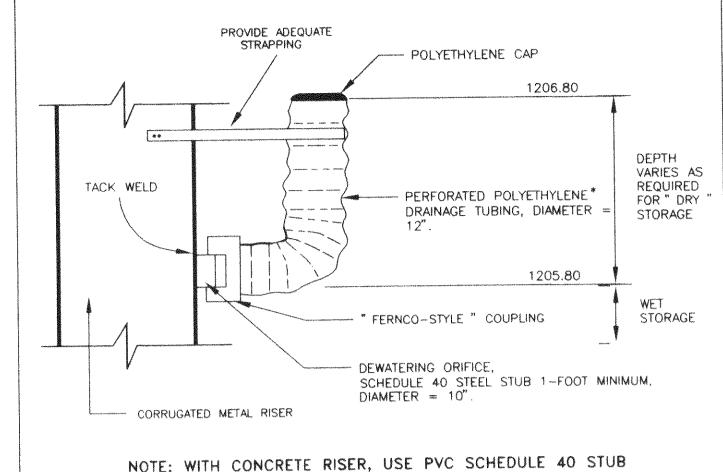


VIRGINIA





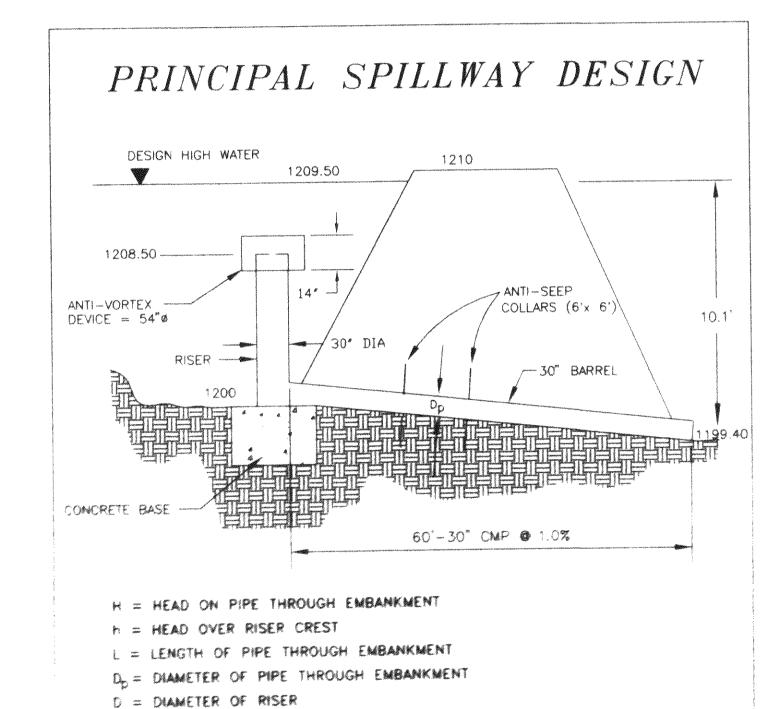
RECOMMENDED DEWATERING SYSTEM FOR SEDIMENT BASINS



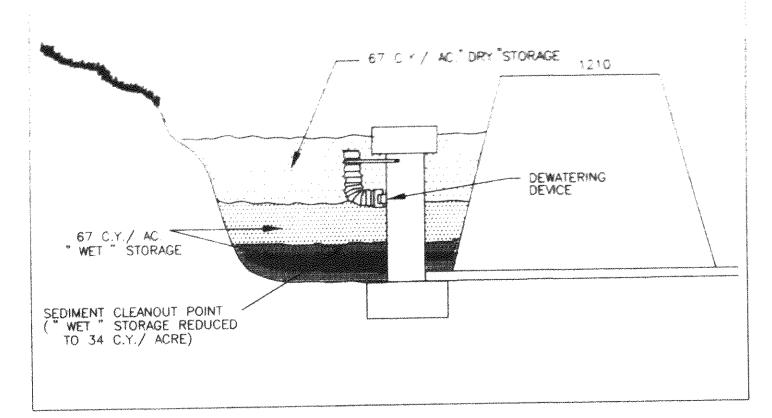
FOR DEWATERING ORIFICE

*DRAINAGE TUBING SHALL COMPLY WITH ASTM F667

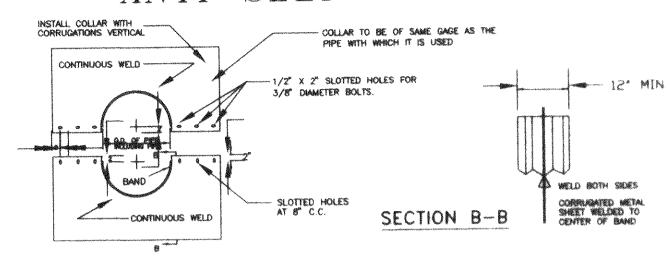
AND AASHTO M294



MINIMUM STORAGE VOLUME AND SEDIMENT STORAGE



DETAILS OF CORRUGATED METAL ANTI-SEEP COLLAR



ELEVATION OF UNASSEMBLED COLLAR

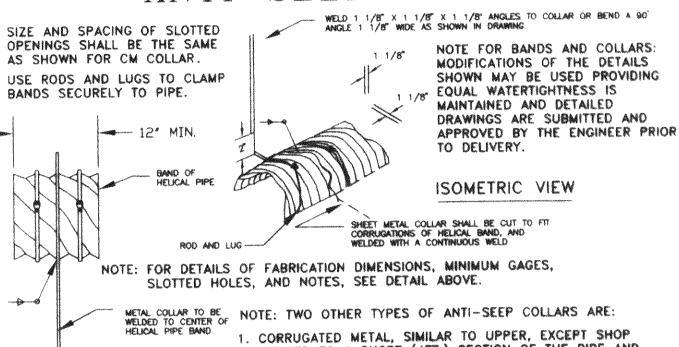
1. ALL MATERIALS TO BE IN ACCORDANCE WITH 4. THE LAP BETWEEN THE TWO HALF SECTIONS CONSTRUCTION AND CONSTRUCTION MATERIAL SPECIFICATIONS. 2. WHEN SPECIFIED ON THE PLANS, COATING OF COLLARS SHALL BE IN ACCORDANCE WITH CONSTRUCTION AND CONSTRUCTION MATERIAL

REF. ENGR. FIELD MANUAL

PAINTING OR TAGGING TO IDENTIFY MATCHING AND BETWEEN THE PIPE AND CONNECTING BAND SHALL BE CAULKED WITH ASPHALT MASTIC AT TIME OF INSTALLATION. 5. EACH COLLAR SHALL BE FURNISHED WITH TWO 1/2" DIAMETER RODS WITH STANDARD TANK LUGS FOR CONNECTING COLLARS TO PIPE.

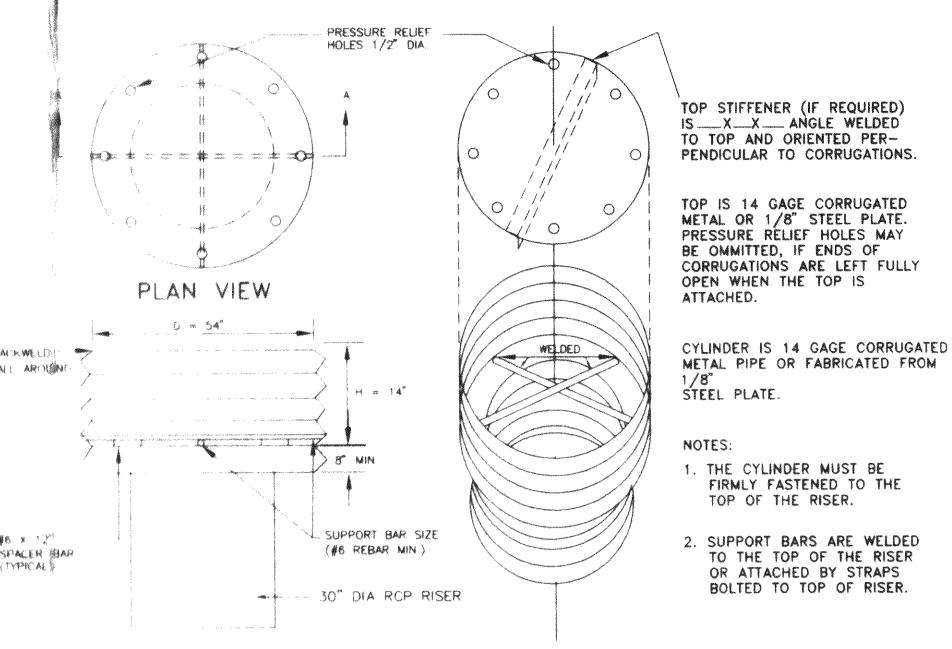
UNASSEMBLED COLLARS SHALL BE MARKED BY

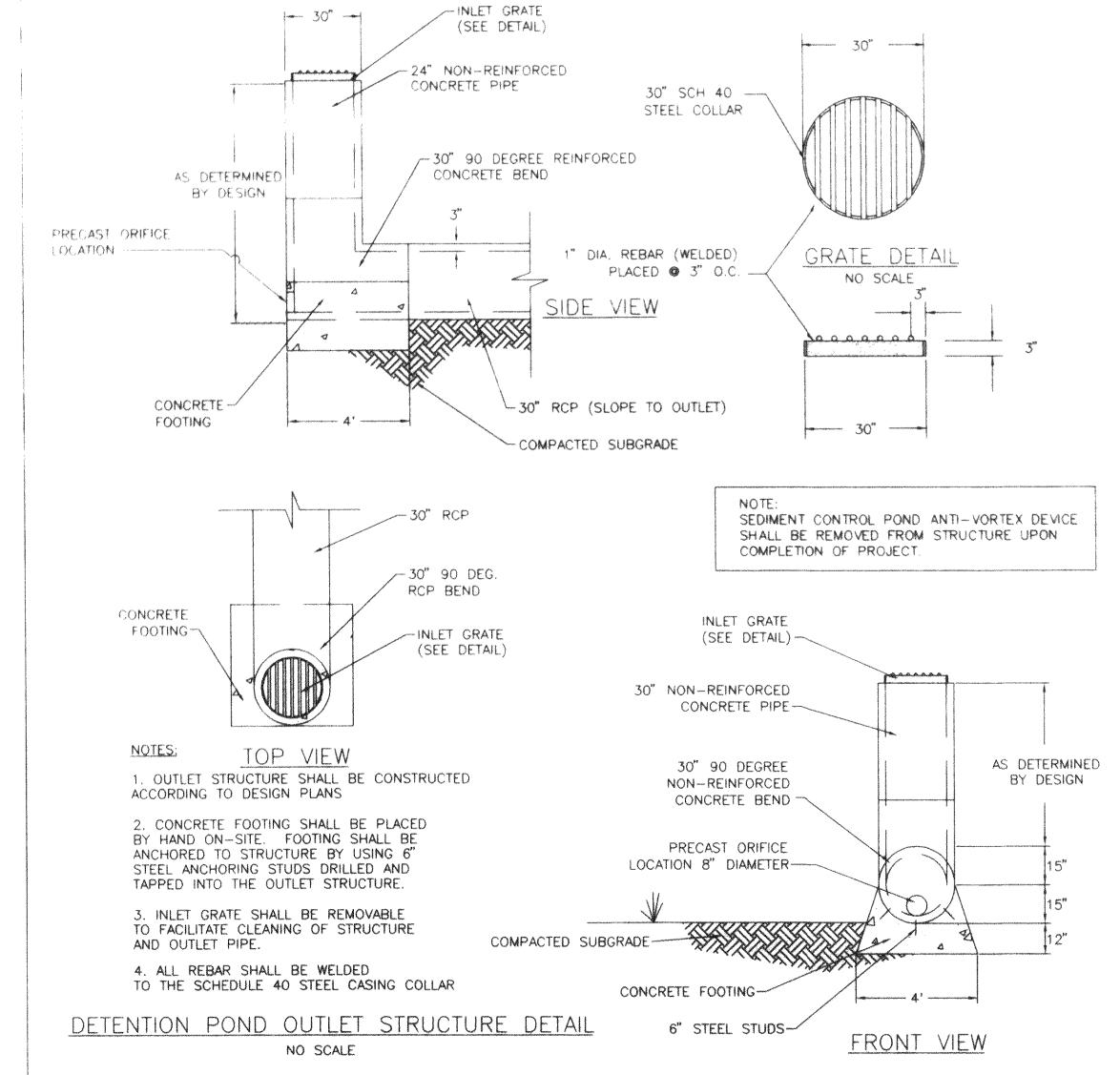
DETAIL OF HELICAL PIPE ANTI-SEEP COLLAR

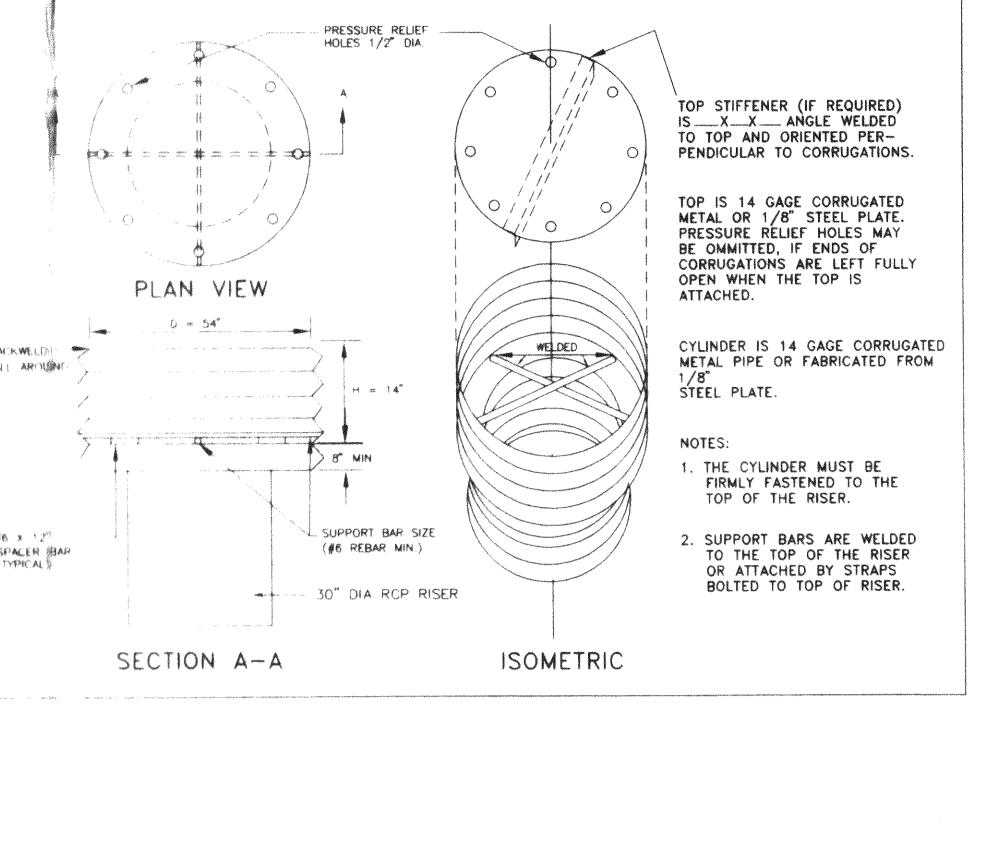


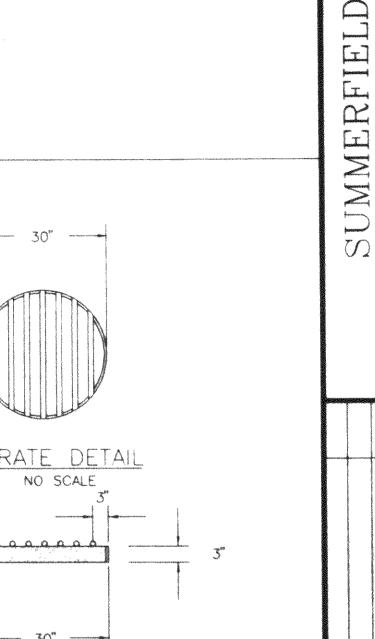
WELDED TO A SHORT (4FT.) SECTION OF THE PIPE AND CONNECTED WITH CONNECTING BANDS TO THE PIPE. 2. CONCRETE, SIX INCHES THICK FORMED AROUND THE PIPE PARTIAL ELEVATION WITH #3 REBAR SPACED 15" HORIZONTALLY AND VERTICALLY.

ANTI- VORTEX DEVICE DESIGN









EDIMENT POND OUTLE Designed By DME Drawn By Checked By DRM Approved By Submitted By Drawing 1702PND.DWG 08/26/98

NONE

10 or 12

mmission No. 1702

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14 A PRECONSTRUCTION CONFERENCE SHALL BE SCHEDULED WITH BOTETOURT COUNTY PRIOR TO COMMENCING WITH CONSTRUCTION.

NO WORK SHALL BEGIN WITHOUT NOTIFYING BOTETOURT COUNTY 24 HOURS IN ADVANCE. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ANY AND ALL NECESSARY PERMITS

3. NO WORK SHALL BEGIN WITHOUT WRITTEN APPROVAL OF

4. WORK SHALL BE SUBJECT TO INSPECTION BY THE COUNTY INSPECTORS AND DESIGN ENGINEER. SANITARY SEWER CUT SHEETS SHALL BE SUBMITTED TO THE BOTETOURT COUNTY ENGINEER

5. CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND UNCOVERING ALL VALVE BOXES AFTER SURFACE TREATMENT OF ROADS AND ADJUSTING BOXES TO FINAL ROAD GRADES, IF

6 ALL EXISTING UTILITIES MAY BE SHOWN OR MAY NOT BE SHOWN IN THE EXACT LOCATION. THE CONTRACTOR SHALL COMPLY WITH THE STATE WATER WORKS REGULATIONS, SECTION 12 05.03 WHERE

THE CONTRACTOR SHALL NOTIFY THE COUNTY AND OBTAIN COUNTY APPROVAL OF ANY FIELD CORRECTION TO THE APPROVED PLANS PRIOR

3 ALL TRENCHES WITHIN THE EXISTING OR FUTURE VIRGINIA STATE DEPARTMENT OF HIGHWAYS AND TRANSPORTATION RIGHT-OF-WAY MUST BE COMPACTED IN SIX INCH LAYERS.

9 ALL LINES TO BE STAKED PRIOR TO CONSTRUCTION

10 CONTRACTOR TO COORDINATE WITH THE ENGINEER TO PROVIDE AS-BUILT PLANS CONTRACTOR SHALL MAINTAIN A SET OF RED-LINE PLANS SHOWING AS-BUILT LOCATION OF ALL STRUCTURES. AS-BUILT INFORMATION TO BE SUBMITTED TO DESIGN ENGINEER FOR PREPARATION OF RECORD AS-BUILT PLANS SUCH AS-BUILT PLANS SHALL BE SUBMITTED TO BOTETOURT COUNTY PRIOR TO COUNTY ACCEPTANCE

11 ALL CONSTRUCTION SHALL BE IN ACCORDANCE TO APPROVED CONSTRUCTION PRACTICES OF THE APPLICABLE TRADES.

12. UNLESS NOTED OTHERWISE HEREIN ALL CONSTRUCTION SHALL BE IN ACCORDANCE TO THE LATEST EDITION OF AWWA STANDARDS EXCAVATION, STABILIZATION AND BEDDING

EXCAVATION FOR TRENCHES SHALL INCLUDE THE REMOVAL OF ALL MATERIAL ENCOUNTERED REGARDLESS OF CLASSIFICATION IN ACCORDANCE WITH THE ELEVATIONS AND GRADES AT THE LOCATIONS AND STATIONS INDICATED ON THE PLANS OR

EXCAVATION, UNLESS OTHERWISE SPECIFIED, SHALL BE OPEN CUT. THE CONTRACTOR SHALL OPEN NO MORE THAN TWO HUNDRED (200) FEET OF TRENCH AT ONE TIME DURING THE LAYING OF PIPE, UNLESS APPROVED BY THE ENGINEER.

TRENCHES SHALL BE EXCAVATED IN STRAIGHT LINES AND SHALL BE ACCURATELY GRADED IN ORDER TO ESTABLISH A TRUE ELEVATION FOR THE INVERT OF THE PIPE

4 THE WIDTH OF TRENCHES, FROM EXISTING GRADE TO ONE (1) FOOT ABOVE THE TOP OF THE PIPE SHALL BE OF SUFFICIENT WIDTH TO PERMIT THE PROPER INSTALLATION OF BRACING. SHORING OR SHEETING

5 THE SIDES OF THE TRENCHES SHALL BE AS VERTICAL AS

6 EXCAVATION FOR STRUCTURES SHALL ALLOW A MINIMUM OF TWELVE (12) INCHES CLEAR BETWEEN THE STRUCTURE AND THE SIDES OF THE TRENCH OR ANY REQUIRED BRACING, SHORING OR SHEETING

EXCAVATED MATERIALS SUITABLE FOR BACKFILL SHALL BE STOCKPILED IN AN ORDERLY MANNER AT A SUFFICIENT DISTANCE FROM THE SIDES OF THE TRENCH IN ORDER TO AVOID OVERLOADING THE BANKS OF THE TRENCH AND TO PREVENT SLIDES

8 EXCAVATED MATERIALS WHICH ARE NOT REQUIRED OR APPROVED FOR BACKFILL SHALL BE REMOVED FROM THE SITE AND DISPOSED OF BY THE CONTRACTOR, AT HIS EXPENSE.

9 CONTRACTOR TO ADHERE TO ALL LOCAL STATE AND FEDERAL CONSTRUCTION LAWS, INCLUDING OSHA TRENCH SAFETY REGULATIONS.

B TRENCH STABILIZATION

TRENCH STABILIZATION MATERIAL SHALL BE COARSE AGGREGATE SIZE NUMBER 2 AND SHALL CONFORM WITH VDOT SECTION 203

WHENEVER EXCESSIVELY WET OR UNSTABLE MATERIAL IS ENCOUNTERED IN THE BOTTOM OF THE TRENCH, WHICH IN THE OPINION OF THE ENGINEER IS INCAPABLE OF PROPERLY SUPPORTING THE PIPE OR STRUCTURES, SUCH MATERIAL SHALL BE REMOVED AND BACKFILLED WITH TRENCH STABILIZATION MATERIAL AND SHALL BE GRADED TO ALLOW FOR THE COMPACTED BEDDING

ALL UNAUTHORIZED OVERDEPTHS OF EXCAVATION SHALL BE BACKFILLED, AT THE CONTRACTOR'S EXPENSE, WITH TRENCH STABILIZATION MATERIAL AND SHALL BE GRADED TO ALLOW FOR THE COMPACTED BEDDING MATERIAL.

C. COMPACTED BEDDING MATERIAL

BEDDING MATERIAL SHALL BE COARSE AGGREGATE SIZE NUMBER 57 AND SHALL CONFORM WITH VDOT SECTION 203 AND/OR ASTM C 33.

THE BOTTOM OF THE PIPE TRENCH SHALL BE EXCAVATED TO A MINIMUM OVERDEPTH OF SIX (6) INCHES BELOW THE BOTTOM OF

THE PIPE, TO PROVIDE FOR THE COMPACTED BEDDING MATERIAL.

BEDDING MATERIAL SHALL BE PLACED, SHAPED AND COMPACTED

BELL HOLES AND DEPRESSIONS REQUIRED FOR THE JOINTING OF THE PIPE SHALL BE DUG AFTER THE COMPACTED BEDDING MATERIAL HAS BEEN GRADED AND SHAPED AND SHALL BE ONLY OF THE LENGTH, DEPTH AND WIDTH REQUIRED TO MAKE THE JOINT

PIPE, JOINTS AND FITTINGS

A. SCOPE OF WORK

ALL MATERIALS AND APPURTENANCES REQUIRED FOR THE WORK SHALL BE NEW, OR FIRST CLASS QUALITY AND SHALL BE FURNISHED, DELIVERED, ERECTED CONNECTED AND FINISHED IN EVERY DETAIL AS SPECIFIED OR INDICATED ALL MATERIALS FOUND DEFECTIVE, REGARDLESS OF THE CIRCUMSTANCES, SHALL BE REPLACED WITH NEW MATERIAL AT THE EXPENSE OF THE

THE MATERIALS SPECIFIED FOR THE CONSTRUCTION SHALL COMPLY WITH THE LATEST REVISIONS OF THE APPLICABLE AMERICAN SOCIETY FOR TESTING MATERIALS (ASTM), AMERICAN NATIONAL STANDARDS INSTITUTE (ANSI) AND/OR THE VIRGINIA DEPARTMENT OF TRANSPORTATION (VDOT) STANDARDS

THE CONTRACTOR SHALL INSTALL ONLY ONE (1) TYPE OF PIPE BETWEEN STRUCTURES EXCEPT WHERE DUCTILE IRON PIPE IS SPECIFIED OR INDICATED. WHERE EXISTING PIPE IS TO BE REPLACED OR EXTENDED THE SAME TYPE OF PIPE SHALL BE INSTALLED, UNLESS SPECIFIED OR INDICATED OTHERWISE

3. SANITARY SEWERS WITH AN INSIDE DIAMETER LESS THAN OR EQUAL TO TWELVE (12) INCHES SHALL BE EITHER POLYVINYL CHLORIDE OR DUCTILE IRON PIPE, AT THE CONTRACTOR'S OPTION, UNLESS SPECIFIED OR INDICATED OTHERWISE. CONTRACTOR SHALL OBTAIN APPROVAL OF PIPE MATERIAL BY BOTETOURT COUNTY ENGINEER PRIOR TO BEGINNING CONSTRUCTION.

TYPES OF PIPE

1. TOLTHING SHAPIDE (PVC) WATER PIPE SHAPE
18 MINIMUM, UNLESS SPECIFIED OR INDICATED C 2. DUCTILE IRON PIPE SHALL CONFORM WITH AWWA C 151/ANSI 21.51 AND FITTINGS SHALL CONFORM WITH AWWA C 110/ANS 21.10 THE PIPE AND FITTINGS SHALL BE BITUMINOUS COATED

21.40. THE PIPE THICKNESS SHALL CONFORM WITH AWWA C 150/ANSI 21.50 AND SHALL BE CLASS 50. AS A MINIMUM, UNLESS SPECIFIED OR INDICATED OTHERWISE. 3 PVC SEWER PIPE AND FITTINGS SHALL BE SDR 35 (ASTM D

AND CEMENT LINED IN ACCORDANCE WITH AWWA C 104/ANSI

D JOINTS COUPLINGS AND APPURTENANCES PVC PIPE AND FITTINGS SHALL BE BELL AND SPIGOT TYPE JOINTS. THE BELL AND SPIGOT JOINT SHALL BE SEALED WITH ELASTOMERIC GASKETS CONFORMING TO ASTM D 3212 THE JOINTS SHALL BE MADE IN STRICT ACCORDANCE WITH THE

IRON PIPE AND FITTINGS SHALL BE EITHER MECH OT TYPE JOINTS AS SPECIFIED SING WATERTIGHT RUBBER WITH AWWA C 111/ANSI L BE MADE IN '

RECOMMENDATION OF THE PIPE MANUFACTURER.

3 GATE VALVES SHALL BE IRON-BODY, BRONZE-MOUNTED, DOUBLE-DISC, PARALLEL-SEAL, O-RING SEALED, INSIDE-SCREW, NON-RISING STEM, FITTING WITH 2 INCH SQUARE OPERATING NUT FOR VALVE VAULT SERVICE, ALL IN ACCORDANCE WITH AVVA STANDARD COOD (LATEST REVISION) CONNECTIONS SHALL BE SUITABLE FOR THE PIPE WITH WHICH IT IS USED. THE VALVES SHALL BE SHITARLE FOR 200 P.S.L. WATER WORKING PRESSURE AND SHALL BE TESTED AT TWICE THE RATED WORKING PRESSURE ALL GATE VALVES SHALL BE INSTALLED IN VALVE VAULTS AND EQUIPPED WITH A 2-INCH SQUARE OPERATING NUT. THE NUT SHALL BE MARKED WITH AN ARROW AND THE WORD "OPEN" AND SHALL OPEN BY TURNING TO THE RIGHT (CLOCKWISE).

4 ALL OTHER MATERIALS AND APPURTENCES TO BE IN ACCORDANCE WITH DETAILS SHOWN ON PLANS PIPE INSTALLATION

1 THE CONTRACTOR SHALL NOT LAY PIPE OR PLACE MANHOLES UNTIL ALL WATER HAS BEEN REMOVED FROM THE TRENCH, OR WHEN IN THE OPINION OF THE ENGINEER, THE TRENCH OR THE WEATHER CONDITIONS ARE UNSUITABLE FOR WORK.

PIPE THAT MAY REQUIRE FIELD CUTTING SHALL BE DONE SO IN A NEAT AND WORKMANLIKE MANNER, SO AS TO LEAVE A SMOTTH END AT RIGHT ANGLES TO THE AXIS OF THE PIPE. CARE SHALL BE TAKEN TO AVOID DAMAGING THE PIPE AND ANY COATINGS OR LININGS. DUCTILE IRON PIPE SHALL NOT BE CUT WITH AN

3 THE MATERIALS SHALL BE VISUALLY INSPECTED FOR DEFECTS BEFORE LOWERING THE PIPE OR PLACING THE MANHOLES INTO THE TRENCH. DURING THE LAYING OPERATION NO TOOLS, CLOTHING O OTHER MATERIAL CHALL RE DIACED IN THE PIPE OR MANHOLE THE INTERIOR OF THE PIPE SHALL BE CLEAR OF ALL SOIL. DEBRIS AND SUPERFLUOUS MATERIALS PRIOR TO AND DURING THE INSTALLATION.

4 THE CONTRACTOR SHALL EXERCISE EVERY PRECAUTION TO PREVENT FOREIGN MATERIAL FROM ENTERING THE PIPE WHILE IT IS BEING PLACED IN THE TRENCH. FAILURE BY THE CONTRACTOR TO TAKE SUCH PRECAUTIONS MAY RESULT IN THE ENGINEER REQUIRING A HEAVY, TIGHTLY WOVEN CANVAS BAG OF SUITABLE SIZE BE PLACED OVER EACH END OF THE PIPE AND REMOVED ONLY WHEN THE JOINT CAN BE MADE PROPERLY

MATERIALS BE DROPPED OR DUMPED INTO THE TRENCHES 6 WHEN WORK IS NOT IN PROGRESS. THE CONTRACTOR SHALL PLUG THE OPEN ENDS OF THE PIPE TO PREVENT TRENCH WATER OR OTHER SUBSTANCES FROM ENTERING THE PIPE. THE PLUG SHALL BE WATERTIGHT AND SHALL REMAIN IN PLACE UNTIL ANY

THE PIPE AND MANHOLES SHALL BE LOWERED CAREFULLY INTO THE

TIMES TO AVOID DAMAGE. UNDER NO CIRCUMSTANCES SHALL THE

TRENCH BY SUITABLE MEANS AND HANDLED WITH CARE AT ALL

REQUIRED DEWATERING HAS BEEN COMPLETED. 7. PARALLEL INSTALLATION - WATER LINES SHALL BE LAID AT LEAST TEN FEET HORIZONTALLY FROM A SEWER OR SEWER MANHOLE WHENEVER POSSIBLE. WHEN LOCAL CONDITIONS PREVENT A HORIZONTAL SEPARATION OF TEN FEET, THE WATER LINE MAY BE

LAID CLOSER TO A SEWER OR SEWER MANHOLE PROVIDED THAT. THE INVERT OF THE WATER MAIN SHALL BE AT LEAST 18 INCHES ABOVE THE CROWN OF THE SEWER.

H WHERE THIS VERTICAL SEPARATION CANNOT BE OBTAINED. THE SEWER SHALL BE CONSTRUCTED OF AWWA APPROVED WATER PIPE, PRESSURE TESTED IN PLACE WITHOUT

III. THE SEWER MANHOLE SHALL BE OF WATER-TIGHT CONSTRUCTIN AND TESTED IN PLACE.

CROSSING - WATER LINES CROSSING SEWERS SHALL BE LAID TO PROVIDE A SEPARATION OF AT LEAST 18 INCHES BETWEEN THE BOTTOM OF THE WATER LINE AND THE TOP OF THE SEWER WHENEVER POSSIBLE. WHEN LOCAL CONDITIONS PREVENT THIS VERTICAL SEPARATION, THE FOLLOWING CONSTRUCTION SHALL BE

LEAKAGE PRIOR TO BACKFILLING

SEWERS PASSING OVER OR UNDER WATER LINES SHALL BE CONSTRUCTED OF AWWA APPROVED WATER PIPE, PRESSURE TESTED IN PLACE WITHOUT LEAKAGE PRIOR TO -RACKFILLING

WATER LINES PASSING UNDER SEWERS SHALL, IN ADDITION, BE PROTECTED BY PROVIDING.

(A) A VERTICAL SEPARATION OF AT LEAST 18 INCHES BETWEEN THE BOTTOM OF THE SEWER AND THE TOP OF THE WATER LINE.

(B) ADEQUATE STRUCTURAL SUPPORT FOR THE SEWERS TO PREVENT EXCESSIVE DEFLECTION OF THE JOINTS AND THE SETTLING ON AND BREAKING OF THE WATERLINE.

(C) THAT THE LENGTH OF THE WATER LINE BE CENTERED AT THE POINT OF THE CROSSING SO THAT JOINTS SHALL BE EQUAL DISTANCE AND AS FAR AS POSSIBLE FROM THE SEWER

NO WATER PIPES SHALL PASS THROUGH OR COME IN CONTACT WITH ANY PART OF A SANITARY SEWER MANHOLE ALL SANITARY SEWER MANHOLES SHALL BE PLACED A MINIMUM OF 10 FEET HORIZONTALLY FROM ALL WATER MAINS WHENEVER POSSIBLE. WHEN THIS HORIZONTAL SEPERATION CAN NOT BE MAINTAINED, THE MANHOLE SHALL BE OF WATERTIGHT CONSTRUCTION AND TESTED IN PLACE.

8 BEFORE JOINTS ARE MADE THE PIPE SHALL BE WELL BEDDED ON A FIRM FOUNDATION AND NO PIPE SHALL BE BROUGHT INTO POSITION UNTIL THE PRECEDING LENGTH HAS BEEN THOROUGHL' FMREDDED AND SECURED IN PLACE. ANY DEFECTS DUE TO SETTLEMENT SHALL BE MADE GOOD BY THE CONTRACTOR AT HIS EXPENSE. BELL HOLES SHALL BE DUG SUFFICIENTLY LARGE TO INSURE THE MAKING OF PROPER JOINTS.

9 PIPE SHALL BE JOINTED IN FULL ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. PUSH-ON JOINTS SHALL BE THOUROUGHLY CLEANED, THE RUBBER GASKET INSERTED IN THE BELL SOCKET, A THIN FILM OF APPROVED GASKET LUBRICANT APPLIED, THE SPIGOT END OF THE PIPE CENTERED INTO THE SOCKET AND THE JOINT COMPLETED BY FORCING THE SPIGOT ENG TO THE BOTTOM OF THE SOCKET BY A JACK-TYPE TOOL OR OTHER DEVICE APPROVED BY THE ENGINEER. MECHANICAL JOINTS SHALL THOROUGHLY CLEANED, THE GLAND SPLIPPED OVER THE SPIGOT END OF THE PIPE. THE RUBBER GASKET PAINTED WITH SOAP SOLUTION AND PLACED ON THE SPIGOT END, THE SPIGOT END OF THE PIPE SEATED IN THE BELL, THE GASKET PRESSED INTO PLACE WITHIN THE BELL. THE GASKET PRESSED INTO PLACE WITHIN THE BELL. THE GLAND MOVED INTO POSITION, AND BOLTS AND NUTS ASSEMBLED BY HAND AND TIGHTENED WITH AN APPROVED TORQUE-LIMITING WRENCH.

THE WATER MAIN SHALL BE LAID AND MAINTAINED AT THE REQUIRED LINES AND GRADES WITH FITTINGS AND VALVES AT THE

REQUIRED LOCATIONS. DEFLECTION OF THE LINE OF PIPE, IN EITHER, THE VERTICAL OR HORIZONTAL PLANE TO AVOID OBSTRUCTIONS, OR IN LOCATIONS WHERE LONG-RADIUS CURVES ARE REQUIRED. THE AMOUNT OF DEFLECTION SHALL NOT EXCEED APPROVED AWWA STANDARDS. ALIGNMENT THAT MAY REQUIRE FEFLECTIONS IN FXCESS OF THE RECOMMENDED LIMITATIONS, SPECIAL BENDS, OR A SUFFICIENT NUMBER OF SHORTER LENGTHS OF PIPE TO PROVIDE THE ANGULAR DEFLECTIONS WITHIN THE LIMITS AS SET FORTH, SHALL BE APPROVED BY THE ; ENGINEER.

ALL PLUGS, EXCEPT MECHANICAL JOINT PLUGS AT CONNECTIONS FOR FUTURE LINES, ALL TEES, AND ALL BENDS IN WATER MAINS LINDER PRESSURE SHALL BE PROVIDED WITH REACTION BACKING CONSISTING OF CONCRETE THRUST BLOCKS. VALVES FOR CONNECTIONS TO FUTURE LINES AND FIRE HYDRANTS SHALL BE ANCHORED TO THE WATER MAIN WITH THE RODS.

4 DETECTION TAPE TO BE INSTALLED 12"-18" ABOVE ALL NEW PVC WATER LINES.

DISINFECTION OF WATER MAINS ALL PIPE SHALL BE DISINFECTED. TESTED AND FLUSHED IN

ACCORDANCE WITH AWWA STANDARD C601 (LATES REVISION) CONTRACTOR SHALL PROVIDE ALL MATERIALS, EQUIPMENT, NECESSARY TAPS AND PERFORM ALL WORK REQUIRED FOR THE

STERILIZATION, TESTING AND FLUSHING OF THE WATER MAIN.

3. NO TESTED SECTION OF WATER LINE SHALL BE APPROVED TO DELIVER WATER SERVICE UNTIL A FAVORABLE LABORATORY REPORT HAS BEEN ACHIEVED. ANY TESTED SECTION OF WATER LINE FAILING TO MEET THE REQUIREMENTS SPECIFIED SHALL BE REPOIRED BY THE CONTRACTOR AND RETESTED UNTIL THE RESULTS ARE WITHIN THE LIMITS SPECIFIED.

4 THE WATER MAIN OR VALVED OFF SECTION THAT HAS BEEN COMPLETED SHALL BE FILLED, TESTED AND FLUSHED. TEST LOCATIONS SHALL BE SUBJECT TO THE DISCRETION OF THE ENGINEER AND AS VALVES AND BLOW-OFFS PERMIT.

5 AFTER TESTING AND BEFORE FINAL INSPECTION OF THE COMPLETED SYSTEMS, WATER MAINS AND SERVICE LATERALS SHALL BE FLUSHED AND DISINFECTED IN ACCORDANCE WITH AWWA SPECIFICATIONS C601 (LATEST REVISION). FLUSHING SHALL BE ACCOMPLISHED AT A FLOW VELOCITY OF NOT LESS THAT 2.5 FEET

DISINFECTION AS DESCRIBED IN AWWA C651 - "PLACING OF CALCIUM HYPOCHLORITE TABLETS" SHALL BE USED. 5 GRAM CALCIUM HYPOCHLORITE TABLETS WITH 3.25 GRAM AVAILABLE. CHLORINE PER TABLET SHALL BE ATTACHED AT THE INSIDE TO OF THE PIPE BY AN ADHESIVE SUCH AS PERMATEX NO. 1 OR FOLIAL THE FOLLOWING NUMBER OF TABLETS FOR THE GIVEN PIPE SIZE SHALL BE USED FOR AN INITIAL DOSE OF 25 MG/1 (PPM) CHLORINE:

NUMBER TABLETS PER 18-20 FT PIPE SECTION PIPE DIAMETER

OR THE NUMBER OF TABLETS EQUAL TO 0.001202L ROUNDED TO THE NEXT HIGHER INTEGER, WHERE D IS THE INSIDE DIAMETER. IN INCHES AND L IS THE LENGTH OF THE PIPE SECTION, IN FEET USE OF THE CONTINUOUS FEED OR SLUG METHOD OF DISINFECTING MAY ONLY BE USED TO RE-CHLORINATE A WATER PIPE AFTER THE INITIAL DISINFECTION OR IN OTHER SPECIFIC CASES APPROVED BY THE DESIGN ENGINEER. WHEN FILLING THE PIPELINE FOR DISINFECTION, THE RATE OF FILLING MUST RESULT IN A VELOCITY OF LESS THAN 1 FT./SEC.

THE DISINFECTION SOLUTION SHALL REMAIN IN THE PIPE LINE FOR NOT LESS THAN TWENTY-FOUR (24) HOURS, AFTER WHICH TIME A CHLORINE RESIDUAL OF 10 PPM AT ALL PARTS OF THE LINE SHALL BE REQUIRED.

FOLLOWING CHLORINATION, THE PIPING SHALL BE THOROUGHLY FILISHED THE VIRGINIA WATERWORKS REGULATIONS REQUIRE AT LEAST TWO CONSECUTIVE SATISFACTORY BACTERIOLOGICAL SAMPLES AT 24 HOUR INTERVALS FROM THE DISTRIBUTION SYSTEM AT MAXIMUM SPACING OF 2000 FEET BEFORE THE SYSTEM CAN BE PLACED IN SERVICE. IF THE INITIAL TESTING IS NOT SATISFACTORY THE NEW LINES WILL BE RETESTED UNTIL SATISFACTORY RESULTS ARE ACHIEVED. THE CONTRACTOR SHALL PAY ALL COSTS ASSOCIATED WITH DISINFECTION AND TESTING OF INSTALLED FACILITIES INCLUDING ANY BACTERIOLOGICAL SAMPLES AND RETESTING IF REQUIRED. SAMPLES WILL BE COLLECTED IN ACCORDANCE WITH THE VIRGINIA WATERWORKS

D INSTALLING SEWER PIPE & MANHOLES

REGULATIONS.

THE INSTALLATION OF THE SANITARY SEWER SYSTEM SHALL BEGIN AT THE DOWNSTREAM MANHOLE AND PROCEED UPSTREAM. THE DOWNSTREAM SECTIONS SHALL BE COMPLETED, TESTED AND APPROVED PRIOR TO ALLOWING SANITARY SEWAGE TO ENTER THE SYSTEM.

THE PIPE SHALL BE INSTALLED IN ACCORDANCE WITH THE PIPE MANUFACTURER'S RECOMMENDATIONS AND AS DIRECTED BY THE ENGINEER. THE PIPE SHALL BE LAID IN TRUE STRAIGHT LINES WITH THE RELL ENDS UPSTREAM AND WITH THE INVERT OF THE PIPE BEING THE TRUE ELEVATION AND GRADE OF THE SYSTEM.

3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING AND MAINTAINING THE HORIZONTAL ALIGNMENT AND VERTICAL ELEVATION AND GRADE OF THE SYSTEM IN ACCORDANCE WITH THE SURVEY INFORMATION INDICATED ON THE PLANS.

4. THE HORIZONTAL ALIGNMENT OF THE PIPE SHALL BE MAINTAINED BY A TRANSIT OR THEODOLITE PLUMBED OVER THE CENTER OF THE DOWNSTREAM MANHOLE. THE VERTICAL ELEVATION AND GRADE SHALL RE MAINTAINED BY NOT LESS THAN THREE (3) BATTER BOARDS PLACED BETWEEN MANHOLES OR BY AN ADJUSTABLE LASER LEVEL MOUNTED AT THE INVERT OF THE DOWNSTREAM MANHOLE WITH TARGET(S) PLACED IN THE BELL END OF THE PIPE BEING

5. SEWER PIPE SHALL BE INSTALLED IN 4 INCH GRAVEL BEDDING EXTENDING TO THE SPRINGLINE OF PIPE AND IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.

6. THE SANITARY SEWER SYSTEM SHALL BE LAID AND JOINED COMPLETE-IN-PLACE SO THAT EACH LENGTH AND SECTION OF PIPE BETWEEN THE MANHOLES SHALL HAVE A SMOOTH AND UNIFORM

7 THE PIPE SHALL BE CONNECTED TO MANHOLES THROUGH PRECAST OPENINGS AND JOINED WITH EITHER A FLEXIBLE BOOT ADAPTER OR A PIPE SEAL GASKET.

8. DETECTION TAPE TO BE INSTALLED 12" TO 18" ABOVE ALL NEW

SEWER PIPE MAINS AND SEWER SERVICE LATERALS.

E CONNECTION TO EXISTING SYSTEMS 1 THE NEW PIPE CONNECTION TO BE MADE TO AN EXISTING MANHOLE WHERE NO STUB OR OPENING EXISTS, SHALL BE MADE THROUGH AN OPENING OF MAXIMUM DIAMETER CUT INTO THE MANHOLE WALL AT THE REQUIRED LOCATION AND ELEVATION

THE EXISTING INVERT CHANNELS AND BENCHES SHALL BE REWORKED AS REQUIRED TO FORM A NEW FLOW CHANNEL FROM THE NEW CONNECTION TO THE EXISTING FLOW CHANNEL

3 THE NEW PIPE CONNECTED TO AN EXISTING MANHOLE SHALL BE SECURED IN POSITION AND THE REMAINING OPENING SHALL BE FILLED AND SEALED WITH BRICK AND MORTAR. THE OUTER SURFACE OF THE CONNECTION SHALL BE GIVEN A COAT OF HEAVY BITUMASTIC WATERPROOFING COMPOUND.

F SERVICE CONNECTIONS

1 THE CONTRACTOR SHALL MAKE ALL SERVICE CONNECTIONS TO THE SEWER PIPE AND FROM MANHOLES WHERE SHOWN ON THE PLANS AND/OR WHERE LOCATED IN THE FIELD. THE SERVICE CONNECTIONS TO THE SEWER PIPE SHALL BE MADE WITH A WYE OR TEE WYE BRANCH FITTING

2. THE WYE AND TEE WYE BRANCH FITTINGS FOR SERVICE CONNECTIONS SHALL BE COMMERCIALLY MANUFACTURED AND INSTALLED IN STRICT ACCORDANCE WITH THE RECOMMENDATIONS OF THE PIPE MANUFACTURER.

3 THE SEWER PIPE SHALL NOT BE CUT OR TAPPED FOR SERVICE CONNECTIONS EXCEPT WHEN AND WHERE PERMITTED BY THE

4 ALL SERVICE CONNECTIONS SHALL BE MADE WITH FOUR (4) INCH PIPE AS A MINIMUM, UNLESS THE SIZE OF AN EXISTING SERVICE CONNECTION DICTATES OTHERWISE, AND SHALL BE INSTALLED ON A MINIMUM GRADE OF ONE-QUARTER (1/4) INCH PER ONE (1 FOOT FROM THE SEWER PIPE OR MANHOLE TO THE PROPERTY OR EASEMENT LINE

5 FUTURE SERVICE CONNECTIONS SHALL EXTEND TO THE PROPERTY OR EASEMENT LINE WITH CLEANOUT AND BE PROPERLY CAPPED WITH A WATERTIGHT FITTING TO PREVENT INFILTRATION INTO THE SEWERAGE SYSTEM. THE FITTING SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE RECOMMENDATIONS OF THE PIPE MANUFACTURER

6. FUTURE SERVICE CONNECTIONS SHALL BE FIELD MARKED BY A TREATED, SOLID WOODEN (2 X 4) MARKER THREE (3) FEET LONG SET VERTICALLY PLUMB WITH THE END OF THE CAPPED EXTENSION. THE TOPS OF THE MARKERS SHALL BE PAINTED YELLOW AND SET 24" ABOVE THE FINISHED GRADE. THE LOCATION AND INVERT DEPTH OF THE SERVICE CONNECTION SHALL RE SHOWN ON THE AS-BUILT PLANS.

BACKFILLING

A JOB CONDITIONS

1 PRIOR TO PLACING BACKFILL, ALL ORGANIC, RUBBISH DEBRIS OR OTHER UNSUITABLE OR OBJECTIONABLE MATERIAL WITHIN THE TRENCH SHALL BE REMOVED. ALL CONCRETE FORMS SHALL BE REMOVED. ALL SHORING OR SHEETING SHALL BE REMOVED OR CUT OFF AT THE DEPTH STIPULATED BY THE ENGINEER.

PRIOR TO PLACING BACKFILL, THE TRENCH BOX SHALL BE REMOVED ALL CONCRETE FORMS SHALL BE REMOVED. ALL SHORING OR SHEETING SHALL BE REMOVED OR CUT OFF AT THE DEPTH STIPULATED BY THE ENGINEER. 3 BACKFILL MATERIAL SHALL BE PLACED IN UNIFORM HORIZONTAL

HAND OPERATED TAMPERS OR OTHER EQUIPMENT AS APPROVED BY THE ENGINEER TO PERFORM SUCH WORK. 4 BACKFILL MATERIAL SHALL BE PLACED AND COMPACTED SO AS TO NOT UNEVENLY SUPPORT, DAMAGE OR DISPLACE THE ALIGNMENT OF

LAYERS AND THOROUGHLY COMPACTED WITH PROPER MECHANICAL OR

THE PIPE OR STRUCTURES. 5. BACKFILL SHALL NOT BE PLACED OR COMPACTED AGAINST CAST IN PLACE CONCRETE UNTIL IT HAS OBTAINED SUFFICIENT STRENGTH TO WITHSTAND THE BACKFILLED PRESSURE PLACED UPON IT.

6 UPON THE COMPLETION OF BACKFILLING, ALL EXCESS SOIL. STONES AND DEBRIS SHALL BE REMOVED FROM THE SITE AND DISPOSED OF BY THE CONTRACTOR.

B. BACKFILL MATERIAL

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MATERIALS FOR BACKFILL SHALL BE APPROVED EXCAVATED MATERIAL OR APPROVED SUITABLE MATERIAL OBTAINED FROM OTHER SOURCES. ALL MATERIAL SHALL BE APPROVED BY A SOILS

2. MATERIAL SHALL CONSIST OF DURABLE NATURAL GRANULAR MATERIAL OR GRANULAR AGGREGATES FREE OF ORGANIC MATERIAL. LOAM, DEBRIS, OR OTHER OBJECTIONABLE MATERIAL WHICH CANNOT BE THOROUGHLY COMPACTED.

3. MATERIAL SHALL NOT CONTAIN STONES LARGER IN DIAMETER THAN THOSE SPECIFIED HEREIN, GRANITE, BROKEN CONCRETE, MASONRY RUBBLE OR OTHER MATERIAL WHICH IN THE OPINION OF THE ENGINEER IS UNSUITABLE FOR BACKFILL.

4 EXCESSIVELY WET EXCAVATED MATERIAL SHALL NOT BE USED AS RACKFILL FROZEN MATERIAL SHALL NOT BE PLACED IN THE TRENCH, NOR SHALL APPROVED BACKFILL BE PLACED UPON FROZEN MATERIAL HOWEVER, BACKFILLING MAY BE ALLOWED IN FREEZING WEATHER WITH PRIOR APPROVAL OF THE ENGINEER

BACKFILL BELOW UNPAVED AREAS

1. BACKFILL FROM THE TOP OF THE PIPE BEDDING OR BOTTOM OF THE PIPE TRENCH TO ONE (1) FOOT ABOVE THE TOP OF THE PIPE SHALL BE FREE OF STONES LARGER THAN ONE (1) INCH IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED SIX (6) INCHES AND COMPACTED WITH HAND OPERATED TAMPERS.

BACKFILL FROM ONE (1) FOOT ABOVE THE TOP OF THE PIPE TO THE TOPSOIL SUBGRADE SHALL BE FREE OF STONES LARGER THAN FIVE (5) INCHES IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED TWELVE (12) INCHES AND COMPACTED WITH MECHANICAL TAMPERS

3. DRAINAGE CHANNELS TO BE CONSTRUCTED OF FILL MATERIAL SHALL BE GRADED AND SHAPED TO THE TOPSOIL SUBGRADE WITH MATERIAL FREE OF STONES LARGER THAN FOUR (4) INCHES IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED EIGHT (8) INCHES AND COMPACTED WITH MECHANICAL TAMPERS

D BACKFILL BELOW EXISTING OR NEW PAVED AREAS AND SIDEWALKS

1. BACKFILL FROM THE TOP OF THE PIPE BEDDING OR BOTTOM OF THE PIPE TRENCH TO ONE (1) FOOT ABOVE THE TOP OF THE PIPE SHALL BE FREE OF STONES LARGER THAN ONE (1) INCH IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED SIX (6) INCHES AND COMPACTED WITH HAND TAMPERS BACKFILL FROM ONE (1) FOOT ABOVE THE TOP OF THE PIPE TO THE PAVEMENT SUBGRADE SHALL BE FREE OF STONES LARGER THAN FOUR (4) INCHES IN DIAMETER AND SHALL BE PLACED IN LAYERS NOT TO EXCEED EIGHT (8) INCHES AND COMPACTED WITH MECHANICAL TAMPERS

INSPECTION AND TESTS

A TESTING OF SANITARY SEWER

THE CONTRACTOR SHALL PROVE THE WATERTIGHNESS OF THE SEWER SYSTEM OR PORTIONS THEREOF BY ONE OF THE FOLLOWING TESTS, AT SUCH TIMES AS THE ENGINEER MAY DIRECT. TESTS SHALL BE MADE ONLY IN THE PRESENCE OF THE ENGINEER. THE CONTRACTOR SHALL FURNISH ALL LABOR AND EQUIPMENT REQUIRED FOR THE TEST AND SHALL MAKE REPAIRS NECESSARY UNTIL TEST RESULTS ARE SATISFACTORY. BOTETOURT COUNTY ENGINEER SHALL BE NOTIFIED OF ALL TESTS 48 HOURS PRIOR TO CONDUCTING SUCH TESTS. ALL TESTS SHALL BE COORDINATED WITH THE DESIGN ENGINEER FOR HIS ATTENDANCE AND

B AIR TEST

THE TESTING EQUPMENT, PROCEDURE, AND RESULTS WILL ALL BE SUBJECT TO THE STRICT APPROVAL OF THE ENGINEER. RESULTS OF THE AIR TEST WILL BE REVIEWED FOR COMPLIANCE WITH ASTM DESIGNATION C-828, CURRENT REVISION. THE AIR TEST IS TO BE CONDUCTED BETWEEN TWO (2) CONSECUTIVE MANHOLES. THE TEST EQUIPMENT SHALL CONSIST OF TWO (2) PLUGS (ONE TAPPED AND EQUIPPED FOR AIR INLET CONNECTION), A SHUT-OFF VALVE, A PRESSURE REGULATING VALVE, A PRESSURE REDUCTION VALVE, AND A MONITORING PRESSURE GAUGE HAVING A PRESSURE RANGE FROM 0 TO 5 PSI, GRADUATED IN 0.10 PSI WITH AN ACCURACY OF PLUS/MINUS .04 PSI. THE TEST EQUIPMENT SHALL BE SET UP OUTSIDE THE MANHOLE FOR EASY ACCESS AND READING. AIR SHALL BE SUPPLIED TO THE TEST SLOWLY AND SHALL BE REGULATED TO PREVENT THE PRESSURE INSIDE THE PIPE FROM EXCEEDING 5.0 PSIG. THE PIPELINE SHALL BE FILLED UNTIL A CONSTANT INTERNAL PRESSURE OF 3.5 PSIG IS MAINTAINED. THE INTERNAL PRESSURE SHALL BE NAINTAINED AT 3.5 PSIG OR SLIGHTLY ABOVE FOR A FIVE (5) MINUTE STABILIZATION PERIOD, AFTER WHICH TIME THE INTERNAL PRESSURE WILL BE ADJUSTED TO 3.5 PSIG, THE AIR SUPPLY SHUT OFF AND THE TEST BEGUN. NO PERSON SHALL REMAIN IN THE MANHOLE WHILE PIPE IS BEING PRESSURIZED OR THROUGHOUT THE TEST FOR SAFETY PURPOSES A PRESSURE DROP OF 1.0 PSI FROM 3.5 TO 2.5 PSIG SHALL BE ALLOWED FOR THE TEST TIMES SPECIFIED IN THE FOLLOWING TABLE, BASED UPON THE DESIGNATED PIPE SIZE AND TEST SEGMENT LENGTH AIR TEST TABLE

BASED ON EQUATIONS FROM ASTM C-828-80 SPECIFICATIONS TIME (MIN: SEC) REQUIRED FOR PRESSURE DROP FROM 3.5 TO 2.5 PSI WHEN TESTING ONE PIPE DIAMETER ONLY

PIPE DIAMETER, INCHES SEGMENT 75 0.13 0:30 0:53 1:23 1:59 3:06 4:27 100 0-18 0.40 1 10 1-50 2 28 4:08 5:56 125 0:22 0:50 1 28 2 18 3:18 5 09 7.26 150 0:26 0 59 1:46 2.45 3:58 6:11 8:30 175 0:31 1:09 2:03 3:13 4:37 7:05 200 0.35 1:19 2.21 3.40 5.17 225 0:40 1 29 2.38 4 08 5 40 250 0:44 1:39 2:56 4:35 275 0.48 1.49 3.14 4.43 300 0.53 1.59 3.31 350 1.02 2.19 3.47 8:16 11:54 6: 03 9: 27 13: 36 450 1:19 2:50 6:48 10:38 15.19 5:14 7:34 11:49 17:01 500 1:28

SHOULD THE 1.0 PSI DROP OCCUR IN LESS TIME THAN THAT SPECIFIED IN THE TABLE, THE SEWER SEGMENT SHALL HAVE FAILED IF THE IME REQUIRED FOR THE PRESSURE TO DROP 1.0 PSI IS GREATER THAN THAT SHOWN IN THE TABLE, THE SEWER SEGMENT SHALL HAVE PASSED.

FOR A MORE DETAILED DESCRIPTION OF THE AIR TEST METHOD REFER

TO ASTM DESIGNATION C-828, CURRENT REVISION AN AIR PRESSURE CORRECTION SHALL BE REQUIRED WHEN THE PREVAILING GROUND WATER IS ABOVE THE SEWER LINE BEING TESTED. IT SHALL BE CALCULATED AS FOLLOWS: GROUND WATER DEPTH (FT) + 3.5 = STARTING TEST PRESSURE

ENDING TEST PRESSURE = STARTING PRESSURE - 1.0 PSI THERE IS NO CHANGE FROM TIME REQUIREMENTS ESTABLISHED FOR THE

MANHOLE VACUUM TESTS

MANHOLES SHALL BE TESTED BY VACUUM TEST, AFTER ASSEMBLY BUT PRIOR TO BACKFILLLING. TEST SHALL COMPLY WITH ASTM STANDARDS OR DIVISION OF WATER PROGRAMS WORKING MEMO #550, DATED NOVEMBER 4, 1987. MANHOLES SHALL BE TESTED BY VACUUM ONLY IF CONSTRUCTED OF PRECAST CONCRETE. TESTING SHALL INCLUDE ALL CONCRETE RISER, INCLUDING SPACER RINGS, AND THE JOINT BETWEEN THE CONCRETE AND THE 2 STUBOUTS, MANHOLE BOOTS AND PIPE PLUGS SHALL BE SECURED TO PREVENT MOVEMENT WHILE THE VACUUM IS

INSTALLATION AND OPERATION OF VACUUM EQUIPMENT AND INDICATING DEVICES SHALL BE IN ACCORDANCE WITH EQUIPMENT SPECIFICATIONS FOR WHICH PERFORMANCE INFORMATION HAS BEEN PROVIDED BY THE MANUFACTURER AND APPROVED BY THE VIRGINIA STATE DEPARTMENT OF

4. A MEASURABLE VACUUM OF 10 INCHES OF MERCURY SHALL BE ESTABLISHED IN THE MANHOLE. THE TIME FOR THE VACUUM TO DROP TO NINE INCHES OF MERCURY SHALL BE RECORDED. 5. ACCEPTANCE STANDARDS FOR LEAKAGE SHALL BE ESTABLISHED FROM THE ELAPSED TIME FOR A NEGATIVE PRESSURE CHANGE FROM 10 INCHES TO NINE INCHES OF MERCURY. THE MAXIMUM ALLOWABLE LEAKAGE RATE FOR FOUR-FOOT DIAMETER MANHOLE SHALL BE IN ACCORDANCE WITH THE FOLLOWING:

MANHOLE DEPTH 10 FT. OR LESS

> 10 FT. BUT < 15 FT. > 15 FT, BUT < 25 FT

FOR MANHOLES FIVE FEET IN DIAMETER, ADD AN ADDITIONAL 30 SECONDS TO THE TIME REQUIREMENTS FOR

FOUR-FOOT DIAMETER MANHOLES.

6 IF THE MANHOLE FAILS THE TEST, NECESSARY REPAIRS SHALL BE MADE, AND THE VACUUM TEST AND REPAIRS SHALL BE REPEATED UNTIL THE MANHOLE PASSES THE

IF A MANHOLE JOINT MASTIC IS COMPLETELY PULLED OUT DURING THE TEST, THE MANHOLE SHALL BE DISASSEMBLED AND THE MASTIC REPLACED

ALL SEWER LINES SHALL BE TESTED BY PULLING A STANDARD TEST MANDREL BETWEEN TEST SECTIONS

E TESTING OF WATER LINES 1 AFTER PLACING ALL HARNESSING AND ALL VALVE SUPPORT CONCRETE, SUFFICIENT BACKFILL SHALL BE PLACED PRIOR TO FILLING THE PIPE WITH WATER AND FIELD TESTING TO PREVENT LIFTING OF THE PIPE. WHEN LOCAL CONDITIONS REQUIRE THAT THE TRENCHES BE BACKFILLED IMMEDIATELY AFTER THE PIPE HAS BEEN LAID, THE TESTING SHALL BE CARRIED OUT AFTER BACKFILLING HAS BEEN COMPLETED BUT PRIOR TO PLACEMENT OF THE PERMANENT SURFACE. AT LEAST FOURTEEN (14) DAYS SHALL ELAPSE AFTER THE LAST VALVE SUPPORT OR HYDRANT BLOCK HAS BEEN CAST (TYPE I PORTLAND CEMENT) PRIOR TO TESTING

IN WHICH CASE THREE (3) DAYS SHALL ELAPSE ALL TESTING WILL BE PERFORMED IN ACCORDANCE WITH THE AWWA

UNLESS HIGH EARLY STRENGTH CONCRETE (TYPE III) IS USED.

C600-82 OR CURRENT REVISION. PRESSURE TEST: AFTER THE PIPE HAS BEEN LAID, ALL NEWLY LAID PIPE OR ANY VALVED SECTION THEREOF SHALL BE SUBJECTED TO A HYDROSTATIC PRESSURE OF AT LEAST 1.5 TIMES THE WORKING PRESSURE AT THE POINT OF TESTING. TEST PRESSURE RESTRICTIONS. TEST PRESSURES SHALL

A. NOT BE LESS THAN 1.50 TIMES THE WORKING PRESSURE AT HE HIGHEST POINT ALONG THE TEST SECTION

B. NOT EXCEED PIPE OR THRUST RESTRAINT DESIGN PRESSURES:

BE OF AT LEAST 2-HOUR DURATION: NOT VARY BY MORE THAN + 5 PS

F NOT EXCEED TWICE THE RATED PRESSURE OF THE VALVES OR HYDRANTS WHEN THE PRESSURE BOUNDARY OF THE TEST SECTION INCLUDES CLOSED GATE VALVES OR HYDRANTS. F NOT EXCEED THE RATED PRESSURE OF THE VALVE.

EACH VALVED SECTION OF PIPE SHALL BE FILLED WITH PROPERLY DISINFECTED WATER SLOWLY AND THE SPECIFIED TEST PRESSURE SHALL BE APPLIED BY MEANS OF A PUMP CONNECTED TO THE PIPE IN A MANNER SATISFACTORY TO THE ENGINEER

BEFORE APPLYING THE SPECIFIED TEST PRESSURE, AIR SHALL BE EXPELLED COMPLETELY FROM THE PIPE, VALVES, AND HYDRANTS.

ALL EXPOSED PIPE, FITTINGS. VALVES, HYDRANTS AND JOINTS SHALL BE EXAMINED CAREFULLY DURING THE TEST. ANY DAMAGED OR DEFECTIVE PIPE, FITTINGS, VALVES, OR HYDRANTS THAT ARE DISCOVERED FOLLOWING THE PRESSURE TEST SHALL BE REPEATED UNTIL IT IS SATISFACTORY TO THE ENGINEER

4 A LEAKAGE TEST SHALL BE CONDUCTED CONCURRENTLY WITH THE PRESSURE TEST LEAKAGE SHALL BE DEFINED AS THE QUANTITY OF WATER THAT MUST BE SUPPLIED INTO THE NEWLY LAID PIPE, OR ANY VALVED SECTION THEREOF, TO MAINTAIN PRESSURE WITHIN 5 PSI OF THE SPECIFIED TEST PRESSURE AFTER THE AIR IN THE PIPELINE HAS BEEN EXPELLED AND THE PIPE HAS BEEN FILLED WITH WATER. NO PIPE INSTALLATION WILL BE ACCEPTED IF THE LEAKAGE IS GREATER THAN THAT DETERMINED BY THE FOLLOWING FORMULA:

> L = S D V (P) 133,200

OF LEAKAGE

IN WHICH I IS THE ALLOWABLE LEAKAGE IN GALLONS PER HOUR: S IS THE LENGTH OF PIPELINE TESTED IN FEET: D IS THE NOMINAL DIAMETER OF THE PIPE, IN INCHES: AND P IS 1 AVERAGE TEST PRESSURE DURING THE LEAKAGE TEST. IN POUNDS PER SQUARE INCH GAUGE. WHEN TESTING AGAINST CLOSED METAL-SEATED VALVES. AN ADDITIONAL LEAKAGE PER ENCLOSED VALVE OF 0.0078 GAL/HR/IN. OF NOMINAL VALVE SIZE SHALL BE ALLOWED. WHEN HYDRANTS ARE IN THE TEST SECTION. THE TEST SHALL BE MADE ON THE BASIS OF ALLOWABLE LEAKAGE. IF ANY TEST OF PIPE LAID DISCLOSES LEAKAGE GREATER THAN THE ALLOWABLE AMOUNT, THE CONTRACTOR SHALL, AT HIS OWN EXPENSE, LOCATE AND REPAIR THE DEFECTIVE MATERIAL UNTIL THE LEAKAGE IS WITHIN THE SPECIFIED ALLOWANCE. ALL

MSIBLE LEAKS ARE TO BE REPAIRED REGARDLESS OF THE AMOUNT

MINIMUM ELAPSED TIME FOR A

60 SECONDS

75 SECONDS

90 SECONDS

PRESSURE CHANGE OF 1 INCH HG

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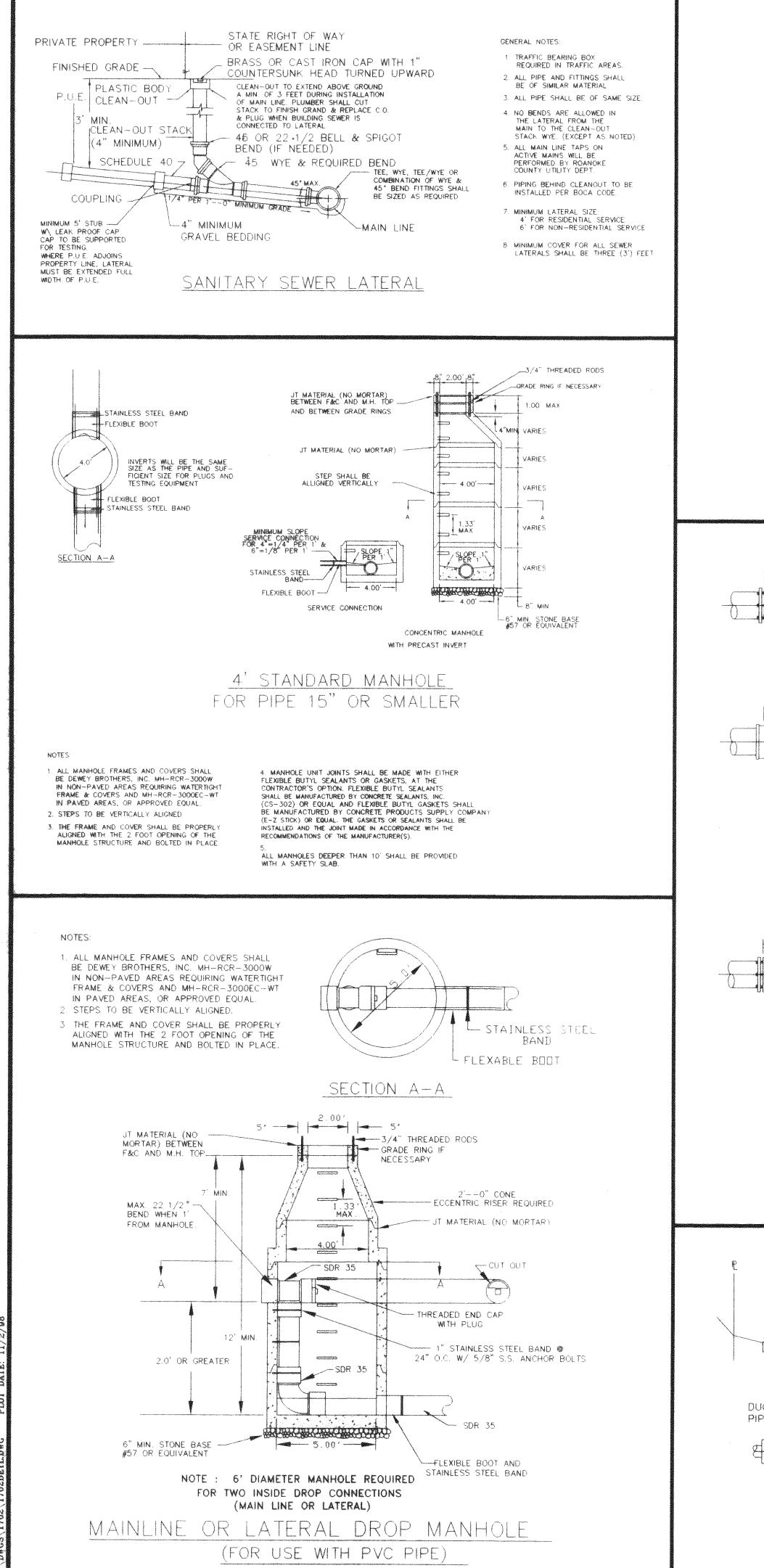
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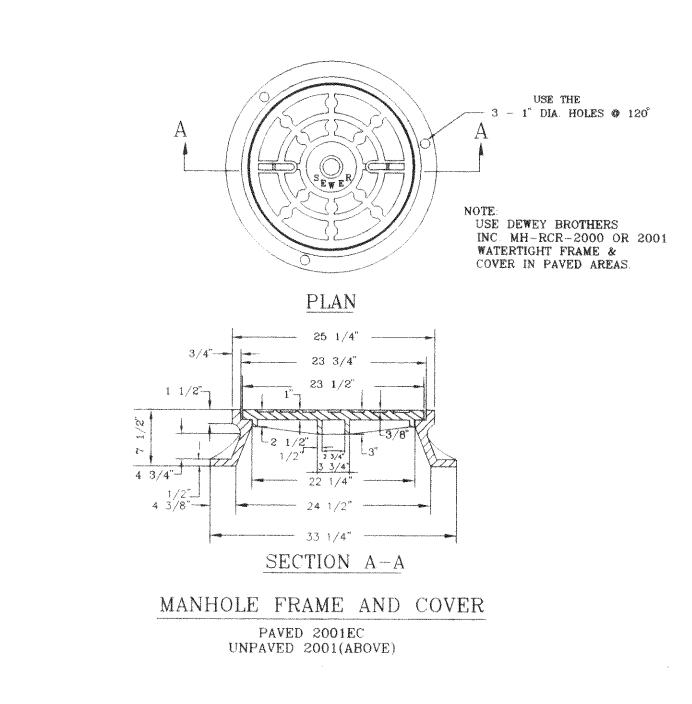
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LESS THAN 18"

18" MIN.

10' MIN.

SEWER LINE

* SEWER LINE --

10' MIN.

-WATER LINE

OF AWWA WATER LINE MATERIAL & PRESSURE

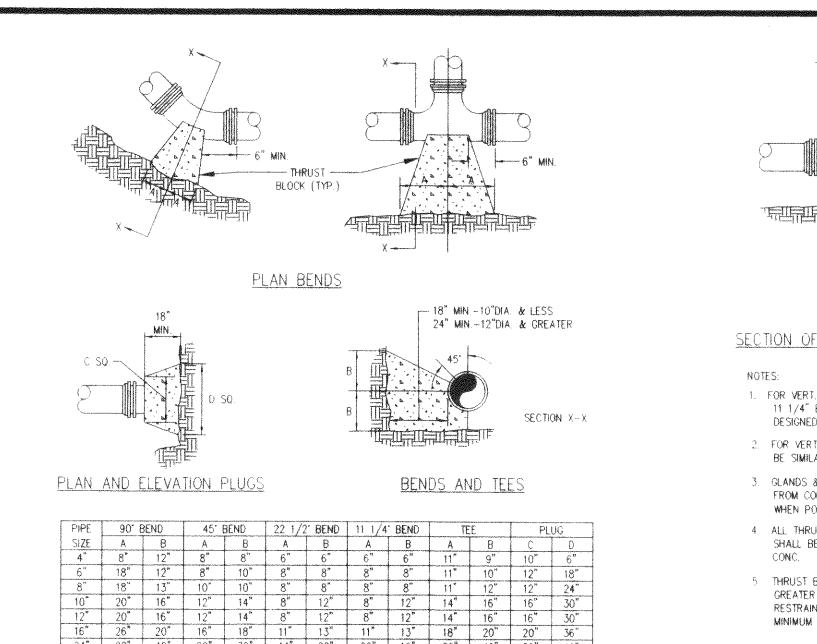
WATER and SEWER CROSSING DETAIL

TESTED (SEE SPEC.) AT NO ADDITIONAL

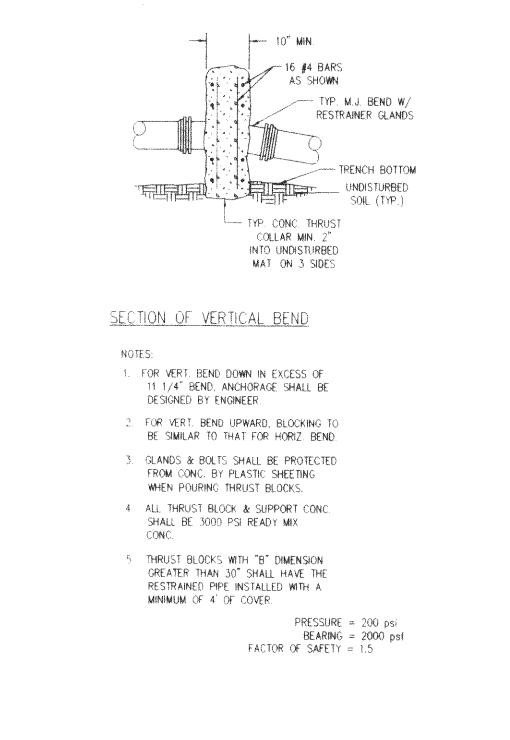
COST (SEE SEWER LINE COUPLING DETAIL)

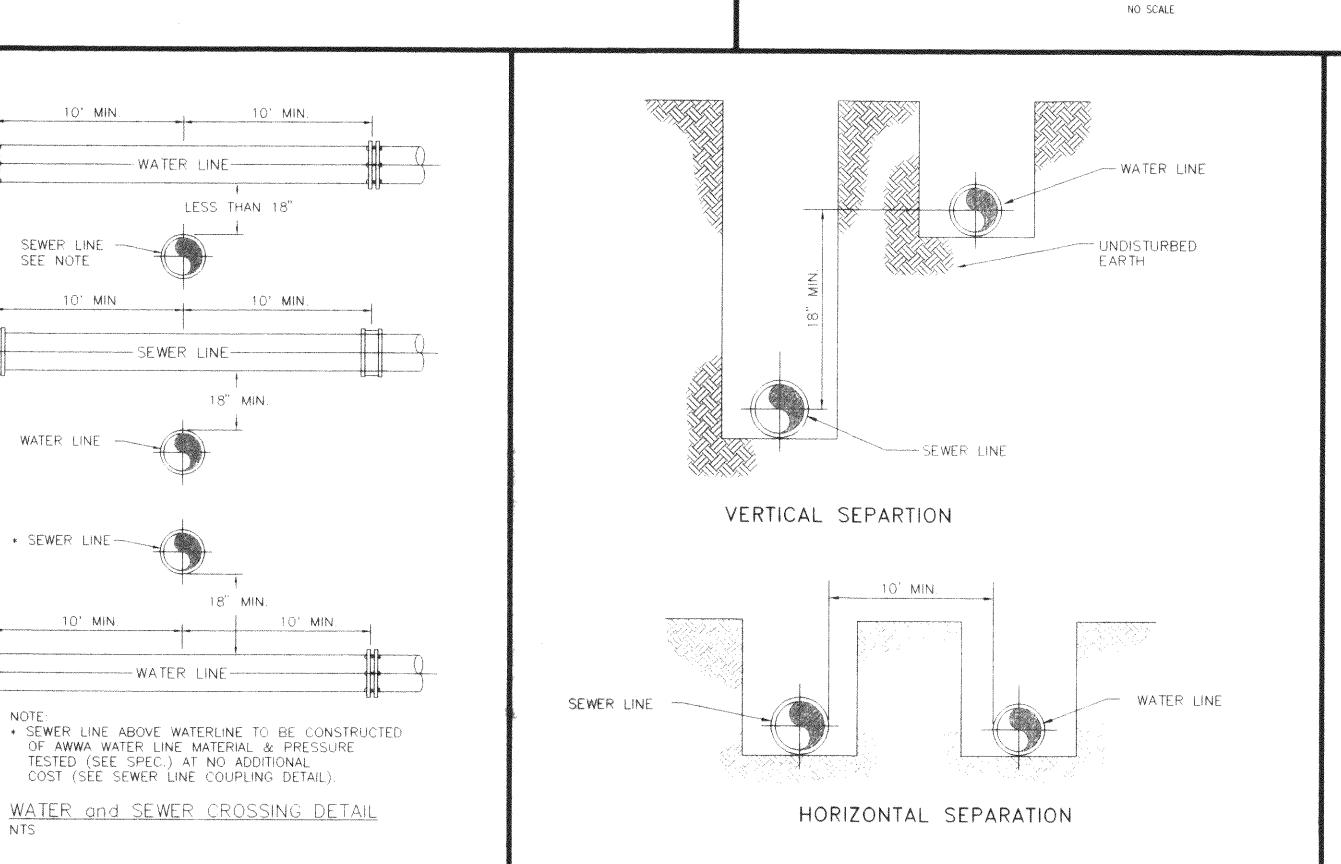
10' MIN

SEE NOTE

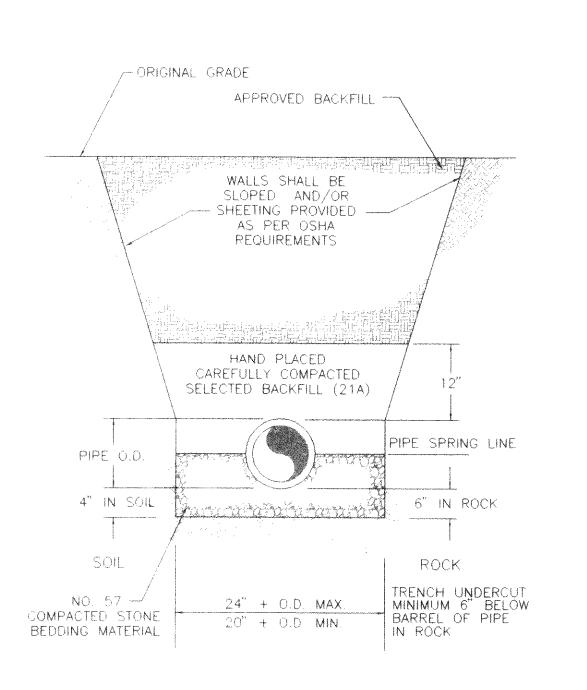


THRUST BLOCK CONSTRUCTION

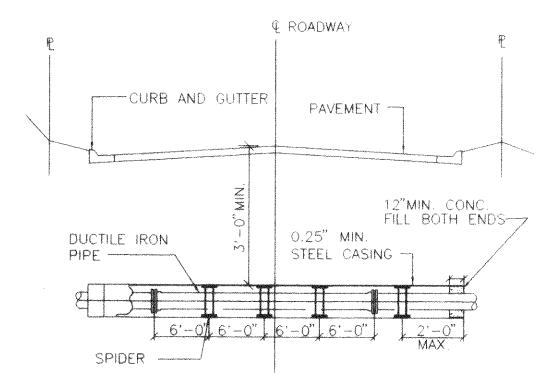




WATER and SEWER SEPARATION DETAIL NTS

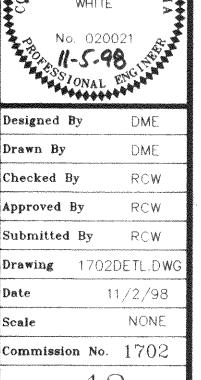






NOTE: STEEL CASING TO EXTEND TO BACK OF CURB OR MIN. 3'-0" BEYOND EDGE OF PAVEMENT ALL JOINTS IN CASING PIPES SHALL HAVE RESTRAINED JOINTS

TYPICAL STEEL SLEEVE INSTALLATION UNDER ROADWAY NTS



RGN

DETALS

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